Pond Siting Report

Gibsonton Drive From Fern Hill Drive to US 301

Project Development & Environment (PD&E) Study

Work Program Item Segment No. 450438-1 ETDM Project No. 14493 Hillsborough County, Florida



Florida Department of Transportation District Seven

In Coordination with:



December 2023

The environmental review, consultation, and other actions required by applicable federal environmental laws for this project are being, or have been, carried out by FDOT pursuant to 23 U.S.C. § 327 and a Memorandum of Understanding dated May 26, 2022, and executed by FHWA and FDOT.

Gibsonton Drive From Fern Hill Drive to US 301

Project Development & Environment (PD&E) Study

Pond Siting Report

Work Program Item Segment No. 450438-1 ETDM Project No. 14493 Hillsborough Counties, Florida

Prepared for:



Florida Department of Transportation **District Seven**

In Coordination with:



Prepared by: American Consulting Engineers of Florida, LLC 2818 Cypress Ridge Boulevard, Suite 200 Wesley Chapel, FL 33544

December 2023



This item has been digitally signed and sealed by:

On the date indicated adjacent to the seal:

Printed Copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

American Consulting Engineers of Florida, LLC 2818 Cypress Ridge Boulevard, Suite 200 Wesley Chapel, FL 33544 Eric K. Nelson Jr., P.E. NO. 79361

EXECUTIVE SUMMARY

The Florida Department of Transportation (FDOT) District Seven in Coordination with Hillsborough County is conducting a Project Development and Environment (PD&E) study along Gibsonton Drive from Fern Hill Drive to U.S. Highway 301 (US 301), in Hillsborough County. The study evaluates the widening of this section of Gibsonton Drive from a 4-lane divided facility to a 6-lane divided facility and includes pedestrian and bicycle accommodations. The study also evaluates issues related to traffic operations, safety, access management, and freight movements. The proposed improvements will include construction of stormwater management facility (SMF) and floodplain compensation (FPC) sites. The proposed improvements in this study will accommodate improvements at the I-75/Gibsonton Drive interchange as well as improvements at Gibsonton Drive/Fern Hill Drive intersection as proposed under other projects.

This *Pond Siting Report* (PSR) has been prepared to evaluate and identify stormwater management requirements for attenuation and treatment of surface water runoff from proposed impervious areas, and for compensation of any impacts to the 100-year floodplain associated with the proposed build alternative. Existing Southwest Florida Water Management District (SWFWMD) permits, and the effective 2020 Alafia River Watershed Management Masterplan Report were used to determine stormwater management and floodplain compensation needs.

The preferred alternative results in the acquisition of right of way for one SMF and one FPC site will be required, along with the relocation of two businesses and two residential properties, for a total of 3.21 acres.

Three SMF alternatives are provided for Basin 1. SMF 1B and FPC 1A are the preferred alternatives, since they are the most cost effective, most hydraulically feasible, and least impactful. They require acquisition of 2 parcels.

The improvements within Basin 2 do not require the acquisition of right of way for stormwater management or floodplain compensation purposes.

Table of Contents

SECTION	1 INTRODUCTION	1-1
1.1	PROJECT DESCRIPTION	1-1
1.2	PROJECT PURPOSE AND NEED	1-1
	1.2.1 Purpose	1-1
	1.2.2 Need	1-1
	1.2.1 Project Status	1-3
	1.2.2 Roadway Capacity	1-3
	1.2.3 Safety	1-3
1.3	EXISTING FACILITY AND PROPOSED IMPROVEMENTS	1-4
	1.3.1 Existing Facility	1-4
	1.3.2 Proposed Improvements	1-4
1.4	REPORT PURPOSE	1-5
SECTION		2 1
2 1		2-1
2.1		2 1 2_1
2.2		
2.5		
2.4		2-1 2_1
2.5	EXISTING DRAINAGE PATTERNS	
2.0		
2.7	SPECIAL BASIN CRITERIA	
2.0		
SECTION	3 PROPOSED CONDITIONS	
3.1	PROPOSED ROADWAY	
3.2	CROSS DRAINS	
3.3	BRIDGE STRUCTURES	
3.4	FLOODPLAINS AND FLOODWAYS	3-1
SECTION	4 STORMWATER MANAGEMENT ALTERNATIVES	4-1
4.1	STORMWATER MANAGEMENT CRITERIA	4-1
	4.1.1 WATER QUALITY	4-1
	4.1.2 DISCHARGE ATTENUATION	4-1
4.2	AGENCY COORDINATION	4-1
4.3	PROJECT STORMWATER MANAGEMENT ALTERNATIVES	4-2
	4.3.1 STORMWATER MANAGEMENT REQUIREMENTS	4-2
	4.3.2 PROPOSED LAND USE	4-2
	4.3.3 BASIN CONSIDERATIONS	4-2
4.4	FLOODPLAIN COMPENSATION SITE ALTERNATIVES	4-5
	4.4.1 FLOODPLAIN COMPENSATION REQUIREMENTS	4-5
	4.4.2 FLOODPLAIN CONSIDERATIONS	4-6
SECTION	5 ENVIRONMENTAL CONSIDERATIONS	
02011011	5.1.1 WFTI ANDS/SUBFACE WATER	
	5.1.2 SOCIO-CULTURAL FEATURES	
	5.1.3 HISTORIC	5-1
	5.1.4 PARKS AND RECREATION	
	5.1.5 THREATENED AND ENDANGERED SPECIES	
	HAZARDOUS MATERIAL AND CONTAMINATION IMPACT	
SECTION		C 1
JECHUN		

List of Figures and Tables

Figures

Figure 1-1	Project Location Map	. 1-2
Figure 1-2	Gibsonton Drive – Existing Typical Section	. 1-4
Figure 1-3	Gibsonton Drive – Proposed Typical Section	. 1-5
Figure 4-1	SMF 1A	. 4-3
Figure 4-2	SMF 18	. 4-4
Figure 4-3	SMF 1C	. 4-4
Figure 4-4	SMF 2	. 4-5
Figure 4-5	FPC 1A	. 4-6
Figure B-1	Soils Map	1
Figure B-2	Land Use Map	2
Figure B-3	Hillsborough County Stormwater Management Model (HCSWMM) Map	3

Tables

<u>Tables</u>		
Table 1-1	Gibsonton Drive Number of Crashes for 2018-2022	1-3
Table 1-2	Crash Rates for Segments	1-3
Table 1-3	Crash Rates for Intersections	1-4
Table 2-1	USDA Soils	2-1
Table 4-1	Stormwater Management Requirements	4-2
Table 4-2	Land Use Summary	4-2
Table 4-3	Floodplain Encroachment Summary	4-5
Table 4-4	SMF and FPC Site Matrix	4-7
Table 6-1	Preferred SMF and FPC Site Matrix	6-0

Appendices

APPENDIX A	CONCEPTUAL DRAINAGE MAPS AND
APPENDIX B	FIGURES
APPENDIX C	CALCULATIONS
APPENDIX D	ENGINEER'S ESTIMATE OF CONSTRUCTION COST
APPENDIX E	SUPPORTING DOCUMENTATION
APPENDIX F	MEETING MINUTES

SECTION 1 INTRODUCTION

The objective of the Project Development and Environment (PD&E) study is to assist the Florida Department of Transportation's (FDOT) Office of Environmental Management (OEM) in reaching a decision on the type, location, and conceptual design of the proposed improvements for the widening of Gibsonton Drive. The PD&E study satisfies all applicable requirements, including the National Environmental Policy Act (NEPA), to qualify for federal-aid funding of subsequent development phases (design, right of way acquisition, and construction).

1.1 PROJECT DESCRIPTION

The project consists of widening Gibsonton Drive from Fern Hill Drive to US 301 in Hillsborough County, a distance of approximately 0.95 miles. Improvements will also include a wide sidewalk to accommodate bicycles and pedestrians. The project includes the evaluation of stormwater management facilities (SMF) and floodplain compensation (FPC) sites. The project traverses the unincorporated census designated place of Riverview and provides access to I-75 for the communities of Riverview, Boyette, Fish Hawk and Lithia. Within the project limits, Gibsonton Drive is a four-lane, divided roadway with paved shoulders and 5-foot (ft) sidewalks along both sides of the road. There are some gaps in the sidewalk on the south side (eastbound direction) of the road. Gibsonton Drive is functionally classified by Hillsborough County as an arterial with an existing posted speed limit of 45 miles per hour (mph). A project location map is provided in **Figure 1-1**.

This project was screened through the FDOT's Efficient Transportation Decision Making (ETDM) process as ETDM Project No. 14493. The ETDM Programming Screen Summary Report was published on October 27, 2022, containing comments from the Environmental Technical Advisory Team (ETAT) on the project's effects on various natural, physical, and social resources. A Type 2 Categorical Exclusion is the class of action for this PD&E study.

1.2 PROJECT PURPOSE AND NEED

1.2.1 Purpose

The purpose of this project is to address future roadway capacity issues as well as improve safety conditions on Gibsonton Drive, which is an important east-west connection between I-75 and US 301.

1.2.2 Need

This project is needed to accommodate traffic volumes for the future year (2045) and to accommodate projected traffic flows from the proposed reconstruction of the I-75/Gibsonton Drive interchange. Additionally, this segment experiences high crash rates that are higher than the statewide average for similar facilities.





1.2.1 Project Status

This project is listed as a candidate for funding in the Hillsborough Transportation Planning Organization (TPO) FY 2023/24-2027-28 Transportation Improvement Program (TIP). Funding for the PD&E study has been requested and an application for Federal funding has been submitted. The project is also listed in the Cost Feasible Plan of the Hillsborough County TPO's 2045 Long Range Transportation Plan (LRTP).

1.2.2 Roadway Capacity

Within the project limits, Gibsonton Drive operates at Level of Service (LOS) F and fails to meet target LOS D, based on 2022 traffic counts. The Gibsonton Drive segment west of Fern Hill Drive is currently not six lanes; however, with the addition of the I-75/Gibsonton Drive interchange improvements, Gibsonton Drive will be widened to six lanes between I-75 and Fern Hill Drive. The segment directly to the east of the project limits is six lanes, thus creating a bottleneck. This segment is projected to continue to operate deficiently in the year 2045 at LOS F with no capacity improvements. This analysis is based on the Generalized Service Volume Tables from the FDOT 2023 Multimodal Quality/Level of Service Handbook for a context classification suburban commercial (C3C) facility and utilizes traffic forecasts from the Tampa Bay Regional Planning Model (TBRPM).

1.2.3 Safety

Crash data was collected for a five-year period including the years 2018 – 2022 and are summarized in **Table 1-1**. This segment suffered a high number of crashes considering its short length (less than one (1) mile). This is reflected in the high crash rates summarized in Table 1-2 and Table 1-3. The calculated crash rates for the segments and intersections are higher than the statewide average rate for similar state facilities except at a short segment between Mathog Road and the Park Place Avenue/Alafia Trace Boulevard intersection, and at the Park Place Avenue/Alafia Trace Boulevard intersection.

Table 1-1 Gibsonton Drive Number of Crashes for 2018-2022						
Limits	2018	2019	2020*	2021	2022	Total
Gibsonton Drive from Fern Hill Drive to US 301	220	239	153	136	162	910

Source: Signal 4 Analytics

*Crashes in 2020 are substantially less than those in 2019 due to COVID

Segr	Crachac	Length	2022	Crash	Statewide	Above		
From	То	Crasiles	(mi)	(mi)	AADT	(MVMT)	Average	Average?
Fern Hill Dr	Mathog Rd	95	0.33	45,800	3.444	1.747	Yes	
Mathog Rd	Park Place Ave	4	0.03	44,000	1.660	1.747	No	
Park Place Ave	US 301	27	0.14	45,600	2.317	1.747	Yes	

Table 1-2 **Crash Rates for Segments**

Note: Crashes reported to occur within intersection turn lanes were extracted out of the segments.

Intersection	Crashes	Entering Volume	Crash Rate (MEV)	Statewide Average	Above Statewide Average?		
Fern Hill Dr	159	57,750	1.509	0.526	Yes		
Mathog Rd	68	45,200	0.824	0.526	Yes		
Park Place Avenue/Alafia Trace Blvd	3	47,500	0.035	0.526	No		
US 301	554	99,800	3.042	0.744	Yes		

Table 1-3 Crash Rates for Intersections

1.3 EXISTING FACILITY AND PROPOSED IMPROVEMENTS

1.3.1 Existing Facility

Gibsonton Drive is owned and maintained by Hillsborough County. Within the project area, Gibsonton Drive is currently a four-lane divided facility functionally classified as an arterial roadway with a posted speed limit of 45 mph. The roadway has two 12-foot (ft) lanes in each direction, a 22-ft median and turn lanes at many locations along the corridor. The shoulders are approximately 10-ft wide (4-ft paved) on the south side and 6.5-ft minimum width (4-ft paved) on the north side throughout the corridor with no dedicated bicycle lanes. There is a 5-ft sidewalk on both sides of the road with a few gaps in the sidewalk on the south side, west of Kendra Drive. Approximately 230 linear feet of the sidewalk on the south side, east of Kendra Drive, is a wooden boardwalk. The existing right of way (ROW) varies along the corridor between 125 ft and 150 ft wide. The existing typical section is provided as **Figure 1-2**. There is one existing SMF east of US 301, but no SMF between Fern Hill Drive and US 301 and no existing FPC sites within the project corridor.



