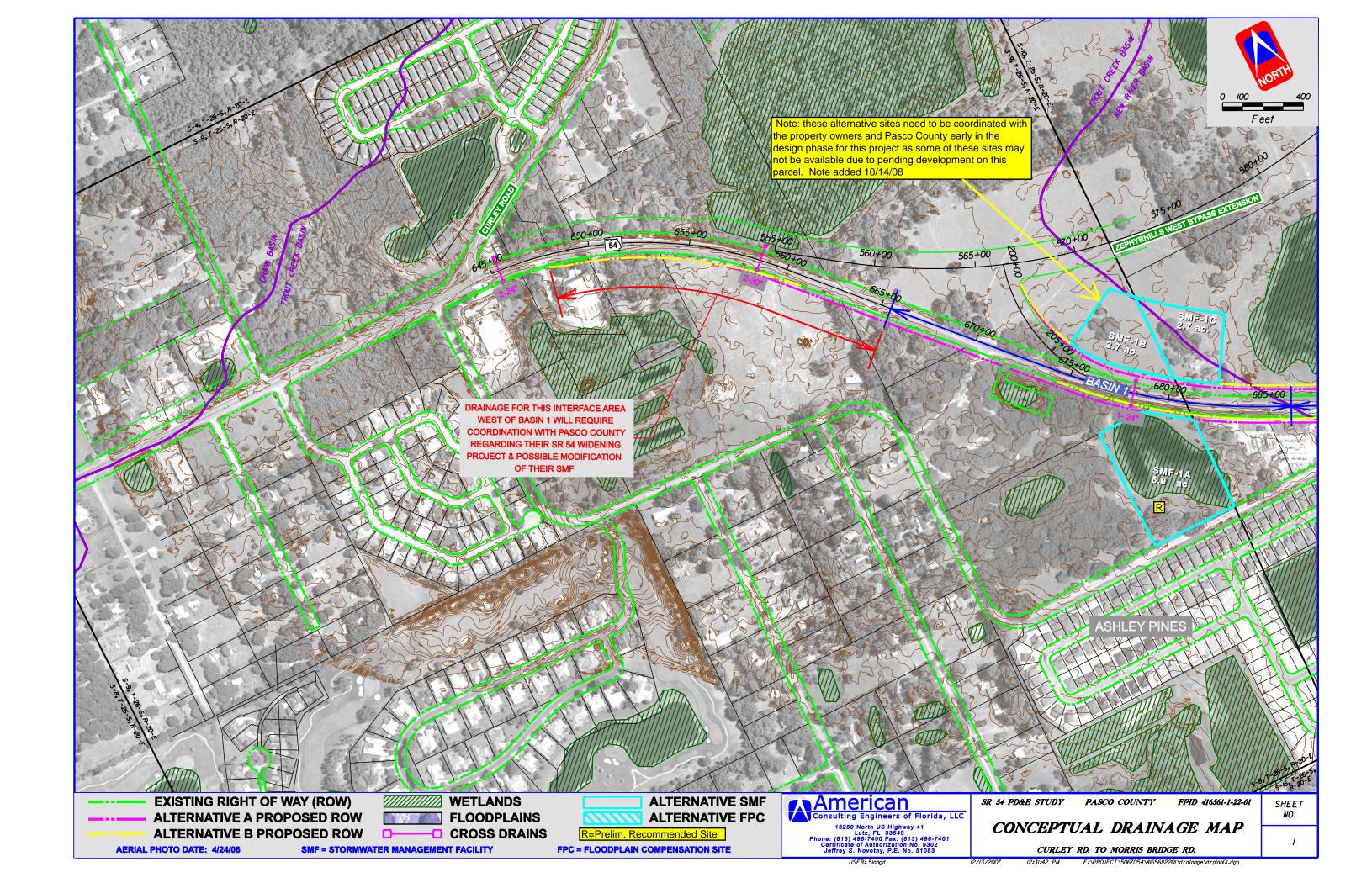
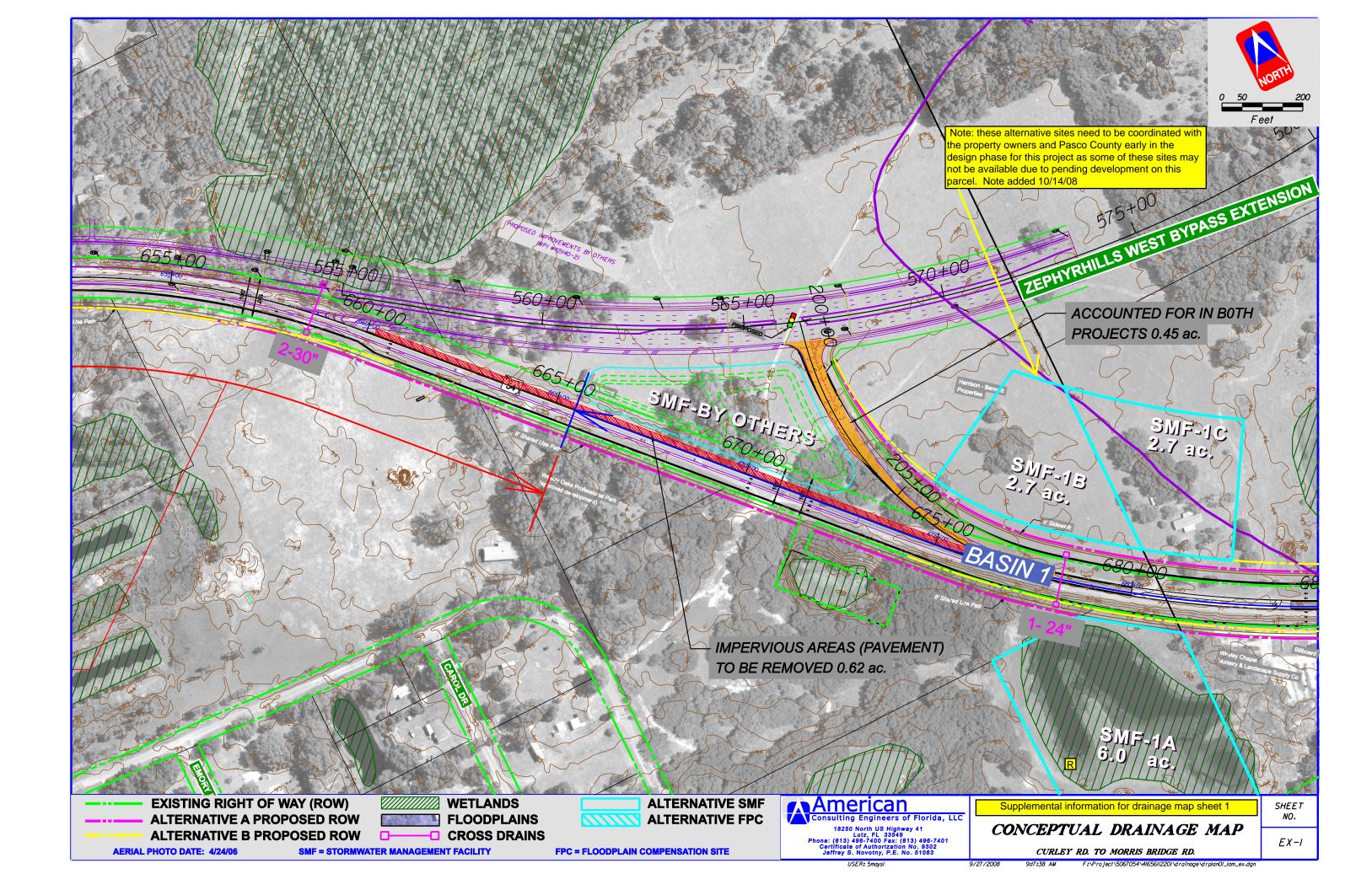
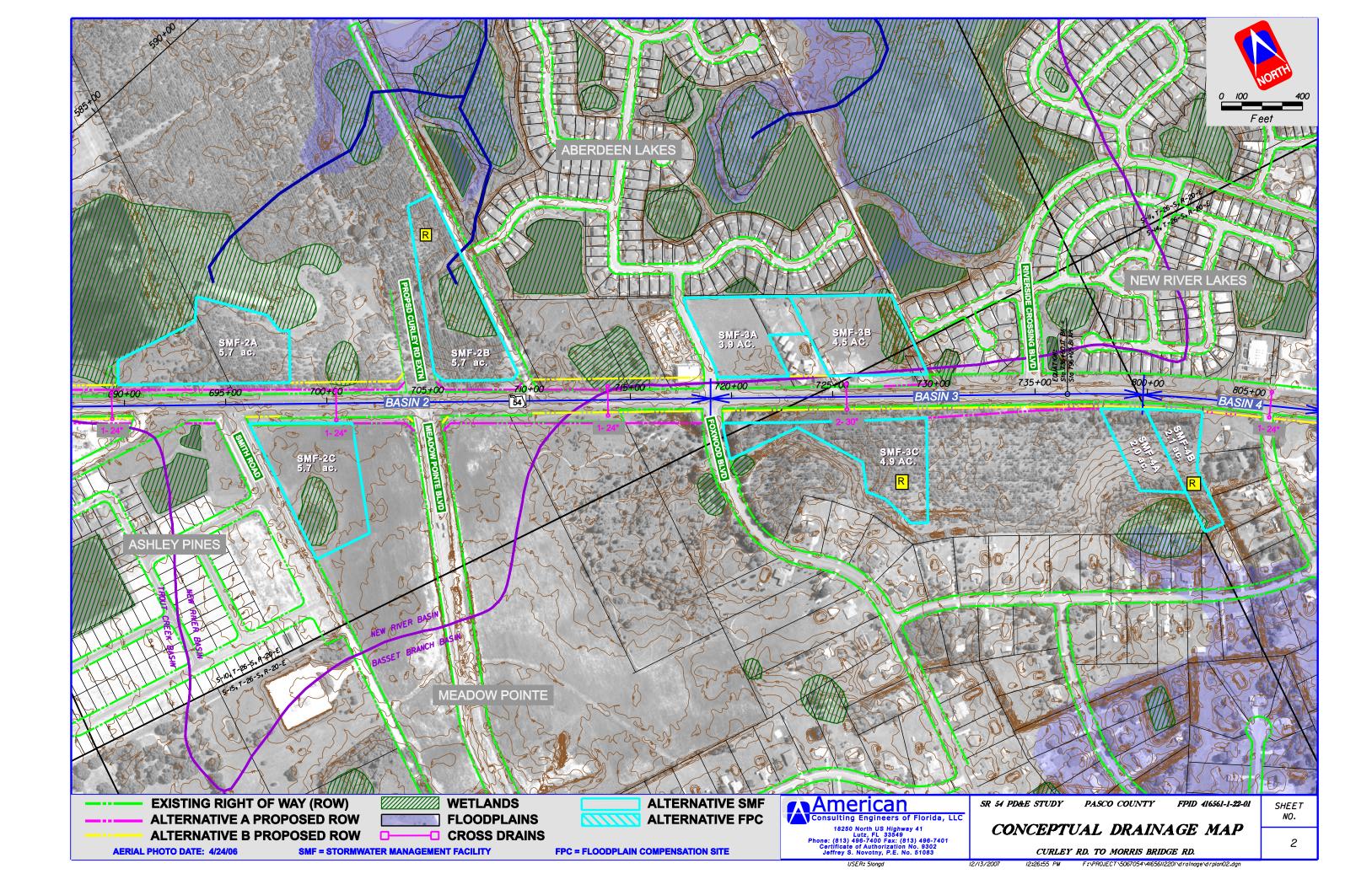
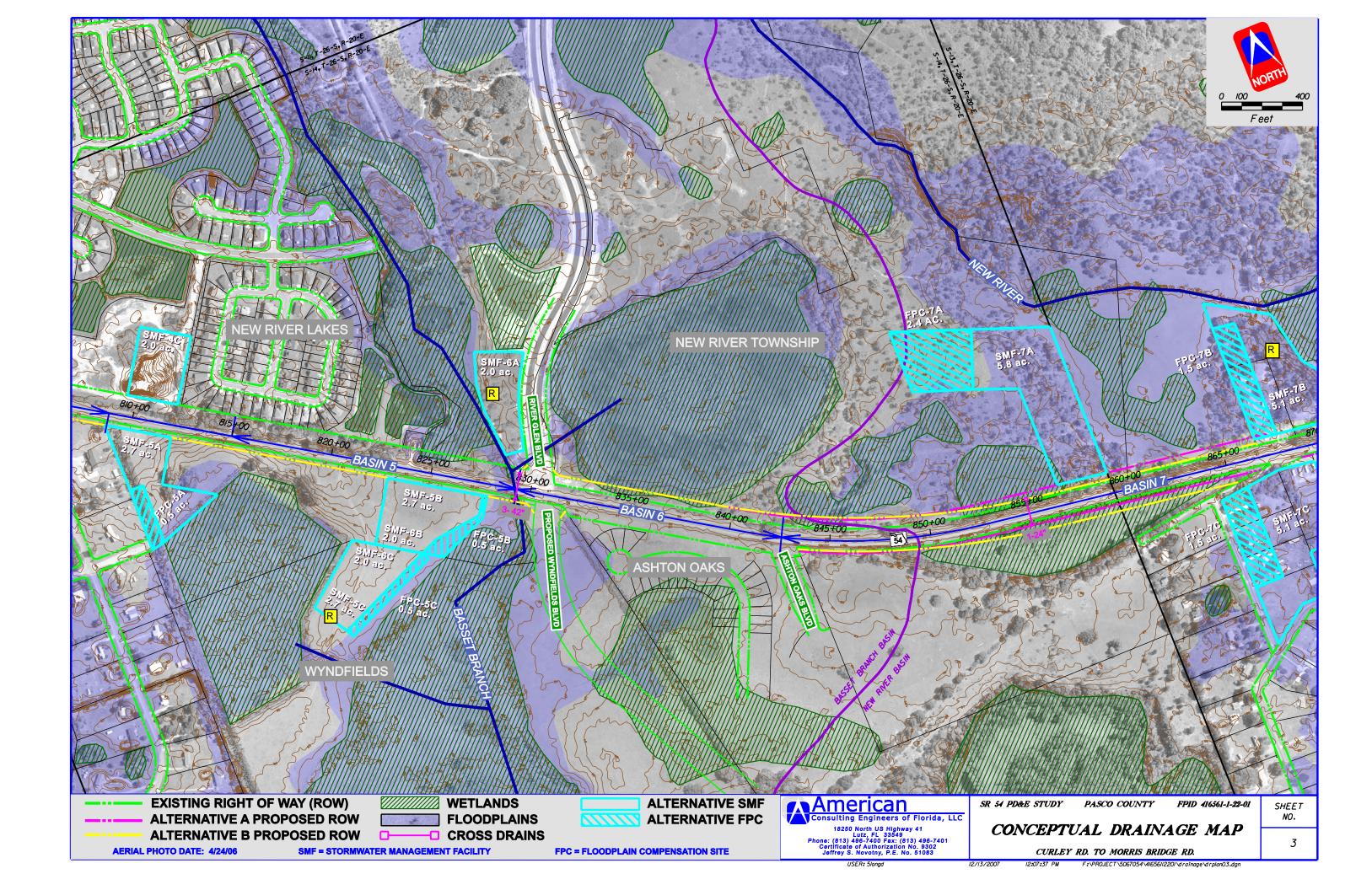