Figure 1-2 Gibsonton Drive – Existing Typical Section

1.3.2 Proposed Improvements

The proposed typical section shows widening Gibsonton Drive to a six-lane divided urban facility with a 22ft raised median. There will be two 11-ft travel lanes and one 12-ft outside travel lane in each direction with curb and gutter, and 10-ft wide sidewalks. The proposed typical section is provided as **Figure 1-3**. Additional ROW will be required along the north side of Gibsonton Drive (0 to 30 ft in width) to accommodate the widening and along the south side of Gibsonton Drive (0 to 7 ft in width) in advance of the US 301 intersection for intersection improvements. One off-site SMF and one off-site FPC are proposed. Additional ROW will be required for off-site SMF and FPC sites.



Figure 1-3 Gibsonton Drive – Proposed Typical Section

1.4 REPORT PURPOSE

As part of the PD&E Study, this *Pond Siting Report* identifies SMF and FPC site alternatives, and includes the analysis for selection of preferred sites. This study analyzed SMF site alternatives that are hydraulically feasible and environmentally permissible based on the best available information. These alternatives were then compared based on potential relocations and community impacts; environmental impacts including wetlands, upland habitat and protected species involvement; petroleum and hazardous materials contamination; and economic factors including right of way costs.

SECTION 2 EXISTING CONDITIONS

2.1 SOILS

Per National Natural Resources Conservation Service (NRCS) soils data, soils within the project limits are described by Hillsborough County, as listed in **Table 2-1** below. See **Figure B-1** in **Appendix B** for a map of the soils within the project area.

Table 2-1 USDA Soils							
Map #	Soil Name	Hydrologic Group	Depth to High Water Table (ft)	Soil Type	Description		
5	Basinger, Holopaw, and Samsula soils	A/D	0	Sandy and loamy soil	Depressional, very poorly drained		
7	Candler fine sand	А	>6.0	Sandy soil	Excessively drained soil, slopes 0-5%		
29	Myakka fine sand	A/D	0-1.0	Sandy and loamy soil	Poorly drained soil, slopes 0-2%		
41	Pomello fine sand	А	2.0 - 3.5	Sandy soil	Moderately well drained, slopes 0-5%		
61	Zolfo fine sand	А	1.5 - 3.5	Sandy soil	Somewhat poorly drained, slopes 0-2%		

2.2 LAND USE

This project lies within Hillsborough County. The existing land use within the project vicinity is a mix of residential, light commercial, and heavy commercial properties. **Figure B-2** in **Appendix B** displays the various land use types within the project area.

2.3 CROSS DRAINS

There is one cross drain within the project limits that ultimately outfalls to the Alafia River. Cross drain CD-1 is a 118 foot, double barrel, 42-inch RCP located at station 94+30, just east of Kenda Drive. The cross drain flows from south to north.

2.4 BRIDGE STRUCTURES

There are no existing bridges within the project limits.

2.5 FLOODPLAINS AND FLOODWAYS

FEMA Flood Insurance Rate Map (FIRM) panel 12057C0502J dated October 7, 2021, identifies the flood zone information for the project area, and can be seen in **Appendix A**. Per the FIRM panel, a small portion of the existing roadway near Kenda Drive/Hagadorn Road lies within Flood Zone A. The Hillsborough County Stormwater Management Model (HCSWMM) for the Alafia River Watershed, dated March 31, 2020, identifies additional floodplains within the project area. Per the County watershed model, the permitted linear ponds, and several segments of the existing ditch between Fern

Hill Road and US 301 are designated Flood Zone AE, with several portions of the existing roadway located within the inundation boundary. See **Figure B-3**, in **Appendix B**, for a map of the HCSWMM floodplain boundaries and nodes.

2.6 EXISTING DRAINAGE PATTERNS

<u>Basin 1</u>

Basin 1 begins near the beginning of the study limits at station 78+73 and extends to station 118+04 at the intersection of Gibsonton Drive and US 301. The parcels adjacent to Basin 1 have a mixed land use, comprising both commercial and residential properties.

Runoff from Basin 1 is conveyed through roadside ditches and side drains, flowing toward the center of the basin. It then reaches a double 42" RCP cross drain located east of Hagadorn Road, crossing Gibsonton Drive, and continues to flow from south to north to the Alafia River, which is tidally influenced.

There is a recently permitted project at the west end of the basin (ERP 45227.000) which involves improvements at the intersection of Gibsonton Drive and Fern Hill Drive. This Hillsborough County project (CIP No. 6960031) has not been constructed as of date.

Basin 2

Basin 2 begins at the intersection of Gibsonton Drive/Boyette Road and US 301, specifically at station 200+00 (118+04 back), and extends beyond the study limits to Balm Riverview Road, located at Station 240+80. The land adjacent to Basin 2 comprises a mix of commercial and residential properties.

Basin 2 flows toward the center of the basin to a permitted pond north of Boyette Road (ERP 2166.001 – Pond 1A), which then discharges into Rice Creek and, ultimately, the Alafia River.

2.7 EXISTING ENVIRONMENTAL PERMITS

There are multiple existing Southwest Florida Water Management District (SWFWMD) environmental resource permits (ERP) within the project corridor. SWFWMD Permit No. 45227.000 (Gibsonton Drive at Fern Hill Drive Intersection) provides information for the two permitted ponds on the north side of Gibsonton Drive. SWFWMD permit No. 2166.001 (Boyette Road - US Hwy. 301 to Balm Riverview Road) provides information for the north side of Boyette Road. Refer to **Appendix D** for excerpts from the permits that are adjacent to Gibsonton Drive and US 301.

2.8 SPECIAL BASIN CRITERIA

This project is within the Alafia River watershed, associated with water body ID (WBID) No. 1621G. This waterbody is impaired for this Dissolve Oxygen, Mercury (in fish tissue), and Nutrients (Chlorophyll-a) WBID 1621G has been placed in category 4a because there is an FDEP Adopted - EPA Approved Dissolved Oxygen and Nutrient TMDL. This project is not located within any Outstanding Florida Waters (OFW). The study basins discharge to the Alafia River.

SECTION 3 PROPOSED CONDITIONS

3.1 PROPOSED ROADWAY

The preferred build alternative proposes widening Gibsonton Drive from a 4-lane, flush shoulder, divided roadway to a 6-lane, curb and gutter, divided roadway with 11-ft inside lanes, 12-ft outside lanes, and typically 10-ft buffered sidewalks. The proposed sidewalk along the south side of Gibsonton Drive from approximately 900-ft west of US 301 will transition to be flush with the proposed curb to minimize right of way impacts. East of US 301, a 6-ft sidewalk is proposed along the south side of Gibsonton Drive to remain within the existing southerly right of way. The western limits of the build alternative will tie into the proposed I-75 and Gibsonton Drive Interchange Improvements (FPID 437650-2-32-01) approximately 500-ft east of Fern Hill Drive. The eastern limits of the build alternative will tie into existing Boyette Road/Gibsonton Drive approximately 900-ft east of US 301. Other than a No-Build alternative, there are no additional build alternatives being considered.

3.2 CROSS DRAINS

The existing cross drain CD-1 will be extended or replaced as needed to accommodate the new road width. A cross drain analysis will be performed during the design phase of the project.

3.3 BRIDGE STRUCTURES

There are no bridges within the project limits; therefore, no new bridge structures or bridge widening will be necessary.

3.4 FLOODPLAINS AND FLOODWAYS

As discussed in **Section 2.5**, Hillsborough County provided their latest HCSWMM to evaluate potential floodplain impacts. The FEMA Flood Insurance Rates Map was also reviewed. However, the Floodplain areas included in the County model were more extensive than the FEMA floodplain areas. Therefore, the County model was solely used for the purposes of this evaluation.

The build alternative is associated with minimal longitudinal encroachments within the floodplains north and south of Gibsonton Drive. The proposed improvements will require fill to be placed below the base flood elevations (BFE) of several floodplains identified in the County watershed model, ranging in elevation from 25.18 to 39.29. Equivalent cup-for-cup volumes of cut will be provided to compensate for the anticipated fill volumes, This will be achieved with floodplain compensation sites and providing compensatory storage volume between the estimated seasonal high-water table (SHWT) elevation and the BFE.

SECTION 4 STORMWATER MANAGEMENT ALTERNATIVES

4.1 STORMWATER MANAGEMENT CRITERIA

The following subsections describe water quality and quantity requirements for the project.

4.1.1 WATER QUALITY

The SWFWMD ERP Applicant's Handbook Vol. II, Part IV – Stormwater Quality identifies water quality treatment criteria. The selected system for treating runoff associated with the build alternative is wet detention. Wet detention facilities require treatment of one inch of runoff from the contributing area, according to Part IV, Section 4.1.a.1.

As discussed in **Section 2.8**, WBID 1621G is impaired for dissolved oxygen (DO). Nutrient loading calculations are provided for the 'worst case' SMF alternative, in **Appendix C**.

4.1.2 DISCHARGE ATTENUATION

The SWFWMD ERP Applicant's Handbook Vol. II, Part III – Stormwater Quantity/Flood Control identifies runoff attenuation and discharge criteria. In general, runoff associated with the build alternative must not cause adverse water quantity impacts to receiving waters or adjacent lands, must not cause adverse flooding to on-site or off-site properties, and must not adversely impact existing surface water storage and conveyance capabilities (Part III, Section 3.a-d).

The project is located within open drainage basins, thus the allowable discharge is equal to either the historical discharge or to amounts determined in previous District permit actions, as stated in Part III, Section 3.1.1-2.

Per Part III, Section 3.1.b, the post development peak discharge shall be no greater than predevelopment peak discharge for the 25-year, 24-hour storm event, and computed using the SCS type II Florida Modified 24-hour rainfall distribution. The previous permits, as described in Section 2.7 of this report, followed the same stormwater quantity criteria.

4.2 AGENCY COORDINATION

A SWFWMD pre-application meeting was held on May 8, 2023, to discuss the project's environmental, water quality, and water quantity considerations. Project coordination meetings with Hillsborough County staff were held on October 31, 2022, and on May 11, 2023, to discuss project status and design alternatives. A Pond Siting Longlist meeting with FDOT District 7 representatives was held on March 9, 2023, to discuss stormwater management facility and floodplain compensation site alternatives. Meeting minutes for the SWFWMD pre-application meeting and FDOT D7 longlist meeting can be found in Appendix **F**.

4.3 PROJECT STORMWATER MANAGEMENT ALTERNATIVES

4.3.1 STORMWATER MANAGEMENT REQUIREMENTS

Table 4-1, below, summarizes the stormwater management facility alternative pond sites. Supporting calculations are provided in Appendix C.

	Tuble + 1 Stormwater Management Requirements							
Basin	Estimated Required. Water Quality Treatment (ac-ft)	Estimated Required Discharge Attenuation Volume (ac-ft)	Estimated Required Stormwater Management Volume (ac-ft)					
1	0.14	1.37	1.51					
2	0.01	0.06	0.07					

Table 4-1 Stormwa	ater Management Requirements
-------------------	------------------------------

4.3.2 PROPOSED LAND USE

Pre- and post-conditions impervious and pervious coverages were determined by computing impervious and pervious areas of the preferred roadway alternative. Table 4-2 below summarizes the proposed land use.

Table 4-2 Land Use Summary						
Basin	Existing	Land Use (Ac) Proposed Land Use (Ac)			
	Impervious	Pervious	Total	Impervious	Pervious	Total
1	9.34	4.08	13.42	12.17	1.25	13.42
2	2.61	0.37	2.98	2.74	0.24	2.98

4.3.3 BASIN CONSIDERATIONS

Conceptual SMF & FPC Maps showing the following SMF and FPC alternatives are provided in Appendix A. Calculations for the following alternatives are in Appendix C.

Furthermore, the SMF and FPC alternatives were considered for environmental considerations. There are no wetlands and other surface waters impacts associated with the proposed SMF and FPC alternatives. The project will not have significant impacts on federally and state listed threatened and endangered species. Though there are multiple medium and low risk contamination sites within the project's 500-ft buffer, these identified sites are not located on or immediately adjacent to the properties evaluated for the SMF and FPC alternatives. Therefore, no contamination involvement is anticipated. The Alafia Preserve, situated north of Gibsonton Drive and approximately 1500-feet east of the intersection with Fern Hill Drive, will not be impacted with the project. Social and economic effects are anticipated to be minimal. Mobility is anticipated to be enhanced through intersection improvements and expansion to a 6-lane divided facility. The project will require two residential and two business relocations for the proposed SMF and FPC alternatives. There are previous archaeological and historic structures adjacent to the project on Gibsonton Drive. A desktop review and Cultural Resources Assessment Survey (CRAS) assessed there are no impacts to historic structures and archaeological resources with the associated proposed SMF and FPC alternatives. Overall,

environmental constraints with the proposed SMF and FPC alternatives are minimal due to work being limited to existing and acquired ROW at these sites. Further detail about the environmental considerations with the proposed SMF and FPC alternatives are discussed in Section 5 of this report.

<u>Basin 1</u>

Basin 1 begins near the beginning of the study limits at station 78+73 and extends to station 118+04 at the intersection of Gibsonton Drive and US 301.

Runoff from Basin 1 will be conveyed through a closed pipe network toward the proposed SMF, which will discharge to the existing double 42" RCP cross drain located east of Hagadorn Road, crossing Gibsonton Drive, discharging to the Alafia River.

There is a recently permitted project at the west end of the basin (ERP 45227.000) which involves improvements at the intersection of Gibsonton Drive and Fern Hill Drive. This Hillsborough County project (CIP No. 6960031) has not been constructed as of date. Two permitted roadside swales (CIP No. 6960031) would be impacted by the improvements proposed herein. However, the existing land use values provided in **Table 4-2** above reflect pre-permitted conditions and proposed land use values reflect both the improvements of the Fern Hill Drive project and the preferred alternative. Therefore, the impacted treatment and attenuation is accounted for within the following three stormwater management facility (SMF) alternatives.

<u>SMF 1A</u>

SMF 1A is located at the west end of the basin near Fern Hill, south of Gibsonton Drive, and is an expansion of a permitted SMF, Pond 2B (ERP 45227.000). The expansion would require the acquisition of four residential parcels to the south. Pond 2B is permitted to be a wet pond and would remain so as the expanded SMF 1A. The seasonal high-water table (SHWT) and design high water (DHW) would

presumably remain unchanged from the permitted condition. The existing freeboard of 6" meets Hillsborough County Criteria. The control structure may require modification. Compensatory treatment and attenuation are required for this SMF to comply with criteria. This means that existing untreated runoff will comingle with a portion of the runoff generated by the proposed improvements. The total directly connected impervious area (DCIA) will be equal to or greater than the total new impervious area of the preferred roadway alternative. This can be accomplished by ensuring that at least 1000 ft of the proposed 6-lane roadway will drain to the SMF. Since this SMF is west of the project limits,



Figure 4-1 SMF 1A

an additional 700 feet of pipe is necessary to convey runoff from the project area to the SMF. The total parcel area required for SMF 1A is 1.47 acres.

<u>SMF 1B</u>

SMF 1B is located on a 1.67-acre residential parcel at the center of the basin between Park Place Avenue and Kenda Drive, south of Gibsonton Drive,. Since this SMF alternative is located near the basin outfall, it is the most hydraulically feasible location. The SHWT is estimated to be 25.84 ft-NAVD per the permitted plans from a nearby project (ERP 21779.009 – Lowes Riverview Town Centre). Compensatory treatment and attenuation may be utilized to meet criteria. This can be accomplished by ensuring that at least 1000 ft of the proposed 6lane roadway will drain to the SMF. Since this SMF is near the primary outfall of the basin, additional pipe is not required. The total parcel area required for SMF 1B is 1.47 acres.



Figure 4-2 SMF 1B



Figure 4-3 SMF 1C

<u>SMF 1C</u>

SMF 1C is located at the east end of the basin between Oakridge Avenue and Pineridge Avenue, north of Gibsonton Drive,. The SMF requires the acquisition of eight residential parcels, two of which are necessary for the expansion of Gibsonton Drive. A SHWT of 34 ft-NAVD is assumed to be 12 inches below the lowest grade of 35 ft-NAVD. The total area is 1.79 acres. Compensatory treatment and attenuation are required for this SMF to comply with criteria. This can be accomplished by ensuring that at least 1000 ft of the proposed 6-lane roadway will drain to the SMF. This will require an additional 500 feet of pipe convey runoff to from

the west back to the pond and then toward the outfall at the cross drain.

<u>Basin 2</u>

Basin 2 begins at the intersection of Gibsonton Drive/Boyette Road and US 301, specifically at station 200+00 (118+04 back), and extends beyond the study limits to Balm Riverview Road, located at Station 240+80.

Basin 2 will continue to flow the permitted pond north of Boyette Road (ERP 2166.001 – Pond 1A), which discharges into Rice Creek and, ultimately, the Alafia River. For the purposes of this report this permitted pond is referred to as SMF 2.

The proposed improvements within Basin 2 consist of the addition of a turn lane of less than a quarter mile and a sidewalk. These improvements can be considered exempt from permitting. The resulting DHW in SMF 2 is calculated to rise a minimal 0.04 feet.



Figure 4-4 SMF 2

4.4 FLOODPLAIN COMPENSATION SITE ALTERNATIVES

4.4.1 FLOODPLAIN COMPENSATION REQUIREMENTS

The improvements proposed within the preferred roadway alternative will require fill to be placed with the floodplain within Basin 1. No encroachments are likely in Basin 2. These encroachments are listed in **Table 4-3** below.

Floodplain	Project Floodplain Limits	Base Flood Elev. (ft-NAVD)	Estimated Floodplain Encroachment Area (ac)	Estimated Floodplain Encroachment Volume (ac-ft)			
1	87+15 to 91+70 (Right)	28.18	0.254	0.142			
2	94+65 to 94+30 (Left)	25.18	0.027	0.007			
3	92+12 to 97+12 (Right)	28.18	0.201	0.076			
4	98+07 to 102+90 (Right)	32.66	0.205	0.016			

Table 4-3 Floodplain Encroachment Summary

4.4.2 FLOODPLAIN CONSIDERATIONS

<u>FPC 1A</u>

FPC 1A is located on a 1.54-acre residential parcel between Park Place Avenue and Kenda Drive, south of Gibsonton Drive. This parcel is directly connected to the floodplain associated with the encroachments. The elevations range from 26.5 to 29.3 ft-NAVD. The SHWT is estimated to be 25.84 ft-NAVD per ERP 21779.009. Floodplain compensation may result in the loss of upland area, placing the parcel entirely within the floodplain. Compensation for the floodplain encroachments would be evaluated on a cup-for-cup basis. This FPC alternative could be used with SMF 1A, SMF 1B, and SMF 1C.

Other FPC sites were not considered because there were no hydraulically connected sites that were feasible within the encroached Floodplain.



Figure 4-5 FPC 1A

SMF #	Pond Area (Ac)	FPC #	FPC Area (Ac)	Conveyance Easement (Ac)	Est. Wetland Impacts (Ac)	Probability of Species Occurrence	Contamination	Cultural Resources	Potential Relocations ¹	Est. Construction Cost ²	Est. Wetland Mitigation Cost	Est. Right of Way Cost SMF ³	Est. Total Cost	Comments
1A	1.47	1A	1.49	N/A	-	Low	None	None	5R + 1B	\$277,857	\$0	\$3,899,800	\$4,177,657	
1B	1.66	1A	2	N/A	-	Low	None	None	2R + 2B	\$73,736	\$0	\$3,197,200	\$3,270,936	Preferred Alternative
1C	1.79	1A	1.49	N/A	-	Low	None	None	9R + 1B	\$221,745	\$0	\$5,733,500	\$5,955,245	

Table 4-4SMF and FPC Site Matrix

 ^{1}R = Residential; B = Business – (the business relocations are for landlord business for the residential relocations)

²Engineer's Estimate of Construction Cost provided in Appendix D.

³Total of SMF and FPC cost.

SECTION 5 ENVIRONMENTAL CONSIDERATIONS

The following sections identify and discuss environmental considerations.

5.1.1 WETLANDS/SURFACE WATER

The project would result in approximately 0.17 acres of wetland and 0.17 acres of surface water impacts with the Preferred Build Alternative. However, these impacts are not associated with the proposed SMF and FPC alternatives. Impacts are planned to be mitigated through the purchase of wetland mitigation credits through an approved mitigation bank, or creation, restoration, or enhancement of wetlands within the project.

5.1.2 SOCIO-CULTURAL FEATURES

Social and economic effects are anticipated to be minimal. There are no planned changes to land use nor aesthetics. Economic conditions may be enhanced through the enhanced mobility. There is no involvement with farmland resources as defined by 7 CFR Part 658.

A *Conceptual Stage Relocation Plan* will be prepared. The project will require 2 residential and 2 business relocations for the preferred SMF and FPC sites. Relocations will be carried out in accordance with Florida Statutes and the *Uniform Relocation Assistance and Real Property Acquisition Policies Act of 1970*.

Mobility is anticipated to be enhanced for motorized vehicles through the improved safety of the corridor through expansion to a 6-lane divided facility and improved operation of the Gibsonton Drive intersections at Fern Hill Drive, Mathog Road, Park Place/Alafia Trace Boulevard and US 301. Pedestrian and bicycle mobility and safety will be improved through constructing the 10-foot-wide sidewalk along both sides of Gibsonton Drive from Fern Hill Drive to US 301.

5.1.3 HISTORIC

According to the *Cultural Resources Assessment Survey* (CRAS), the proposed undertaking should have no adverse effect on resources listed, eligible or potentially eligible for listing for the NRHP. A desktop analysis of the project area and SMF/FPC site alternatives from the records of the Florida Master Site File (FMSF) along with the *Cultural Resources Assessment Survey* were assessed to document the presence of any historic or archaeological resources that meet the eligibility criteria for inclusion into the National Register of Historic Places (NHRP). Previous records from the FMSF were examined to determine the location of any previously conducted CRAS(s) and recorded historic resources within 0.5 miles of the project area.

According to the FMSF, one previous recorded resource group (8HI12137), and one historical resource (8HI11301) were identified in the project area. 8HI11301 has been demolished, and the FMSF will be updated with this information. The US 301 (8HI12137) resource group is a previously recorded linear resource built in 1870. Field survey methods included pedestrian and subsurface testing throughout

the project area in the form of shovel pit tests (STPs) plotted at 25-meter (m), 50-m, and 100-m intervals. In total, 32 STPs were pre-plotted, of which 8 STPs could be safely excavated due to buried utilities in the adjacent ROW of Gibsonton Drive. No archaeological materials were encountered during the field survey. The segment was recently recorded and the SHPO determined there was insufficient information to evaluate the resource for listing in the NHRP. The scope of work within the boundary of 8HI12137 includes widening to accommodate through and right-turn lanes. The proposed construction of traffic islands and concrete curb/gutter/sidewalks, and providing additional bicycle/pedestrian facilities. Due to the nature of the scope of work, the CRAS determined the proposed activities will have no adverse effect on 8HI12137.

The CRAS identified nine newly recorded structures (8HI1551-8HI1559) and one newly recorded resource group (8HI15513). The structures are Masonry Vernacular or Frame Vernacular residences built between 1956-1979. The District recommends all sites are ineligible for listing in NHRP. It is the district's recommendation that these structures in the context of a group do not meet the eligibility criteria for nomination of a historic district. The CRAS assessed archaeological Gibsonton Drive (8HI15513) is a newly recorded linear resource built in 1921 and cannot be fully documented outside of the project area. The proposed construction of traffic islands and concrete curb/gutter/sidewalks, and providing additional bicycle/pedestrian facilities. Due to the nature of the scope of work, the CRAS determined the proposed activities will have no adverse effect on 8HI12137.

The proposed ponds location is a residential area bordered by commercial properties. Seven of the eight pre-plotted STPs within the proposed pond area were safely excavated. All STPs were negative.

5.1.4 PARKS AND RECREATION

The Alafia Scrub Nature Preserve is situated north of Gibsonton Drive and lies adjacent to the roadway right of way approximately 1500-feet east of the intersection with Fern Hill Drive. This parcel will not be directly impacted with the project. There are no effects to protected properties pursuant to *Section 4(f) of the USDOT Act of 1966, Section 6(f) of the Land and Water Conservation Fund of 1965,* nor other recreational or protected lands.

5.1.5 THREATENED AND ENDANGERED SPECIES

After SMF and FPC site alternatives were identified, desktop and field reviews were conducted for the study limits and compiled into the *Natural Resources Evaluation* (NRE). The NRE documented all potential involvement of species and wetlands within the project area. The project will not have significant impacts on natural resources. There are several listed species that may be present, or their habitat may be present, but the effect determination of may affect, not likely to affect was made for these species including the following Federal Listed faunal and floral species: eastern indigo snake, wood stork, Britton's beargrass, and Florida golden aster. A no adverse effect is anticipated for the following State Listed faunal species: gopher tortoise, short-tailed snake, southeastern American kestrel, Florida pine snake, little blue heron, tricolored heron, reddish egret, roseate spoonbill, and

Florida sandhill crane. The proposed SMF and FPC sites will have no impact on federal and state listed flora/fauna species.

HAZARDOUS MATERIAL AND CONTAMINATION IMPACT

After SMF and FPC site alternatives were identified, desktop and field reviews were conducted for the study limits and compiled into the *Contamination Screening Evaluation Report (CSER)*. The assignment of a contamination risk rating was based on the current and past presence of contamination and the potential of contamination to be encountered during proposed roadway activities and the potential impact on roadway construction. The contamination risk rating system is divided into four degrees of risk including No Risk, Low Risk, Medium Risk, and High Risk. The risk ratings are defined by the FDOT PD&E Manual. There were six potential contamination sites identified as Medium Risk and nine identified as Low Risk for contamination involvement. These identified sites are not located on or immediately adjacent to the properties evaluated for the SMF and/or FPC alternatives; therefore, the SMF and FPC alternatives do not pose a risk for contamination involvement.

SECTION 6 SUMMARY AND RECOMMENDATIONS

This evaluation finds that acquisition of right of way for one SMF and one FPC site will be required. **Table 6-1** presents a site matrix indicating the preferred SMF and FPC alternatives.

Three SMF alternatives are provided for Basin 1. SMF 1B and FPC 1a are the preferred alternatives, since they are the most cost effective, and most hydraulically feasible.

The improvements within Basin 2 do not require the acquisition of right of way for stormwater management or floodplain compensation purposes.

See **Appendix E** for the right of way cost estimate at each SMF and FPC.

SMF #	Pond Area (Ac)	FPC #	FPC Area (Ac)	Conveyance Easement (Ac)	Est. Wetland Impacts (Ac)	Probability of Species Occurrence	Contamination	Cultural Resources	Potential Relocations ¹	Est. Construction Cost ²	Est. Wetland Mitigation Cost	Est. Right of Way Cost SMF ³	Est. Total Cost
1B	1.66	1A	2	N/A	-	Low	None	None	2R + 2B	\$73,736	\$0	\$3,197,200	\$3,270,936

Table 6-1Preferred SMF and FPC Site Matrix

¹R = Residential; B = Business – (the business relocations are for landlord business for the residential relocations)

²Engineer's Estimate of Construction Cost provided in Appendix D.