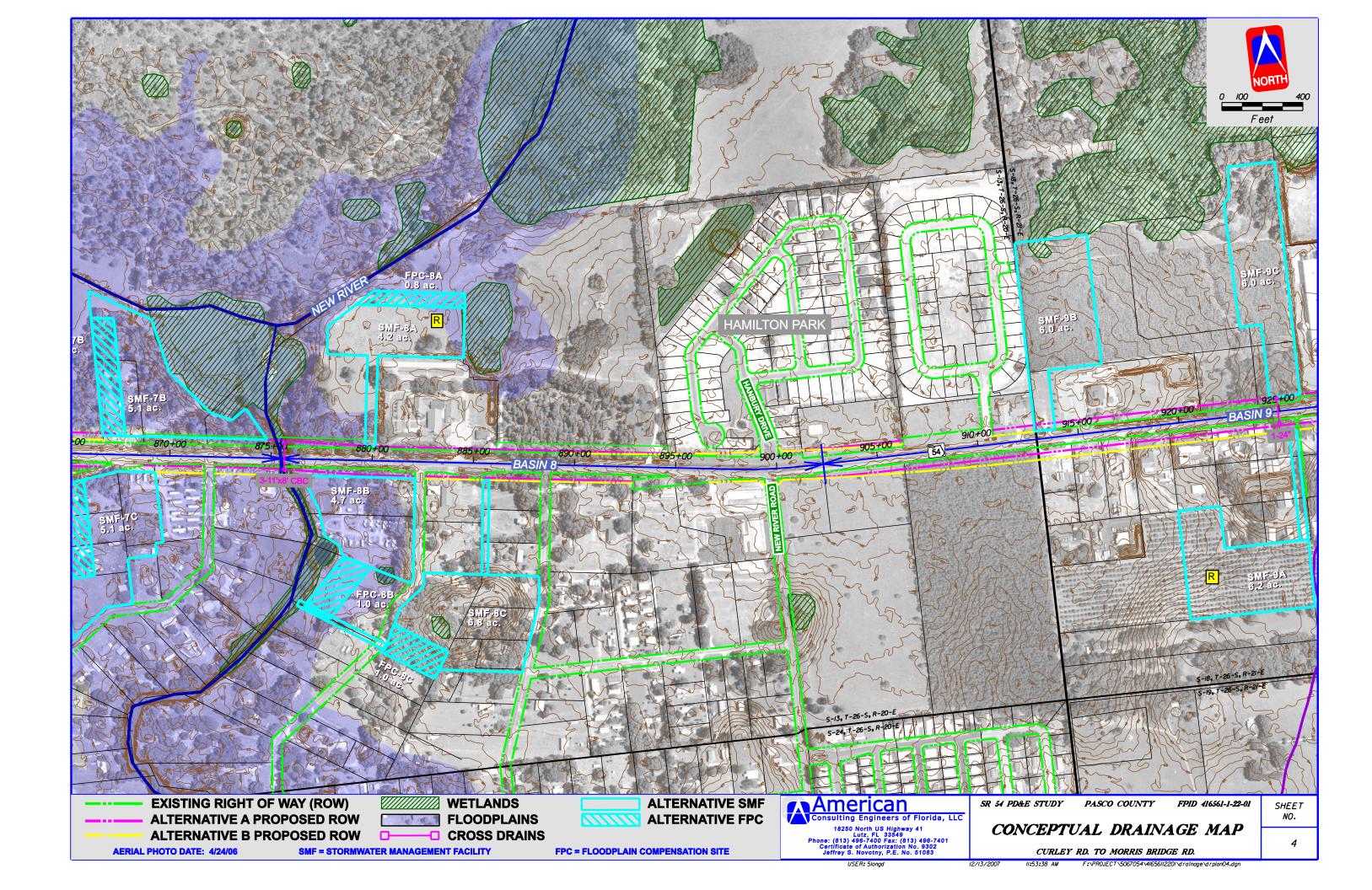
SR 54 Project Development and Environment (PD&E) Study From CR 577 (Curley Road) to CR 579/CR 54 (Morris Bridge Road)	