³Total of SMF and FPC cost.

APPENDICES

APPENDIX A CONCEPTUAL DRAINAGE MAPS AND

- APPENDIX B FIGURES
- APPENDIX C CALCULATIONS
- APPENDIX D ENGINEER'S ESTIMATE OF CONSTRUCTION COST
- APPENDIX E SUPPORTING DOCUMENTATION
- APPENDIX F MEETING MINUTES



APPENDIX A

Conceptual Drainage Maps and FEMA FIRM MAPS



Dec 04, 2023 - 11:58am C:\PWDrive\ACP\5217733_002\3. CADD\drainage\DRMPRD_PSR01.dwg





Dec 04, 2023 - 11:58am C:\PWDrive\ACP\5217733_002\3. CADD\drainage\DRMPRD_PSR01.dwg



FLOOD HAZARD INFORMATION

SPECIAL FLOOD

OTHER AREAS OF

FLOOD HAZARD

OTHER

AREAS

GENERAL

OTHER FEATURES

HAZARD AREAS

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

THE INFORMATION DEPICTED ON THIS MAP AND SUPPORTING

DOCUMENTATION ARE ALSO AVAILABLE IN DIGITAL FORMAT AT

HTTPS://MSC.FEMA.GOV

Regulatory Floodway

See Notes. Zone X

STRUCTURES Levee, Dike, or Floodwall

(8)---- Coastal Transect

 $\langle \mathbf{E} \rangle$

Future Conditions 1% Annual

Chance Flood Hazard Zone X

NO SCREEN Area of Minimal Flood Hazard Zone X

----- Channel, Culvert, or Storm Sewer

<u>17.5</u> Water Surface Elevation

— --- Coastal Transect Baseline

-- Profile Baseline

Limit of Study

- Hydrographic Feature

· Jurisdiction Boundary

Base Flood Elevation Line (BFE)

<u>18.2</u> Cross Sections with 1% Annual Chance

Without Base Flood Elevation (BFE) Zone A,V, A99

With BFE or Depth Zone AE, AO, AH, VE, AR

0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average

depth less than one foot or with drainage areas of less than one square mile *Zone X*

Area with Reduced Flood Risk due to Levee

Area with Flood Risk due to Levee Zone D

Area of Undetermined Flood Hazard Zone D

NOTES TO USERS

For information and questions about this Flood Insurance Rate Map (FIRM), available products associated with this FIRM, including historic versions, the current map date for each FIRM panel, how to order products, or the National Flood Insurance Program (NFIP) in general, please call the FEMA Mapping and Insurance eXchange at 1-877-FEMA-MAP (1-877-336-2627) or visit the FEMA Flood Map Service Center website at https://msc.fema.gov. Available products may include previously issued Letters of Map Change, a Flood Insurance Study Report, and/or digital versions of this map. Many of these products can be ordered or obtained directly from the website.

Communities annexing land on adjacent FIRM panels must obtain a current copy of the adjacent panel as well as the current FIRM Index. These may be ordered directly from the Flood Map Service Center at the number listed above.

For community and countywide map dates refer to the Flood Insurance Study Report for this jurisdiction.

To determine if flood insurance is available in this community, contact your Insurance agent or call the National Flood Insurance Program at 1-800-638-6620.

Base map information shown on this FIRM was provided by Hillsborough County, dated 2008 and 2018; the Florida Department of Transportation, dated 2017; the Florida Resources and Environmental Analysis Center, dated 2003; and the U.S. Department of Agriculture, dated 2018.





NATIONAL FLOOD INSURANCE PROGRAM FLOOD INSURANCE RATE MAP HILLSBOROUGH COUNTY, FLORIDA and Incorporated Areas PANEL 502 OF 801 Panel Contains: COMMUNITY NUMBER PANEL SUFFIX HILLSBOROUGH COUNTY 120112 0502 J

National Flood Insurance Program

ZONE

VERSION NUMBER 2.4.3.5 MAP NUMBER 12057C0502J MAP REVISED **OCTOBER 7, 2021**

APPENDIX B

Figures



Figure B-1 Soils Map



Figure B-2 Land Use Map



Figure B-3 Hillsborough County Stormwater Management Model (HCSWMM) Map

APPENDIX C CALCULATIONS
Basin Evaluation

Existing Conditions							
	Impervious	Pervious	Total	Curve Number			
Basin 1	9.337	4.085	13.422	83.1			
Roadway	8.914		8.914	98.0			
Sidewalk	0.423		0.423	98.0			
Pervious		4.085	4.085	49.0			
Basin 2	2.611	0.368	2.979	92.0			
Roadway	2.541		2.541	98.0			
Sidewalk	0.070		0.070	98.0			
Pervious		0.368	0.368	49.0			
Total	11.948	4.452	16.400				

1.37

Preferred Alternative

	Preferre	a Alterna	live		
	Impervious	Pervious	Total	Curve Number	
Basin 1	12.168	1.254	13.422	93.4	
Roadway	10.581		10.581	98.0	
Sidewalk	1.587		1.587	98.0	
Pervious		1.254	1.254	49.0	
Basin 2	2.740	0.238	2.979	94.1	
Roadway	2.620		2.620	98.0	
Sidewalk	0.120		0.120	98.0	
Pervious		0.238	0.238	49.0	
Total	14.908	1.493	16.400		

Required Treatment

Basin 1	
New Impervious Area (non-exempt)	1.667
Post Development Roadway minus Predevelopment Roadway	
Proposed Treatment Type	Wet Detention
Required Treatment Volume	0.14
1 inch over the new impervious area	
Basin 2	
New Impervious Area (non-exempt)	0.080
Post Development Roadway minus Predevelopment Roadway	
Treatment Type	Wet Detention
Required Treatment Volume	0.01
1 inch over the new impervious area	

Basin Evaluation Required Attenuation

Basin 1

Total New Impervious Area Hydrologic Soil Group Open Area Curve Number Weighted Curve Number 25 yr / 24hr rainfall

Pre	Post	_
13.422	13.422	acres
Α		
49		
83.1	93.4]
7.91]in
	Pre 13.422 A 49 83.1 7.91	Pre Post 13.422 13.422 A

	TR-55						
S =	2.04	0.70	in				
=	0.41	0.14	in				
Q =	5.90	7.12	in				
	6.60	7.97	ac-ft				
		1.37	ac-ft				

Total Runnoff Required Attenuation (Post minus Pre Runnoff)

Basin 2

Total New Impervious Area Hydrologic Soil Group Open Area Curve Number Weighted Curve Number 25 yr / 24hr rainfall

	Pre		
A =	2.979	2.979	acres
	A]
	49]
	92.0	94.1	ac-ft
P =	7.91		in
			-
	TI	R-55	
S =	0.87	0.63	in

	TI	TR-55					
S =	0.87	0.63	in				
=	0.17	0.13	in				
2 =	6.95	7.20	in				
	1.72	1.79	ac-ft				
		0.06	ac-ft				

Total Runnoff Required Attenuation (Post minus Pre Runnoff)

Stormwater Managem	nent Facility Alte	ernatives	
SMF 1A			
Project Gibsonton Drive	By:	EKN	Date: 7/13/2023
FPID NO: 254552 1 22 21	Checked:	WLA	Date: 7/14/2023
Water Quality			
Required Treatment Volume		0.1 0.0	Wet Detention 77 Dry Retention
Water Quantity			
Required Attenuation		1.3	37
Total Required Pond Volume (Estimate 10% additional)		1.6	Wet Detention 58 Dry Retention

Treatment Type

Wet Detention

Stage Areas						
Stage	Area	Area	Inc. Volume	Total Volume	Description	Domorko
ft-NAVD	ft²	acres	ac-ft	ac-ft		Remarks
23.50	77,747	1.78	0.00	0.00	Pond Bottom	
25.00	81,196	1.86	2.74	2.74	SHWT	Per ERP Permit 21779.009
25.07	81,427	1.87	0.14	2.88	Control	
26.40	85,657	1.97	2.54	5.42	DHW	
27.00	88,107	2.02			Top of Bank	

Parcel Area

75,260 *ft*² 1.73 acres

v

,

	Stormwater Management Facility Alternatives							
SMF 1B								
Project Gibsonton Driv	e	By:	EKN	Date:	7/13/2023			
FPID NO: 254552 1 22 21	L	Checked:	WLA	Date:	7/14/2023			
Water Quality				-				
Required Treatment Volume			0.14	Wet De	tention			
			0.07	Dry Rete	ention			
Water Quantity				_				
Required Attenuation			1.37	J				
Total Required Pond Volume	(Estimate 10% additional)		1.66	Wet De	tention			
			1.58	Dry Rete	ention			

Treatment Type

Wet Detention

Stage Areas						
Stage	Area	Area	Inc. Volume	Total Volume	Description	Domorko
ft-NAVD	ft²	acres	ac-ft	ac-ft		Remarks
22.85	16,187	0.37	0.00	0.00	Pond Bottom	
24.35	19,854	0.46	0.40	0.40	Littoral Zone	
25.85	23,749	0.55	0.75	1.15	SHWT	Per ERP Permit 21779.009 Wetland "D"
26.10	24,359	0.56	0.14	1.29	Control	
28.85	32,215	0.74	1.78	3.08	DHW	
29.85	35,238	0.81			Inside Top of Bank	
30.85	47,471	1.09			Outside Top of Bank	

Parcel Area

 75,260
 ft²

 1.73
 acres

v

	Stormwater Mar	nagement Facil	ity Alterna	atives	
SMF 1C					
Project Gibsonton Drive	e	Ву:	EKN	Date:	7/13/2023
FPID NO: 254552 1 22 21		Checked:	WLA	Date:	7/14/2023
Water Quality Required Treatment Volume			0.14	ו	Wet Detention
			0.07]	Dry Retention
Water Quantity					
Required Attenuation			1.37]	
Total Required Pond Volume	(Estimate 10% additional)		1.66 1.58]	Wet Detention Dry Retention

Treatment Type

Wet Detention

Stage Areas						
Stage	Area	Area	Inc. Volume	Total Volume	Description	Barranta
ft-NAVD	ft²	acres	ac-ft	ac-ft		Remarks
31.00	35,731	0.82	0.00	0.00	Pond Bottom	
32.50	40,233	0.92	0.85	0.85	Littoral Zone	
34.00	35,731	0.82	1.31	2.16	SHWT	Assumed 12" below Lowest Grade of 35'
34.17	36,529	0.84	0.14	2.30	Control	
36.00	46,589	1.07	1.75	4.05	DHW	
37.00	49,918	1.15			Inside Top of Bank	
38.00	63,295	1.45			Outside Top of Bank	·

Parcel Area

 75,260
 ft²

 1.73
 acres

Y₂

Stormwater Management Facility Alternatives												
SMF 2B												
Project	Gibsonton Drive	е	By:	EKN	Date:	7/13/2023						
FPID NO:	254552 1 22 21		Checked:	WLA	Date:	7/14/2023						
Water Quality												
Required Treatn	Required Treatment Volume 0.01 Wet Detention											
				0.00	Dry Retent	ion						
water Quantity	Water Quantity											
Required Attenu	Required Attenuation 0.06											
Total Required	Pond Volume	(Estimate 10% a	additional)	0.08	Wet Deten	tion						
iotal nequileu i		(Estimate 10/0 t	additionaly	0.07	Drv Retent	ion						
				0.07	Diff Heteric							
Treatment Type		Wet Detent	ion									
Stage Areas						· · · · · · · · · · · · · · · · · · ·						
Stage	Area	Area	Volume	Des	cription	Remarks						
ft-NAVD	ft²	acres	ac-ft			inclinal k5						
10.10				S	SHWT							
18.40	66,910	1.54	0.00	Permi	itted DHW							
18.44	66,996	1.54	0.06	Propo	osed DHW							

4.16

Inside Top of Bank

21.00

72,610

1.67

Floodplain Encroachments

Area No.	Area Area		Avg. Depth of	Approximate Volume	Approximate Volume
	SF	ас	ft	СҮ	ac-ft
1	11,077.00	0.254	0.560	230	0.142
2	1,170.00	0.027	0.270	12	0.007
3	8,735.00	0.201	0.380	123	0.076
4	8,909.00	0.205	0.080	26	0.016
		Total		391	0.242

SHWT BFE 25.85 ft-NAVD 28.42 ft-NAVD

BMPTrains Inpu	t		
Catchment 1			
Total Pre-Development Catchment Area		13.42	ac
Total Post-Development Catchment Area		13.42	ac
Pre-Development Non-DCIA CN		49	
Pre-Development DCIA Percentage		69.57	%
Post-Development Non-DCIA CN		49	
Post-Development DCIA Percentage		90.66	%
Wet Pond Area (No Loading)			ac
	SFM 1A	1.869	ac
	SFM 1B	0.559	ac
	SFM 1C	0.839	ас
SME 1A			
Permanent Pool Volume		2.74	ac-ft
Littoral Zones Improvement Credit		10	%
SMF 1B			
Permanent Pool Volume		1.15	ac-ft
Littoral Zones Improvement Credit		10	%
·			
SMF 1C			
Permanent Pool Volume		2.16	ac-ft
Littoral Zones Improvement Credit		10	%

APPENDIX D

ENGINEER'S ESTIMATE OF CONSTRUCTION COST

Engineer's Estimate of Construction Cost

Earthwork							
	Cut	Fill	Unit Co	st			
SMF/FPC #	су	су	0120 1	0120 6	Linear Ft of 30" Pipe	430175130	Total
SMF 1A	11,636	4	7.95	16.26	750.00	197.97	\$ 241,048.7
SMF 1B	8,506	376	7.95	16.26		197.97	\$ 73,736.5
SMF 1C	9,795	497	7.95	16.26	500.00	197.97	\$ 184,936.5
FPC 1A	4,630	0	7.95	16.26		197.97	\$ 36,808.5

Pipe Size				
	SMF 1A	SMF 1B	SMF 1C	
Runniff Coef	0.98	0.98	0.98	
Rainfall Itensity	6.38	6.38	6.38	in, 10 yr / 24hr
Area	1.67	1.67	1.67	ас
Flow	10.42	10.42	10.42	cfs
Pipe Length	750		500	ft
Pipe Drop	1.00	No Additional	0.38	ft
Slope	0.13%	Conveyance	0.08%	ft/ft
Pipe Size	30	Required	30	in
Capacity	16.27		12.27	cfs

n, 10 yr / 24hr rainfall (Hillsoborough County Requirement)

APPENDIX E

K.

SUPPORTING DOCUMENTATION

	FLORIDA DEPARTMENT OF TRANSPORTATION DISTRICT SEVEN RIGHT OF WAY COST ESTIMATE HDB#: 10310529-4.20										
FM#	450428-1		Alternate:		SMF-1A			District		Seven	
County:	Hillsbord	ugh	Segment:		N/A			Date:		8-Jun-23	
State Rd.:	N/A	BI 5 1111 - 110 cm	FAP#:		N/A			C.E. Sequence		N/A	
Project Des.	Gibsonto	n Rd. Fern Hill to US 30	1 Ponds				Estimated Relo	catees.			
Commercial	0	0					Business	calces.	0		
Residential	4	4					Residential		4		
Unimproved	0	0					Signs		0		
Total Parcels	4	Д					Special Total Relocate	29	4		
B/W SUPPORT CO	STS (PHAS	SE 41)				ע מופיר אין און	Total Holoculo	Amount			and the second second
1. Direct Labor Co	ost	(Parcels	4	х	20,000 =	Rate)	80,000			
2. Indirect Overhe	ead	(Parcels	4	x	0 =	Rate)	0			
3.								TOTAL PHASE 41		International Property	\$80,000
R/W OPS (PHASE	4B)								Amount		
4. Appraisal Fees	s Through	Frial				4	Parcels >	x 30,000 =	120,000		
5. Business Dam	age CPA F	ees Inrougn Iriai s Servers	50%	¥	4 -	2	Parcels x	k 19,000 = k 500 =	1.000		
7. Expert Witnes	is a ribbes.	5 5017015	75%	x	4 =	3	Parcels	x 30,000 =	90,000		
8. Mediators			75%	x	4 =	3	Parcels >	c 2,400 =	7,200		
9. Demolition, As	sb. Abate.,	Survey, etc.				9	Imprvmet >	k 15,000 =	135,000		
10. Miscellaneous	s Contracts	5				1	Per Project x	5 000 -	15,000		
12 Appraisar ree	NEVIEW					1			3,000		\$373,200
PAN LAND COOTO		2)	C the state to pay with the address of the total			an an ann an	The second second second second	Amount	Subtatal		
n/w LAND CUSTS	mente 8 6	oy averance Damagaa						Amount	Subtotal		
and Cost to C	inclus & Si	t	n	Y	120%	• Design	lan stane -				
14 Water Retentio	on & Mit (l Pond)	975 396	Ŷ	120% (0	Parcels w	/ο R/W Δca)	1 170 500			
15. SUBTOTAL (10	15 099 SF)	r r ond,	575,550	Ŷ	12070 (0	(Lines 13	&14)	1,170,000	1,170,500		
16. Admin. Settler	nents	(Factor	20%	х	60% o	f Line 15)	_	= 140,500			
17. Litigation Awa	ards	(Factor	45%	x	40% o	f Line 15)	=	210,700			
18. Business Dam	ages	(Claims	0	х	0)		=	= 0			
19. Bus. Damages	Incrs.	(Factor	25%	x	\$-)			- 0			
20. Owner Appr. F	ees	(Parcels	4	x	\$15,000)		=	60,000			
21. Owner CPA Fe	ees	(Claims	0	х	\$16,000)		=	<u> </u>			
22. Defend.Atty Fe	ees	(Sum of Lines 16, 17 & 19)	351,200	х	33%)		Ŧ	=			
23. Owner Expert	Witness	(Comm.+Unimp.)	0	+	():	x <u>18,000</u>	<u> </u>	=			
24. Other Condem	n. Costs		4	X	\$1,000	(1)	(h	4,000	521 100		
25. SUBTUTAL						(Lines to	uiru 24) =	TOTAL PHASE 43	551,100		\$1 701 600
* Design conting	ency for de	sinn nlan stane						TUTALI HAGE 45			\$1,701,000
(1) PD&E pla	ans - 120%	(2) 30% plans - 115%	(3) 60% plans - 110	% (4)) 90% plans -105%	(5) 268 Dat	te -100%				
R/W ACQUISITION	CONSUL	ANT (PHASE 42)				THE OWNER OF THE OWNER					
27. Acquisition Co	onsultant-50	0% of parcels	\$20,000	×	0			TOTAL PHASE 42			\$0
RELOCATION COS	TS (PHAS	E 45)									
	Replace	ment Housing			Number		Amount				
28. Owner			\$35,000	х	4	=	140,000				
29. Tenant	Maya Ca	ata	\$25,000	х	0	=	0				
30. Residential	INIONE CO	515	\$5,000	¥	4	-	20 000				
31. Business/Farn	n		\$40,000	x		-	0				
32. Personal Prop	erty		\$3,000	x	0	=	0	-			
33. (Lines 28 thru 3	32)							TOTAL PHASE 45			\$160,000
34. Relocation Se	rvices Cos	t			\$16,000	(Not in P	hase Total)				
35.						-					
30.						<u></u>	(All Phases)	TOTAL FORMATE			\$2,314,800
Real Estate:	Reger D	Patton	Signade	12	20HICH		1711 1 110303/	Data	06/15/22		-2,011,000
Bus. Dam. :	Alfred J.	Thompson	Signed:	-	action	Q.T.	a Mar	Date:	06/15/23		
Relocation:	Roger D.	Patton	Signed:	F	aden		- Aller	Date:	06/15/23		
Overall Review:	Alfred J.	Thompson	Signed:		a	.g. TA	ingson	Date:	06/15/23		
0		Detail				0	/		- D.t.		
Cost Estimate Seq	uence #:	Dated:		Int	ne Amount of \$	And the second second	L	Jata Input Completi	on Date:		
REMARKS:		POND. SME-1A									
		TOND. ONIT TA									
The following ind	icates the	estimator's confidence	in the above estim	ate:							
	Type A -	indicates the most con	fidence								
	Туре В -	indicates above avera	ge confidence								
x	_Type C -	indicates below avera	ge confidence								
	_Type D -	indicates the least or i	io confidence								
The fellowing in the	inotes the	Department	for this set				alon of the later Prototo and				
Work Program Un	date:	Department's purpose	Gaming 1.			Special	Purnose.	Y	Docs to RW/		
Comments:				_		Shoolail		~			

	FLORIDA DEPARTMENT OF TRANSPORTATION										
F7.5.4	450400.4		DISTRICTS			AT 603	ESTIMATE	D'	пµп#:	10310529-4.20	
FM#: County:	450438-1 Hillshore	uab	Alternate:		SMF-1B & FPC			District: Date:		Seven 8-Jun-23	
State Rd.:	N/A	ugn	FAP#:		N/A			C.E. Sequence		N/A	
Project Des.	Gibsonto	n Rd. Fern Hill to US 30	1 Ponds								
Parcels	Gross	Net					Estimated Relo	ocatees:	2		
Commercial	0	1					Business		2		
Ilnimnroved	0	<u> </u>					Signs		2		
Chimproved	ĭ						Special		0		
Total Parcels	0	2					Total Relocate	es	4		ur v scatter side
R/W SUPPORT CO	STS (PHAS	SE 41)	5					Amount			
1. Direct Labor Co	ost .	(Parcels	2	х	20,000 =	Rate)	40,000			
2. Indirect Overhe	ead	(Parcels	2	x	=	Rate)				¢40.000
3.					n politika da Spanica da escar da se	all parts for		TOTAL PHASE 41			\$40,000
R/W OPS (PHASE	4B)	1					Dennelle	20.000	Amount		
4. Appraisal rees	S Inrougn	i fiai pos Through Trial				2	Parceis 2	x 30,000 =	60,000 N		
6. Court Reporter	r & Process	s Servers	50%	x	2 =	1	Parcels	x 15,000 =	500		
7. Expert Witnes	S		75%	x	2 =	2	Parcels	x 30,000 =	60,000		
8. Mediators			75%	х	2 =	2	Parcels 2	x 2,400 =	4,800		
9. Demolition, As	sb. Abate.,	Survey, etc.				5	Imprvmet	x 15,000 =	75,000		
10. Miscellaneous	s Contracts	5				1	Per Project >	<pre> 15,000 =</pre>	15,000		
11. Appraisai ree	Review					1	Parceis)		5,000		\$220 300
12.		allet all was drever to the operation in the second				and a start of the second		TUTAL PHASE 46			\$220,300
R/W LAND COSTS	(PHASE 43	3)						Amount	Subtotal		
13. Land, Improve	ments & Se	everance Damages			1000/						
and Cost to Ci	ure Amount		0	x	120%	" Design p	lan stage =	=			
14. Water Retention	01 & Wit. (I	J Ponds)	1,636,291	x	<u> </u>	Parcels w	//o K/W Acq)	1,963,500	1 000 500		
15. SUBIUIAL (14	10,699 SF)	(Footor	200/		C00/ -	(LINES 13	&14)	225 600	1,963,500		
17 Litigation Awa	nents	(Factor	20%	x	0%_0	f Line 15)		- 253,000			
18 Business Dam	anes	(Claims	45 /8	Ŷ	40% 0	i Line 15)		= <u> </u>			
19. Bus, Damages	Incrs	(Factor	25%	x	\$ -)						
20. Owner Appr. F	ees	(Parcels	2	x	\$15,000)			= 30.000			
21. Owner CPA Fe	es	(Claims	0	x	\$16,000)	-	-	= 0			
22. Defend.Atty Fe	es	(Sum of Lines 16, 17 & 19)	589,000	x	33%)		-	= 194,400			
23. Owner Expert	Witness	(Comm.+Unimp.)	1	+	0):	x 18,000	0 =	= 18,000			
24. Other Condem	n. Costs		2	x	\$1,000		-	= 2,000			
25. SUBTOTAL						(Lines 16	thru 24) =	=	833,400		
26.								TOTAL PHASE 43			\$2,796,900
* Design continge	ency for de	sign plan stage:	(2) (20) - 1 (40)	0/ 10	000/	(5) 000 D-	1. 10001				
(I) PD&E pla	ans - 120%	(2) 30% plans - 115% (3) 60% plans - 110	% (4)	90% plans -105%	(5) 268 Dat	te -100%	annen sider commenter et besid	and the second of the second		
R/W ACQUISITION	CONSULT	ANT (PHASE 42)	000 000					TOTAL DUADE 40			<u></u>
27. Acquisition Co	onsultant-50	1% of parcels	\$20,000	X	U			TUTAL PHASE 42			\$0
RELOCATION COS	TS (PHASE	45)									
28 Owner	Keplacer	nent Housing	\$25,000	v	Number	_	Amount				
29. Tenant			\$25,000	×		-	50,000				
	Move Co	sts	\$23,000	^		-					
30. Residential			\$5,000	x	2	=	10,000				
31. Business/Farm	n		\$40,000	x	2		80,000				
32. Personal Prop	erty		\$3,000	×	0	=	0				
33. (Lines 28 thru 3	32)							TOTAL PHASE 45			\$140,000
34. Relocation Se	rvices Cos			-	\$14,000	(Not in P	hase lotal)				
35. 36											
37.							(All Phases)	TOTAL ESTIMATE		2	\$3,197 200
Real Estate	Roger D	Patton	Signad	17		and a construction of	,/ di 1 nuoco/	Bata	06/15/22		,,
Bus, Dam. :	Alfred J.	Thompson	Signed:	T	ACTION	127	Kanada	Date:	06/15/23		
Relocation:	Roger D.	Patton	Signed:	TS	DATON 1		nonpsin	Date:	06/15/23		
Overall Review:	Alfred J.	Thompson	Signed:		20-11-	2. V. T.	hongson	Date:	06/15/23		
						0					
Lost Estimate Seq	uence #:	Dated:		In t	he Amount of \$		I	Jata Input Completi	on Date:		
REMARKS:											
		PUND: SIVIF-IB									
	n o to an ann an ta			a the second			and the part of the second of the second		A MARKET MARKET CARTER OF A MARKET AND A DATA		ur anarostation
The tollowing indi	icates the	estimator's confidence	in the above estim	ate:							
	Type A -	indicates the most cont indicates above average	iuence e confidence								
x	Type C - i	indicates below average	e confidence								
	Type D -	indicates the least or n	o confidence								
The following indi	icates the l	Department's purpose f	or this estimate:								
Work Program Up	date:		Gaming 1:	_		Special I	Purpose: _	x	Docs to RW:	<u>.</u>	
Comments:											