	APPENDIX
	A
	Drainage Maps
	With
	Alternative
	Pond Sites
Final Preliminary Engineering Report	

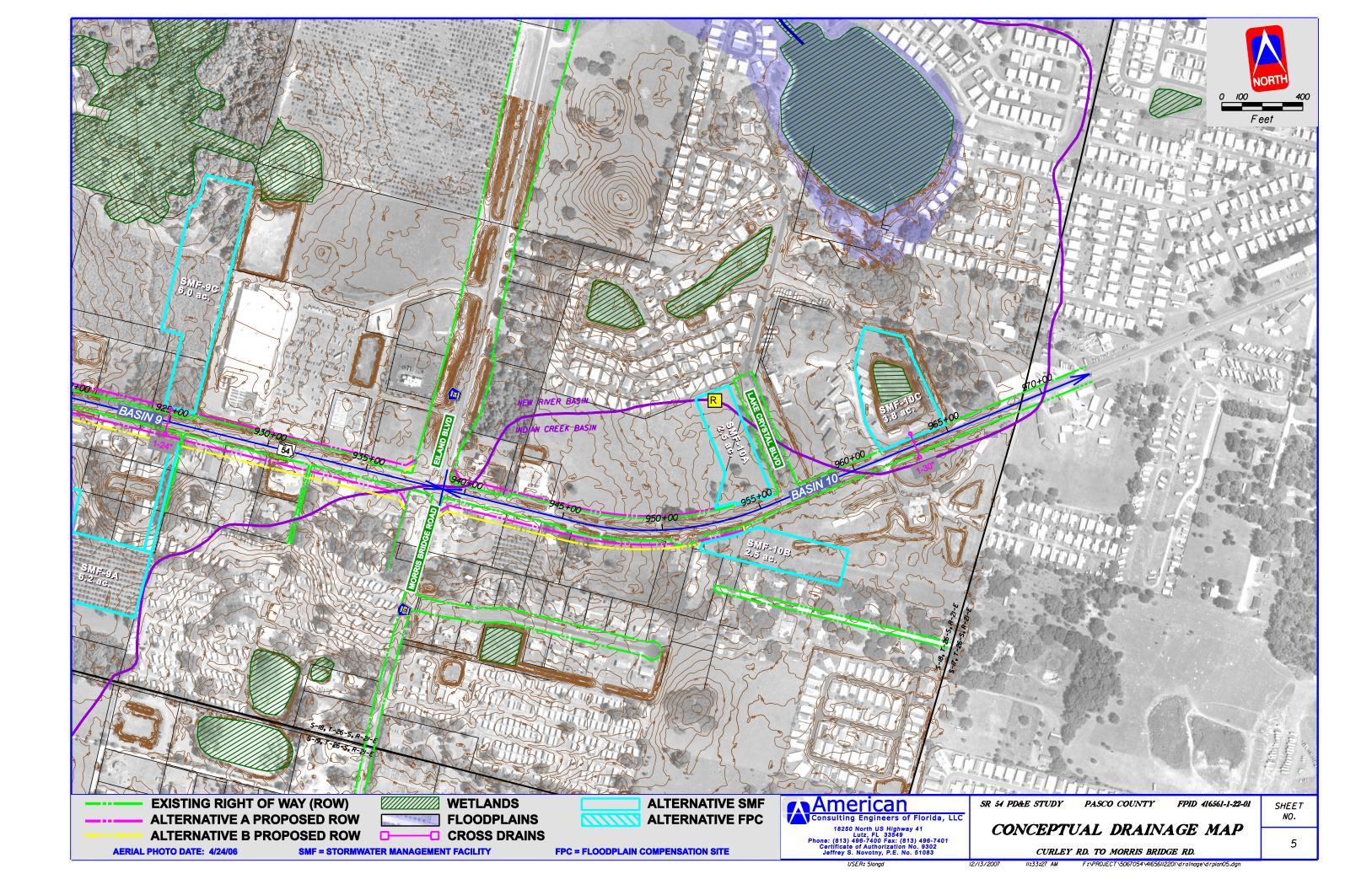