			FLORIDA I	DEP/	ARTMENT OF	TRANSP	ORTATION T ESTIMATE		HDR#·	10310529-4 20	
EM#	450438-1		Alternate:	the W lies	SMF-1C			District:		Seven	
County:	Hillsborough		Segment:		N/A			Date:		8-Jun-23	
State Rd.:	N/A		FAP#:		N/A			C.E. Sequence		N/A	
Project Des.	Gibsonton Rd. I	Fern Hill to US 301	Ponds				Estimated Pala	actors:			
Commercial	Gross Net	0					Business	icalees.	5		
Residential	7	8					Residential		7		
Unimproved	0	0					Signs		0		
Total Parcole	7	0					Special Total Relocate	95	0		
R/M SUPPORT CO	STS (DUASE A1)	0		1		State of the second	Total nelocate	Amount			
1. Direct Labor Co	ost	(Parcels	8	x	20,000 =	= Rate	:)	160,000			
2. Indirect Overhe	ead	(Parcels	8	х	0 =	Rate	.)	0			
3.			a participante a construinte a constru	Citati- Inde		Non-Address To	and a final second state for any state such	TOTAL PHASE 41			\$160,000
R/W OPS (PHASE	4B)						_		Amount		
4. Appraisal Fees	s Through Trial	warrah Trial				8	Parcels 2	x 30,000 =	240,000		
5. Business Dam	age UPA rees In	rougn Triai ers	50%	v	8 -	- 4	Parcels 3	x 19,000 = x 500 =	2 000		
7. Expert Witnes	S	615	75%	x	8 =		Parcels 2	x 30,000 =	180,000		
8. Mediators			75%	x	8 =	= 6	Parcels 2	x 2,400 =	14,400		
9. Demolition, As	sb. Abate., Survey	y, etc.				13	Imprvmet 2	x 15,000 =	195,000		
10. Miscellaneous	s Contracts Review					1	Per Project >	c 15,000 =	15,000		
12.	neview					J		TOTAL PHASE 4B	10,000		\$661,400
B/W LAND COSTS	(PHASE 43)			i i anna a' chuir ann a' chuir a	. All and a second second second second			Amount	Subtotal		Contraction of the second
13. Land. Improve	ments & Severan	ice Damages						Anothe	Sustatui		
and Cost to Cu	re Amount		-20,000	x	120%	* Design p	lan stage =	-24,000			
14. Water Retention	on & Mit. (1 Pond	I)	1,669,392	x	120% (0 Parcels w	/o R/W Acq)	2,003,300			
15. SUBTOTAL (71	,098 SF)					(Lines 13	&14)		1,979,300		
16. Admin. Settler	nents (Facto	r	20%	х	60% 0	of Line 15)		= 237,500			
17. Litigation Awa	ards (Facto	r	45%	x	40% c	of Line 15)	=	= 356,300			
18. Business Dam	ages (Claim	IS	0	х	0)		-	=			
19. Bus. Damages	Incrs. (Facto	r	25%	x	<u>\$</u> -)		÷	= 0			
20. Owner Appr. F	ees (Parce		8	x	\$15,000)			120,000			
22 Defend Atty Fe	es (Grann	lines 16 17 & 19)	593 800	×	33%)			- 196,000			
23. Owner Expert	Witness (Comn	n.+Unimp.)	0	+	0)	x 18.00	0 =	= 0			
24. Other Condem	n. Costs	and a state of the second	8	x	\$1,000			= 8,000			
25. SUBTOTAL						(Lines 16	i thru 24) 🛛 =		917,800		
26.								TOTAL PHASE 43		}	\$2,897,100
* Design continge (1) PD&F nl:	ency for design p ans - 120% (2) 30	lan stage: 1% nlans - 115% /3	2) 60% nlans - 110	% (1)	90% nlans -105%	(5) 268 Da	te -100%				
	CONSULTANT (,	10 11	50 % prano 100 %	10/ 200 0 20	100 100 10				an a
27. Acquisition Co	insultant-50% of n	arcels	\$20,000	x	n			TOTAL PHASE 42			\$0
BELOCATION COS	TS (PHASE 45)		,10,000						a see ingelandrige worden gehade		
	Replacement H	lousing			Number		Amount				
28. Owner			\$35,000	х	2	=	70,000				
29. Tenant	Maya Caata		\$25,000	x	5	=	125,000				
30. Residential	wove costs		\$5,000	x	7	=	35,000				
31. Business/Farm	1		\$40,000	x	5	-	200,000				
32. Personal Prop	erty		\$3,000	x	0	=	0	-			
33. (Lines 28 thru 3	32)				Bacterio, 2019/201	42900 000 000		TOTAL PHASE 45			\$430,000
34. Relocation Se	rvices Cost				\$43,000	(Not in P	hase Total)		- And a book of the strength of the		
35. 36											
37.							(All Phases)	TOTAL ESTIMATE	_		\$4,148,500
Real Estate:	Roger D.Patton		Signed:	P	Clathon _	ander, och förstand stora för		Date:	06/15/23		
Bus. Dam. :	Alfred J. Thom	pson	Signed:	1 -04	1 CHICK	L. V. J.	hompson	Date:	06/15/23		
Relocation:	Roger D.Patton		Signed:	P	2PHUL-	0	1	Date:	06/15/23		
Overall Review:	Alfred J. Thom	pson	_Signed:		a	J. Jh	mpm	_ Date:	06/15/23		
Cost Estimate Seg	uence #:	Dated:		In th	ne Amount of \$	Q	′	Data Input Completi	on Date:		
REMARKS:			and a second			ala kitika perilan				an ann an	
	POND	: SMF -1C									
	and a second second								A second with the second second	an a	
The following indi	cates the estima	tor's confidence i	n the above estimation	ate:							
	Type A - Indica	ites above average	acince confidence								
x	Type C - indica	tes below average	confidence								
	Type D - indica	tes the least or no	confidence								
								and a second state and a second second second second			nt, in all they doe until
The following indi	icates the Depart	tment's purpose fo	r this estimate:			Succial I	Durnesse	~	Doos to DM		
Comments:	udic		_ daming 1:			special	r urpose:	X	_0005 to KW:	-	
								14			

			FLORIDA I DISTRICT S	DEPA EVEN	RTMENT OF T I RIGHT OF W	RANSP(AY COST	ORTATION ESTIMATE		HDR#:	10310529-4.20	
FM#:	450438-1		Alternate:		FPC			District:		Seven	
County:	Hillsborough		Segment:		N/A			Date:		8-Jun-23	
State Ro.: Proiect Des.	N/A Gibsonton Rd	. Fern Hill to US 301	FAP#: Ponds		N/A			c.e. Sequence		N/A	
Parcels	Gross N	et					Estimated Relo	ocatees:			
Commercial	1	<u>1</u>					Business		1		
Unimproved	0	<u> </u>					Signs		0		
							Special		0		
Total Parcels	1	1					Total Relocate	es	2		
R/W SUPPORT CO	STS (PHASE 41) (Parcels	1	v	20 000 -	Rate		Amount 20 000			
2. Indirect Overhe	ad	(Parcels	<u> </u>	× X	0 =	Rate)		0			
3.				-				TOTAL PHASE 41		\$	\$20,000
R/W OPS (PHASE 4	4B)								Amount		
4. Appraisal Fees	Through Trial	Through Trial				1	Parcels	x 30,000 =	30,000		
5. Business Dama	aye CPA rees & Process Sei	rvers	50%	x	1 =	0 1	Parcels	k = 19,000 = 100	U 500		
7. Expert Witness	S 1100000 001		<u>75%</u>	x	<u> </u>	1	Parcels 2	x 30,000 =	30,000		
8. Mediators			75%	X	1 =	1	Parcels 2	x 2,400 =	2,400		
9. Demolition, As	b. Abate., Surv Contracts	ey, etc.				2 1	Imprvmet 2 Per Project 2	x = 15,000 = 15,000 = 15,000 = 1000 = 1000 = 10000 = 10000 = 100000000	30,000 15 000		
11. Appraisal Fee	Review					0	Parcels x	5,000 =	0		
12.								TOTAL PHASE 4B		\$1	107,900
R/W LAND COSTS	(PHASE 43)							Amount	Subtotal		
13. Land, Improven	nents & Severa	ance Damages									
and Cost to Cu	re Amount		0	x	120% *	Design p	lan stage =	=0			
14. Water Retentio	on & Mit. (0 Poi	nds)	805,899	<u>х</u>	<u>120%</u> (0	Parcels w	/o R/W Acq)	967,100			
15. SUBTOTAL (14)	0,699 SF) conto (Ecci	tor	200/		C00/ of	(Lines 13)	&14)	116 100	967,100		
10. Aumin. Section 17. Litigation Awa	rds (Fac	iur tor	20% Δ5%	X. X	<u> </u>	Line 15)		- 174 100			
18. Business Dama	ages (Clai	ims	0	х. Х	<u> </u>	Line 13/		= 0			
19. Bus. Damages	Incrs. (Fac	tor	25%	x	\$ -)		=	0			
20. Owner Appr. Fe	ees (Par	cels	1	x	\$15,000)		=	= 15,000			
21. Owner CPA Fee	es (Clai	ims	0	x	<u>\$16,000</u>)		=	=0			
22. Defend.Atty Fe	es (Sum	of Lines 16, 17 & 19)	290,200	X	33%)	10.000	=	<u> </u>			
23. Uwner Expert V	Witness (Con 2 Costs	nm.+Unimp.)	1	+	<u> </u>	(<u>18,000</u>		= <u>18,000</u>			
24. Other Condenii 25. SUBTOTAL	1. 00313		I	Χ.	\$1,000	(Lines 16	= thru 24) =	= <u>1,000</u>	420.000		
26 .								TOTAL PHASE 43		\$1,3	387,100
* Design continge	ency for design	plan stage:		•							
(1) PD&E pla	ns - 120% (2) .	30% plans - 115% (3,) 60% plans - 110%	% (4) :	90% plans -105%	(5) 268 Dat	e -100%				
R/W ACQUISITION	CONSULTAN	「(PHASE 42)									<u> </u>
27. Acquisition Co	nsultant-50% of	f parcels	\$20,000	X	0			TOTAL PHASE 42			\$ 0
RELOCATION COST	TS (PHASE 45) Benlacement	Housing			Number		Amount				
28. Owner	πεμιασειπειπ	nousing	\$35,000	x		=	Amount 0				
29. Tenant			\$25,000	x	1	=	25,000				
	Move Costs		¢F 000				F 000				
30. Residential 31. Rusiness/Farm			\$5,000	X X	1	=	<u> </u>				
32. Personal Prope	erty		\$3,000	X	<u>.</u> 0	=	0				
33. (Lines 28 thru 3	32)			· -				TOTAL PHASE 45		\$	\$70,000
34. Relocation Ser	rvices Cost				\$7,000	(Not in Pl	nase Total)				
35. 26											
30. 37.							(All Phases)	TOTAL ESTIMATE		\$1 5	585.000
Real Estate:	Roger D.Patto	n	Signed:	T			(Date:	07/19/23	+-/-	
Bus. Dam. :	Alfred J. Tho	mpson	_Signed:	ag	r. Thom	poon		Date:	07/19/23		
Relocation:	Roger D.Patto	on	Signed:			Der		Date:	07/19/23		
Overall Review:	Alfred J. Tho	mpson	_Signed:	a.g	. shory	- on	-	Date:	07/19/23		
Cost Estimate Sequ	uence #:	Dated:		In the	e Amount of \$		[Data Input Completio	on Date:		
REMARKS:											
	PON	D: FPC									
The following indi	aataa tha aatin	otov's confidence in	the chave estimation								
Ine following indi	cates the estin Type Δ - indic	nator's confidence in cates the most confid	i the above estima lence	ate:							
	Type B - indic	cates above average	confidence								
X	Type C - indic	cates below average	confidence								
	_Type D - indic	cates the least or no	confidence								
The following indi	nation the Dam-	rtmont's surress for	this potimotor								
Work Program Und	vates tile Depa late:	n anena s purpose toi	Gamina 1:			Special P	Purpose:	x	Docs to RW:		
Comments:											
1				_					_		

Permit 2166.001 (App 6220) Boyette Rd.-US Hwy. 301 to Balm Riverview Rd



		REVISIONS
DA	TE #	DESCRIPTION
	-	
-		1.11
	~	





Permit 21779.009 (App ID 45811)_Lowes Riverview Town Centre

FACILITIES SHOWN HEREON HAVE BEEN, TO THE BEST OF MY KNOWLEDGE, CONSTRUCTED IN SUBSTANTIAL ACCORDANCE WITH APPROVED PLANS, SPECIFICATIONS AND MODIFICATIONS FOR THIS PROJECT. VERIFICATION IS BASED ON PERIODIC CONSTRUCTION OBSERVATION AND SURVEY NOTATIONS SHOWN ON PLANS. SURVEYED DIMENSIONS AND ELEVATIONS SHOWN AS RECORD INFORMATION HAVE BEEN FIELD VERIFIED.

RECORD DRAWING LEGEND & NOTES CLOUDED AREAS HAVE RECORD INFORMATION

PROPOSED/DESIGN ELEVATION OR DIMENSION LINED THROUGH, 547-"RECORD" ITEM, ELEVATION OR DIMENSION WRITTEN IN. FIRE HYDRANT 140.5' ITEMS THAT ARE PER PLAN ARE SHOWN BY A CHECK MARK THUSLY: 10" VALVE / LINES NOT INSTALLED PER PLAN LOCATION ARE SHOWN THUSLY: -/--/-/-/

RECORD INFORMATION PROVIDED BY LEFTCOAST SURVEYORS, INC. 2363 1st Avenue North St. Petersburg, Florida 33713 AND David Nelson Construction Co. 3483 Alternate 19 Palm Harbor, Florida 34683



\$ 63.

why down it -

SITE CONSTRUCTION PLANS

FOR

LOWE'S RIVERVIEW "RECO I FF-S I NITU TOWNCENTER

10425 GIBSONTON DRIVE

RIVERVIEW, FLORIDA 33569

PREPARED FOR

LOWE'S HOME CENTERS, INC. P.O. Box 1111 N. Wilkesboro, NC 28656

			THE COMPLETE CON
UTILITIE	LOWE'S RIVÉRVIEW		
DWNER	CONTACT PERSON	TELEPHONE	FOLLOWING PLAN SH
BOROUGH COUNTY WATER DEPT.	MARCEL DIAZ	813-272-5081	
BAY WATER	RICK MENZIES	813-740-4037	1 CITE CONCTDUCT
PEOPLES GAS	LUIS CASTELLANOS	813-275-3743	1.511E CONSTRUCT
ON COMMUNICATIONS	TOM FAULKNER	813-989-7911	2. ROADWAY IMPROV
WARNER COMMUNICATIONS	BARRY BEATTY	813-684-6100×325	
ELECTRIC COMPANY	ARLENE BROWN	813-228-4674	
AL FLORIDA PIPELINE CORP.	JERRY COLEMAN	813-241-1104	
	JERRY EUGENIO	813-230-2191	





Permit 45227.000 (App ID 815372) Gibsonton Dr. at Fern Hill Dr



LOCATION MAP SECTIONS 19 TOWNSHIP 305, RANGE 20E

	LENGTH OF PROJECT		
DESCRIPTION	LINEAR FT.	MILES	
ROADWAY	1448.8	0.274	
BRIDGES	0.0	0.000	
NET LENGTH OF PROJECT	1448.8	0.274	
EXCEPTIONS	0.0	0.000	
GROSS LENGTH OF PROJECT	1448.8	0.274	

COUNTY PROJECT MANAGER: SANDRA GONZÁLEZ, P.E.

GOVERNING STANDARD PLANS:

DATE

BY

Florida Department of Transportation, FY 2021-22 Standard Plans for Road and Bridge Construction and applicable Interim Revisions (IRs).

Standard Plans for Road Construction and associated IRs are available at the following website: http://www.fdot.gov/design/standardplans

GOVERNING STANDARD SPECIFICATIONS:

Hillsborough County Public Works Standard Specifications for Construction October 2017 and FDOT Standard Specifications for Roadway and Bridge Construction, Divisions II & 111 as directed under the Hillsborough County Standard Specifications for Construction.

REVISIONS

DESCRIPTION	

CONTRACT NO .: BPCW12000245 VENDOR NO.: 56-2565488

HEATHER A. ROCHA, P.E. ENGINEER OF RECORD: P.E. NO.: 80378

NOTE: THE SCALE OF THESE PLANS MAY HAVE CHANGED DUE TO REPRODUCTION

FERN HILL DR.



9F





APPENDIX F

Meeting Minutes

 THIS FORM IS INTENDED TO FACILITATE AND GUIDE THE DIALOGUE DURING A PRE-APPLICATION MEETING BY PROVIDING A PARTIAL

 "PROMPT LIST" OF DISCUSSION SUBJECTS. IT IS NOT A LIST OF REQUIREMENTS FOR SUBMITTAL BY THE APPLICANT.

 SOUTHWEST FLORIDA WATER MANAGEMENT DISTRICT

 RESOURCE REGULATION DIVISION

 PRE-APPLICATION MEETING NOTES

 PA 410396

Date:	05/08/2023				
Time:	09:00				
Project Name:	Gibsonton Drive Project Development and Environment (PD&E) Study				
District Engineer:	Rob McDaniel, Julio Herrera				
District ES:	Al Gagne				
Attendees:	Tom Daniel, Eric Nelson, Cameron Jones				
County:	Hillsborough	Sec/Twp/Rge:	19, 20/30/20		
Total Land Acreage:	1.0 mile	Project Acreage:	1.0 mile		

Prior On-Site/Off-Site Permit Activity:

 Permit No 2166.001 issued for the east side of US 301; Permit No. 45227.000 issued for the west side near I-75. Permit No. 9622.000 for a force main. ETDM # - 14493

Project Overview:

 1.0 mile long FDOT PD&E Study along Gibsonton Drive from Fern Hill Drive to US 301 within Hillsborough County. Widening this section of Gibsonton Drive from a 4-lane divided facility to a 6-lane divided facility and includes pedestrian and bicycle accommodations. A stormwater management system is proposed.

Environmental Discussion: (Wetlands On-Site, Wetlands on Adjacent Properties, Delineation, T&E species, Easements, Drawdown Issues, Setbacks, Justification, Elimination/Reduction, Permanent/Temporary Impacts, Secondary and Cumulative Impacts, Mitigation Options, SHWL, Upland Habitats, Site Visit, etc.)

- There are wetlands/surface waters located within the project area. There are both upland and wetland cut ditches along with two wetland areas that may be impacted.
- Provide the limits of jurisdictional wetlands and surface waters. Roadside ditches or other water conveyances, including permitted and constructed water conveyance features, can be claimed as surface waters per Chapter 62-340 F.A.C. if they do not meet the definition of a swale as stated under Rule 403.803 (14) F.S.
- Demonstrate elimination and reduction of wetland impacts. The elimination and reduction criteria can be found in subsection 10.2.1 of Applicant's Handbook Volume 1. Be advised that the use of subsection 10.2.1.2 (a) of the handbook may put the project in conflict with the state's 404 program. Coordination with the DEP, the during application review process, is recommended if the applicant wishes to use subsection 10.2.1.2 (a).
- Maintain minimum 15 foot, average 25 foot wetland conservation area setback or address secondary impacts.
- Provide appropriate mitigation using UMAM for impacts.
- The site is located in the Alafia River ERP Basin. Mitigation Banks that serve this area include the Alafia River Mitigation Bank and the Alafia River Wetland Mitigation Bank. For an interactive map of permitted mitigation banks and their service areas, use this <u>LINK</u>. Be advised that use of a bank with a modified service area (i.e. a service area that is larger than the basin the bank is located in), may require the submittal of a cumulative impact analysis pursuant to subsection 10.2.8 of Applicant's Handbook volume 1.
- If the wetland mitigation is appropriate and the applicant is proposing to utilize mitigation bank credit as wetland mitigation, provide a letter of reservation of credits from the wetland mitigation bank. The wetland mitigation bank current credit ledgers can be found out the following link: <u>https://www.swfwmd.state.fl.us/business/epermitting/environmental-resource-permit</u>, Go to "ERP Mitigation Bank Wetland Credit Ledgers"
- Please note, the Florida Department of Environmental Protection (FDEP) has assumed the Federal dredge and fill permitting program under section 404 of the Federal Clean Water Act within certain waters. State 404 Program streamlining intentions direct Agency staff to coordinate joint site visits for overall consistency between the two State programs. As such, District staff and the FDEP will need to conduct a joint site visit for evaluation of the wetland/surface water systems proposed for impact. District staff will coordinate with FDEP staff on determining dates/times of joint Agency availability. Upon determination of joint availability,

staff will provide the applicant's representative with site visit scheduling options. A site visit will not be scheduled until the appropriate signatures on the application and the fee is submitted.

Site Information Discussion: (SHW Levels, Floodplain, Tailwater Conditions, Adjacent Off-Site Contributing Sources, Receiving Waterbody, etc.)

- <u>WBIDs need to be independently verified by the consultant</u> WBID 1621G Alafia River Above Hillsborough Bay. There is a BMAP for nutrients and dissolved oxygen.
- Net improvement is required.
- Document/justify SHWE's at pond locations, wetlands, and OSWs.
- Determine normal pool elevations of wetlands.
- Determine 'pop-off' locations and elevations of wetlands.
- Provide documentation to support tailwater conditions for quality and quantity design.
- Contamination issues need to be resolved with the FDEP. Check FDEP MapDirect layer for possible contamination points within/adjacent to the project area. Multiple markers shown near the intersection of US 310 according <u>FDEP MapDirect Link</u>

FDEP Contacts:

- For projects located within Citrus, Hernando, Pasco, Hillsborough, Pinellas, Manatee, Polk and Hardee Counties: Phil Wilkerson Philip.Wilkerson@floridadep.gov

- Stormwater retention and detention systems are classified as moderate sanitary hazards with respect to public and private drinking water wells. Stormwater treatment facilities shall not be constructed within 100 feet of an existing public water supply well and shall not be constructed within 75 feet of an existing private drinking water well. Subsection 4.2, A.H.V.II.
- District GIS identifies multiple Well Construction Permits (WCP) along the corridor. These may not be mapped correctly.
- Any wells on site should be identified and their future use/abandonment must be designated.

Water Quantity Discussions: (Basin Description, Storm Event, Pre/Post Volume, Pre/Post Discharge, etc.)

- Demonstrate that post development peak discharges from proposed project area will not cause an adverse impact for a 25-year, 24-hour storm event.
- Demonstrate that site will not impede the conveyance of contributing off-site flows.
- Demonstrate that the project will not increase flood stages up- or down-stream of the project area(s).
- Delineate the area and quantify the volume of any fill placement within the floodplain.
- Alafia River Watershed Model (2020) information may be available for download using the following link: <u>https://watermatters.sharefile.com/d-s8c9019e00fd243908654e733a6b2016c.</u> The county may have a more recent version.
- Provide equivalent compensating storage for all 100-year, 24-hour riverine floodplain impacts if applicable. Providing cup-for-cup storage in dedicated areas of excavation is the preferred method of compensation- if no impacts to flood conveyance are proposed and storage impacts and compensation occur within the same basin. In this case, tabulations should be provided at 0.5-foot increments to demonstrate encroachment and compensation occur at the same levels. Otherwise, storage modeling will be required to demonstrate no increase in flood stages will occur on off-site properties, using the mean annual, 10-year, 25-year, and 100year storm events for the pre- and post-development conditions.
- Please be aware that if there is credible historical evidence of past flooding or the physical capacity of the downstream conveyance or receiving waters indicates that the conditions for issuance will not be met without consideration of storm events of different frequency or duration, applicants shall be required to provide additional analyses using storm events of different duration or frequency than the 25-year 24-hour storm event, or to adjust the volume, rate or timing of discharges. [Section 3.0 Applicant's Handbook Volume II]

Water Quality Discussions: (Type of Treatment, Technical Characteristics, Non-presumptive Alternatives, etc.)

- Provide water quality treatment for entire project area and all contributing off-site flows.
- The project discharges to an impaired water body, must provide a net environmental improvement.
- Applicant must demonstrate a net improvement for the parameters of concern by performing a pre/post pollutant loading analysis based on existing land use and the proposed land use.
- Also, replace treatment function of existing ditches to be filled.
- Presumptive Water Quality Treatment for Alterations to Existing Public Roadway Projects:
 -Refer to Section 4.5 A.H.V.II for Alterations to Existing Public Roadway Projects.
 -Refer to Sections 4.8, 4.8.1 and 4.8.2 A.H.V.II for Compensating Stormwater Treatment, Overtreatment, and Offsite Compensation.

<u>Net improvement</u>

 Refer to Rule 62-330.301(2), F.A.C.
 The application must demonstrate a net improvement for nutrients. Applicant may demonstrate a net improvement for the parameters of concern by performing a pre/post pollutant loading analysis based on existing land use and the proposed land use. Refer to ERP Applicant's Handbook Vol. II Subsection 4.1(g).
 Effluent filtration is known to be ineffective for treating nutrient related impairments, unless special nutrient adsorption media is provided. However, please note special nutrient adsorption media has extremely low conductivity values compared to typical sand type effluent filtration filter media. Note: if treatment volume required for net improvement is less than the treatment volume required for 'presumptive' treatment, then use of effluent filtration is ok.

Sovereign Lands Discussion: (Determining Location, Correct Form of Authorization, Content of Application, Assessment of Fees, Coordination with FDEP)

• N/A

Operation and Maintenance/Legal Information: (Ownership or Perpetual Control, O&M Entity, O&M Instructions, Homeowner Association Documents, Coastal Zone requirements, etc.)

- The permit must be issued to the entity that owns or controls the property, the county. FDOT may be involved in the western portion.
- Provide evidence of ownership or control by deed, easement, contract for purchase, etc. Evidence of ownership or control must include a legal description. A Property Appraiser summary of the legal description is NOT acceptable.

Application Type and Fee Required:

- SWERP New Individual Sections A, C, and E of the ERP Application.
- Between 10 and 40 acres of project area and between 1 and 3 acres of wetland or surface water impacts -\$2,491.50
- Consult the fee schedule for different thresholds.

Other: (Future Pre-Application Meetings, Fast Track, Submittal Date, Construction Start Date, Required District Permits – WUP, WOD, Well Construction, etc.)