SR 54 Project Development and Environment (PD&E) Study From CR 577 (Curley Road) to CR 579/CR 54 (Morris Bridge Road)	
	APPENDIX
	В
	Conceptual
	Design Plans
	(Recommended Alternative)
	Aitemative
Final Preliminary Engineering Report	

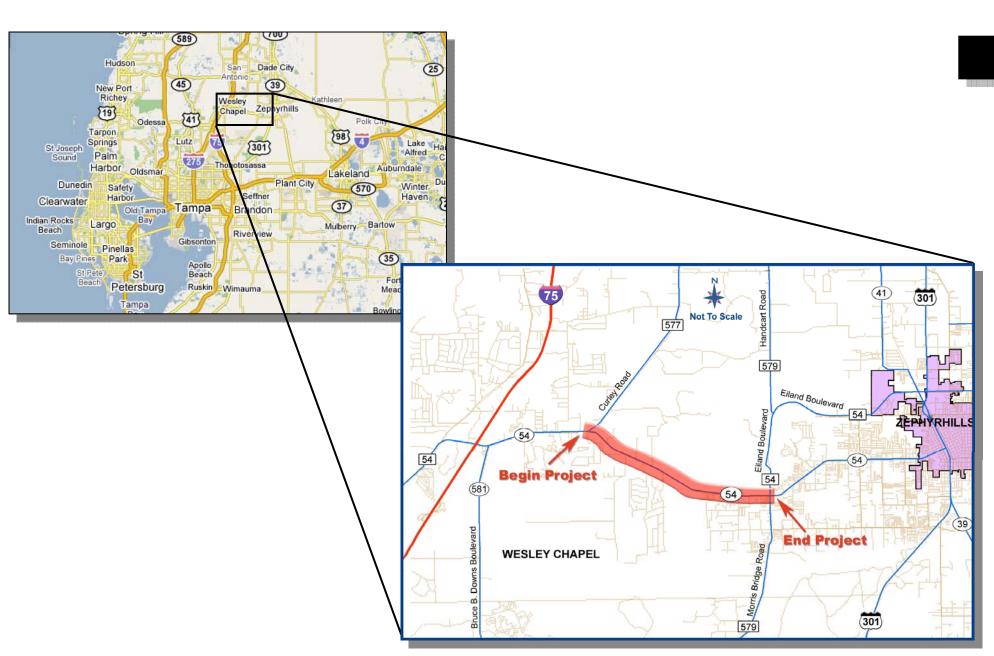