- An application for an individual permit to construct or alter a dam, impoundment, reservoir, or appurtenant work, requires that a notice of receipt of the application must be published in a newspaper within the affected area. Provide documentation that such noticing has been accomplished. Note that the published notices of receipt for an ERP can be in accordance with the language provided in Rule 40D-1.603(10), F.A.C.
- Provide a copy of the legal description (of all applicable parcels within the project area) in one of the following forms:
 - a. Deed with complete Legal Description attachment.
 - b. Plat.
 - c. Boundary survey of the property(ies) with a sketch.
- The plans and drainage report submitted electronically must include the appropriate information required under Rules 61G15-23.005 and 61G15-23.004 (Digital), F.A.C. The following text is required by the Florida Board of Professional Engineers (FBPE) to meet this requirement when a digitally created seal is not used and must appear where the signature would normally appear:

ELECTRONIC (Manifest): [NAME] State of Florida, Professional Engineer, License No. [NUMBER] This item has been electronically signed and sealed by [NAME] on the date indicated here using a SHA authentication code. Printed copies of this document are not considered signed and sealed and the SHA authentication code must be verified on any electronic copies

DIGITAL: [NAME] State of Florida, Professional Engineer, License No. [NUMBER]; This item has been digitally signed and sealed by [NAME] on the date indicated here; Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

- Provide soil erosion and sediment control measures for use during construction. Refer to ERP Applicant's Handbook Vol. 1 Part IV Erosion and Sediment Control.
- Demonstrate that excavation of any stormwater ponds does not breach an aquitard (see Subsection 2.1.1, A.H.V.II) such that it would allow for lesser quality water to pass, either way, between the two systems. In those geographical areas of the District where there is not an aquitard present, the depth of the pond(s) shall

not be excavated to within two (2) feet of the underlying limestone which is part of a drinking water aquifer. [Refer to Subsection 5.4.1(b), A.H.V.II]

On December 17, 2020, the Environmental Protection Agency (EPA) formally transferred permitting authority under CWA Section 404 from the U.S. Army Corps of Engineers (Corps) to the State of Florida for a broad range of water resources within the State. The primary State 404 Program rules are adopted by the Florida Department of Environmental Protection (FDEP) as Chapter 62-331 of the Florida Administrative Code (F.A.C.). While the State 404 Program is a separate permitting program from the Environmental Resource Permitting program (ERP) under Chapter 62-330, F.A.C., and agency action for State 404 Program verifications, notices, or permits shall be taken independently from ERP agency action, the FDEP and the Southwest Florida Water Management District (SWFWMD) will be participating in a Joint application Process. Upon submittal of an ERP application that proposes dredge/fill activities in wetlands or surface waters within state assumed waters, the SWFWMD will forward a copy of your application to the FDEP for activities under State 404 jurisdiction. The applicant may choose to have the State 404 Program and ERP agency actions issued concurrently to help ensure consistency and reduce the need for project modifications that may occur when the agency actions are issued at different times. Additional information on the FDEP's 404 delegation can be found at: https://floridadep.gov/water/submerged-lands-environmental-resources-coordination/content/state-404-program

Additionally, for those projects located in areas where the Corps retains jurisdiction, the applicant is advised that the District will not send a copy of an application that does not qualify for a State Programmatic General Permit (SPGP) to the U.S. Army Corps of Engineers. If a project does not qualify for a SPGP, you will need to apply separately to the Corps using the appropriate federal application form for activities under federal jurisdiction. Please see the Corps' Jacksonville District Regulatory Division Sourcebook for more information about federal permitting. Please call your local Corps office if you have questions about federal permitting. Link: http://www.saj.usace.army.mil/Missions/Regulatory/Source-Book/

Disclaimer: The District ERP pre-application meeting process is a service made available to the public to assist interested parties in preparing for submittal of a permit application. Information shared at pre-application meetings is superseded by the actual permit application submittal. District permit decisions are based upon information submitted during the application process and Rules in effect at the time the application is complete.

MEETING NOTES

Meeting Date:	March 9, 2023	Date Issued:	March 9, 2023	3	
Location:	D7-HQ, Planning Conference Room and virtual via Microsoft Teams				
Project Name:	WPI Seg # 450438-1 – Gibsonton Drive PD&E Study from Fern Hill to US 301				
Purpose:	Pond Siting Long List Meeting				
Notes by:	Eric Nelson	Amerio	can Project #:	5217733.02	

Attendees:

<u>FDOT</u>: Kirk Bogen, Ashley Henzel, Anthony Celani, Bill McTeer, Robin Rhinesmith, Allison Conner, Marcel Goss, Craig Fox American Consulting: Eric Nelson, Jeff Novotny

A meeting agenda is attached. The meeting began at approximately 11am and was held in the D7-HQ, Planning Conference Room and virtually via Microsoft Teams. The purpose of the meeting was to discuss pond siting and floodplain compensation site options to consider as alternatives for the PD&E study.

Jeff Novotny discussed the background of the project, status and schedule. Google Earth aerial mapping was shown for reference during the meeting along with potential pond site options. Kirk Bogen mentioned this is a County project that FDOT is funding and executing the PD&E study. The status of future phases (design/ROW/construction) is unknown, but the County may be seeking a grant to construct the I-75 interchange improvements which could also include this project.

Eric Nelson discussed the drainage criteria for the project. The anticipated pond design will be wet retention which requires treatment for the first 1 inch of runoff. The basins are open basins and must meet 25yr/24hr pre vs post discharge. The majority of the project is within WBID 1621G – Alafia River Basin, which is impaired for DO and is within a Basin Management Action Plan (BMAP). There is a potential for minor floodplain involvement near Kendra Drive. American will look to minimize the encroachment in that area where there is already an existing boardwalk and possible use of retaining walls. There are 2 general drainage basins. West of US 301 is Basin 1 and east of US 301 is Basin 2.

Mr. Nelson then discussed the potential pond site options to consider as SMF Alternatives as follows (naming is based upon the provided KMZ file):

- Basin 1 Options
 - SMF 10-A is the furthest west site and north of Gibsonton Drive. These two parcels are proposed to be developed and were dropped from further consideration.
 - SMF 10-B is an existing residential site 2 parcels west of Kendra Drive and on the south side of Gibsonton Drive. This site appears to be a viable option. The size may depend on actual needs and may need to be combined with SMF 10-C below
 - SMF 10-C is a residential site immediately west of Kendra Drive and on the south side of Gibsonton Drive (east of 10-B). This site appears to be a viable option. This site would also be ideal for floodplain compensation since it is adjacent to the potential encroachment and the floodplain appears to already cross the site. See note concerning combining with SMF 10-B
 - SMF 10-D is on the north side of Gibsonton Drive between Mathog Rd. and Alafia Trace Blvd. The site is permitted for future development and most likely not a viable option and was dropped from further consideration.
 - SMF 10-E would be an offline pond site about 800' south of Gibsonton Drive and on the west side Mathog Rd. The site is permitted for future development and most likely not a viable option and was dropped from further consideration.

Pond Siting Long List Meeting Notes 450438-1 – Gibsonton Drive PD&E Study March 9, 2023 Page 2

- SMF 10-F would be an offline pond site about 500' south of Gibsonton Drive and on the east side of Mathog Rd. The site is permitted for future development and most likely not a viable option and was dropped from further consideration.
- SMF 10-G is a series of several residential parcels along the north side of Gibsonton near Oakridge and Pineridge side streets. Four are single family residential lots. A fifth has a utility easement (power, gas, etc.) crossing at an angle which may not be viable to consider. These residential sites will require right of way acquisition for the proposed roadway widening improvements. The remnants of the parcels may not be large enough to accommodate a pond large enough needed to treat and attenuate the proposed runoff. Therefore, additional acquisition of parcels north of them may be necessary. Thus, some combination of the properties will be evaluated further. Closing off Oakridge Ave or Pineridge Ave. to make one contiguous pond is not a viable option due to needing to keep the roads open for access. American will further evaluate this area to better refine the drainage needs and identify impacted parcels.
- Basin 2 Option
 - SMF 20 is an existing county pond east of US 301 which is large enough to provide additional treatment and attenuation for the increased impervious area east of US 301. An adjustment in the control elevation may only be needed to support the drainage needs at this location. No additional pond site alternatives would be required for this basin.

Bill McTeer suggested looking at an additional site in Basin 1 south of the proposed Pond 2B located at the bend in Fern Hill Drive on the south side of Gibsonton Drive. This is part of the Gibsonton Drive at Fern Hill Intersection Improvement project (CIP #69600311). The proposed pond could potentially be expanded to the south to accommodate the proposed improvements for this project.

There is a large parcel owned by Hillsborough County that is designated as Alafia Scrub Preserve and Trails north of Gibsonton Drive and extends to the Alafia River. This is not a viable alternative since it's a County ELAPP parcel and would open potential Section 4f coordination and lengthy coordination with the County. This parcel will be discussed at the next coordination meeting with the County engineering staff to confirm.

Anthony Celani noted that there is a Hillsborough County SWMM model that has different floodplain boundaries than the FEMA FIRM, and suggested that American contact the County to obtain this information.

Action items:

- American will proceed further with evaluation of SMF 10-B/10-C, 10-G and the site south of Pond 2B on Fern Hill as SMF alternatives for Basin 1. Basin 2 will not have any options.
- American will contact Hillsborough County to obtain the SWMM model and Floodplain information, then verify floodplain involvement.
- American will schedule a meeting with SWFWMD to discuss stormwater criteria, specifically the Alafia River BMAP criteria. Anthony Celani and Craig Fox requested to be in attendance. American will forward a list of available meeting times.
- American will provide status on these alternative sites being evaluated for information and comment at the next coordination meeting with Hillsborough County when the proposed typical section and traffic analysis are discussed.
- American will provide Bill McTeer (through Craig Fox) with aerial maps and information needed to prepare ROW cost estimates on the alternative sites and will initiate desktop environmental screening in order further identify the preferred SMF sites in the coming months.

Meeting Agenda

Gibsonton Drive PD&E Study From Fern Hill to US 301 WPI Seg No: 450438-1

Drainage Coordination – SMF/FPC Longlist meeting Thursday March 9, 2023 – 11am – FDOT District 7 – FDOT Planning

- 1. Welcome and Introductions
- 2. Current Project Status & Proposed Improvements Jeff Novotny
- 3. Design Criteria Eric Nelson
 - a. Water Quality
 - b. Discharge
 - c. Floodplain Compensation
 - d. Existing Permits
- 4. Basin requirements and options under consideration Eric Nelson
- 5. Next steps
 - a. Finalize SMF/FPC viable alternatives
 - b. Desktop environmental review
 - c. Cost estimates & ROW estimates
 - d. Arrive at Preferred Sites (May)
 - e. Pond Siting Report & Location Hydraulics Report
MEETING NOTES

Meeting Date:	August 23, 2023	Date Issued:	August 30, 20	23					
Location:	D7-HQ, Pelican Conference Room and virtual via Microsoft Teams								
Project Name:	WPI Seg # 450438-1 – Gibsonton Drive PD&E Study from Fern Hill to US 301								
Purpose:	Pond Siting Selection								
Notes by:	Jeff Novotny	Amerie	can Project #:	5217733.02					
Copies to:	File								

Attendees:

<u>FDOT</u>: Kirk Bogen, Ashley Henzel, Anthony Celani, Bill McTeer, Robin Rhinesmith, Allison Conner, Lisa Quinn, Marcel Goss, Craig Fox, Lonnie Whitmeyer, Matt (ROW), Robert (Relocations) <u>American Consulting</u>: Eric Nelson, Tom Daniel, Jeff Novotny

A concept plan map showing alternatives sites and an evaluation matrix was distributed to FDOT staff in advance of the meeting.

The meeting began at approximately 9:30am and was held in the D7-HQ, Pelican Conference Room and virtually via Microsoft Teams.

The purpose of the meeting was to discuss alternative pond sites further evaluated following an earlier March 3, 2023 pond shortlist meeting. Since the meeting, the pond alternatives were refined, ROW cost information was provided by the District and the sites were evaluated for potential environmental effects through a desktop review of archaeological, biological and contamination resources. Construction costs were also estimated for the alternative sites. The results of this information was provided on the matrix.

Jeff Novotny discussed the background of the project, status and schedule. The concept plan map was shown Google Earth aerial mapping was shown for attendees reference during the meeting showing the pond site alternatives.

Eric Nelson discussed the alternative pond sites (SMF 1a, 1b, 1c). Site SMF 2 is located east of US 301 and an existing site. There will be no footprint change to that site. The control elevation would be proposed to change very slightly to gain the required treatment/attenuation. There is one FPC site (FPC 1a) located in the parcel east of SMF 1b and would work for all other SMF sites as well.

Other than potential relocations, there was no appreciable difference in other environmental impacts for the alternative sites. Based on the evaluation matrix, SMF 1b and FPC 1a would result in lower costs and relocations than for SMF 1a/FPC1a or SMF 1c/FPC1a.

After short discussion, SMF 1b and FPC1a were selected as the preferred pond site/floodplain compensation site.

<u>Action Items:</u> American will finalize draft Pond Siting Report for review submittal. The field work will be initiated for the clearance evaluation of cultural resources, wetlands/protected species, and contamination to keep the project moving forward.

SMF #	Pond Area (Ac)	FPC #	FPC Area (Ac)	Conveyance Easement (Ac)	Est. Wetland Impacts (Ac)	Wildlife and Habitat	Contamination	Cultural Resources	Potential Relocations ¹	Est. Construction Cost ²	Est. Wetland Mitigation Cost	Est. Right of Way Cost SMF ²	Est. Total Cost	Comments
1A	1.47	1A	1.49	N/A	-	None	None	None	5R + 1B	\$277,857	\$0	\$3,899,800	\$4,177,657	
1B	1.66	1A	2	N/A	-	None	None	None	2R + 2B	\$73,736	\$0	\$3,197,200	\$3,270,936	Recommended Alternative
1C	1.79	1A	1.49	N/A	-	None	None	None	9R + 1B	\$221,745	\$0	\$5,733,500	\$5,955,245	

¹R = Residential; B = Business

²Total of SMF and FPC cost.

APPENDIX A

CONCEPTUAL DRAINAGE MAPS