Preliminary Conceptual Design Plans



SR 54 PD&E STUDY

From CR 577 (Curley Rd) to CR 579 / CR 54 (Morris Bridge Rd)

WPI Segment Number 416561-1



Preferred Alternative



Prepared For:
The Florida Department of
Transportation District Seven Modal
Planning and Development Section

By:

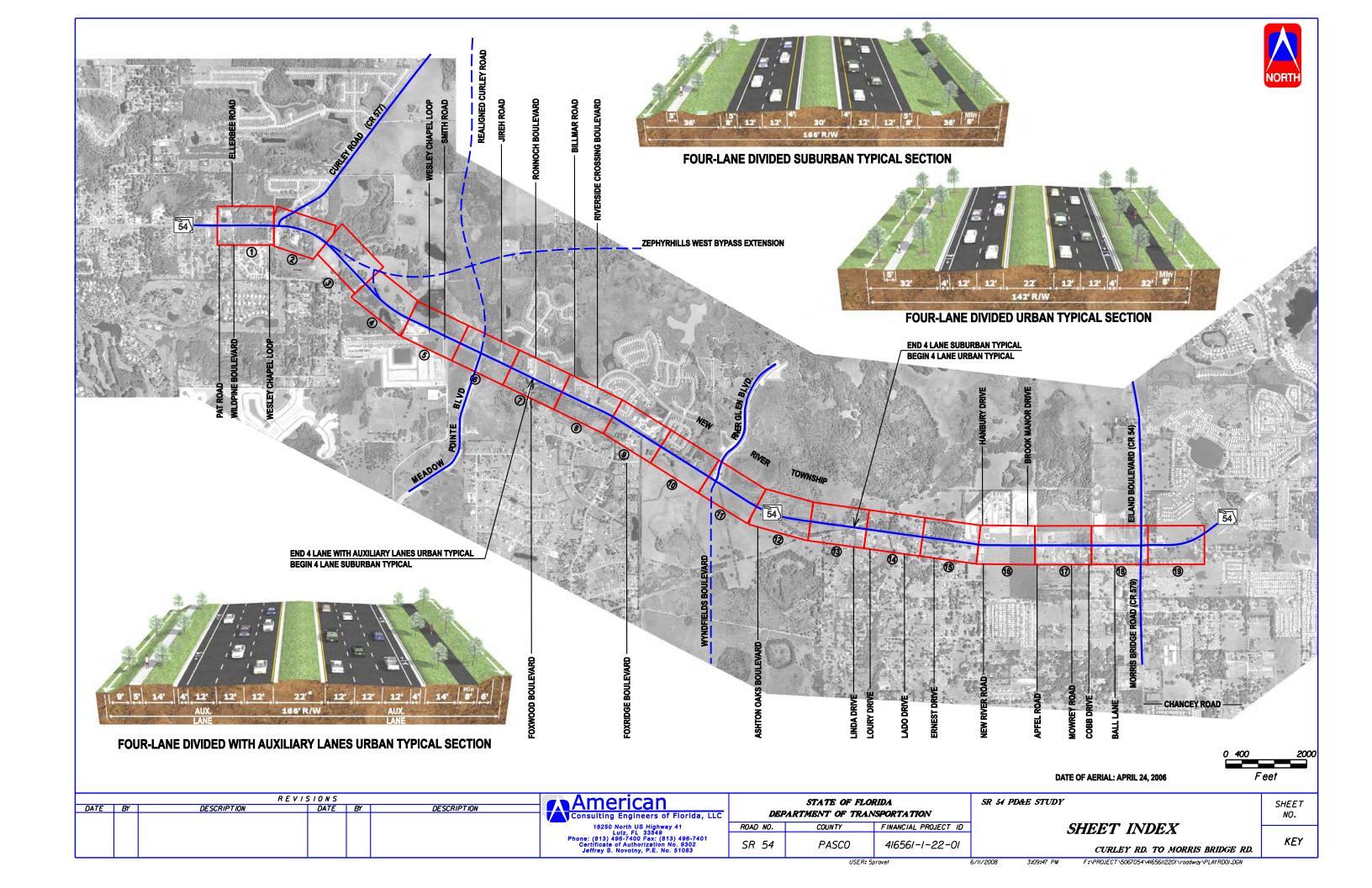


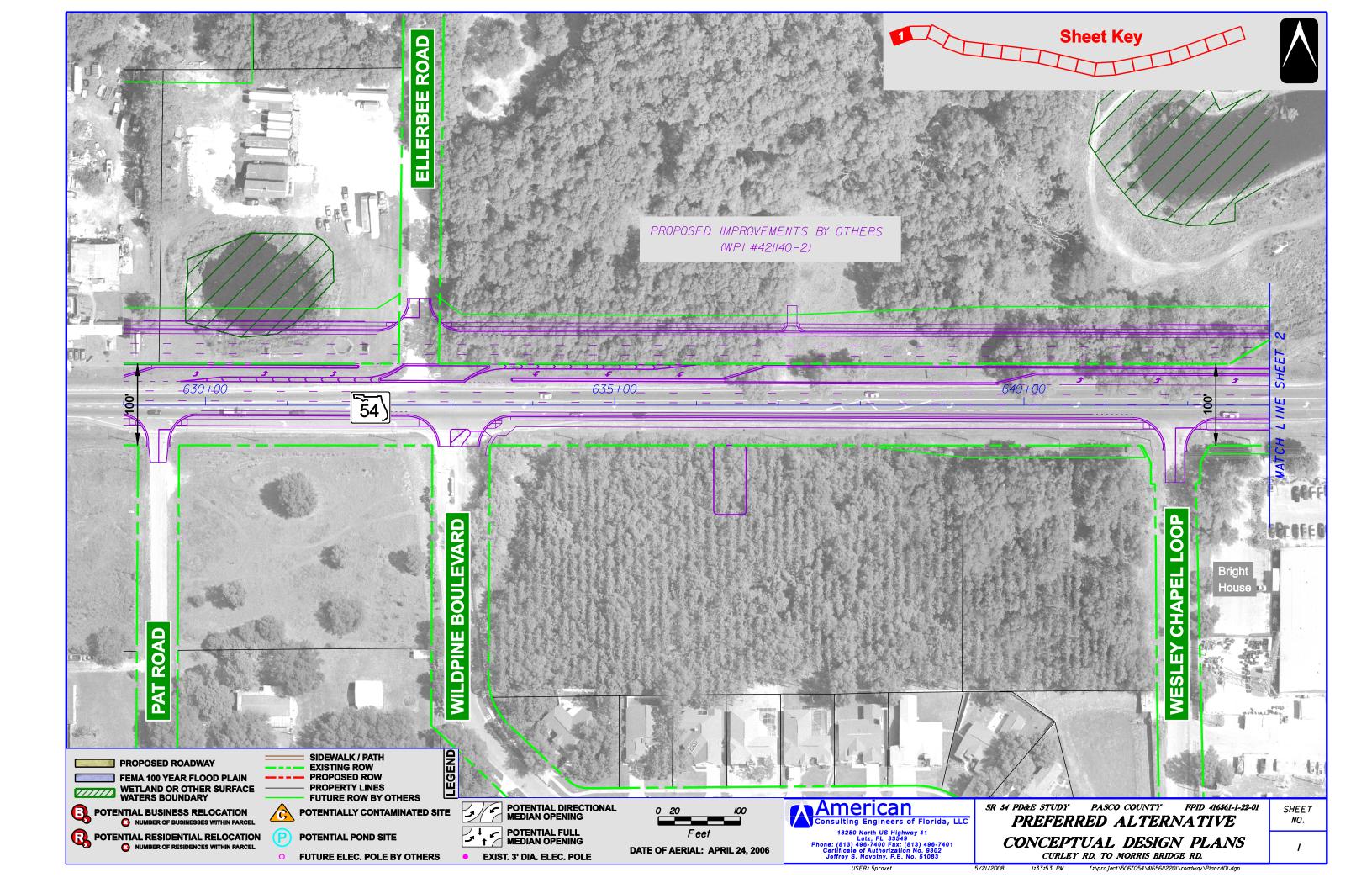
Rev. October 6, 2008

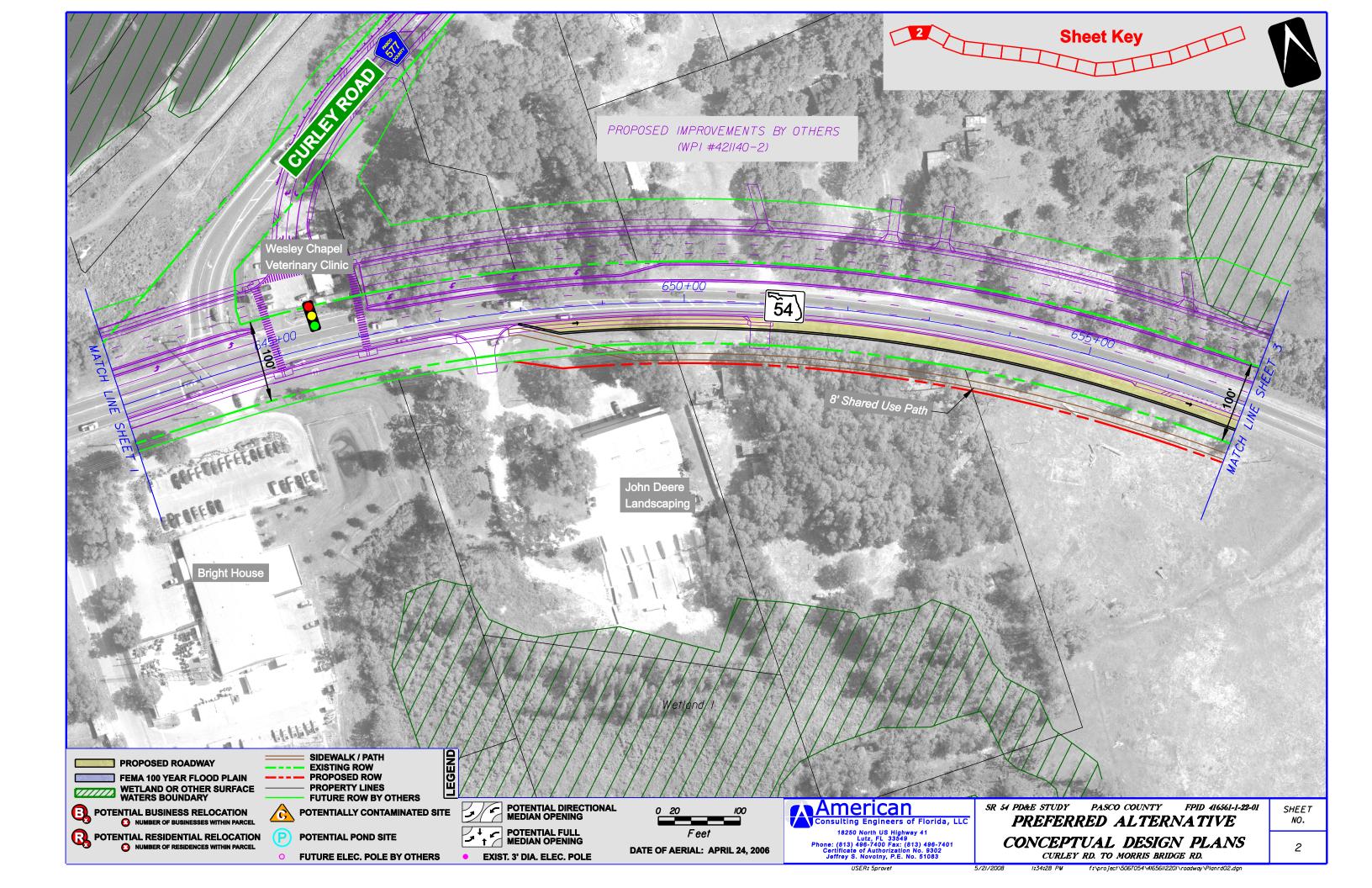
Sheets 3, 5, and 6 Rev. 3/25/09

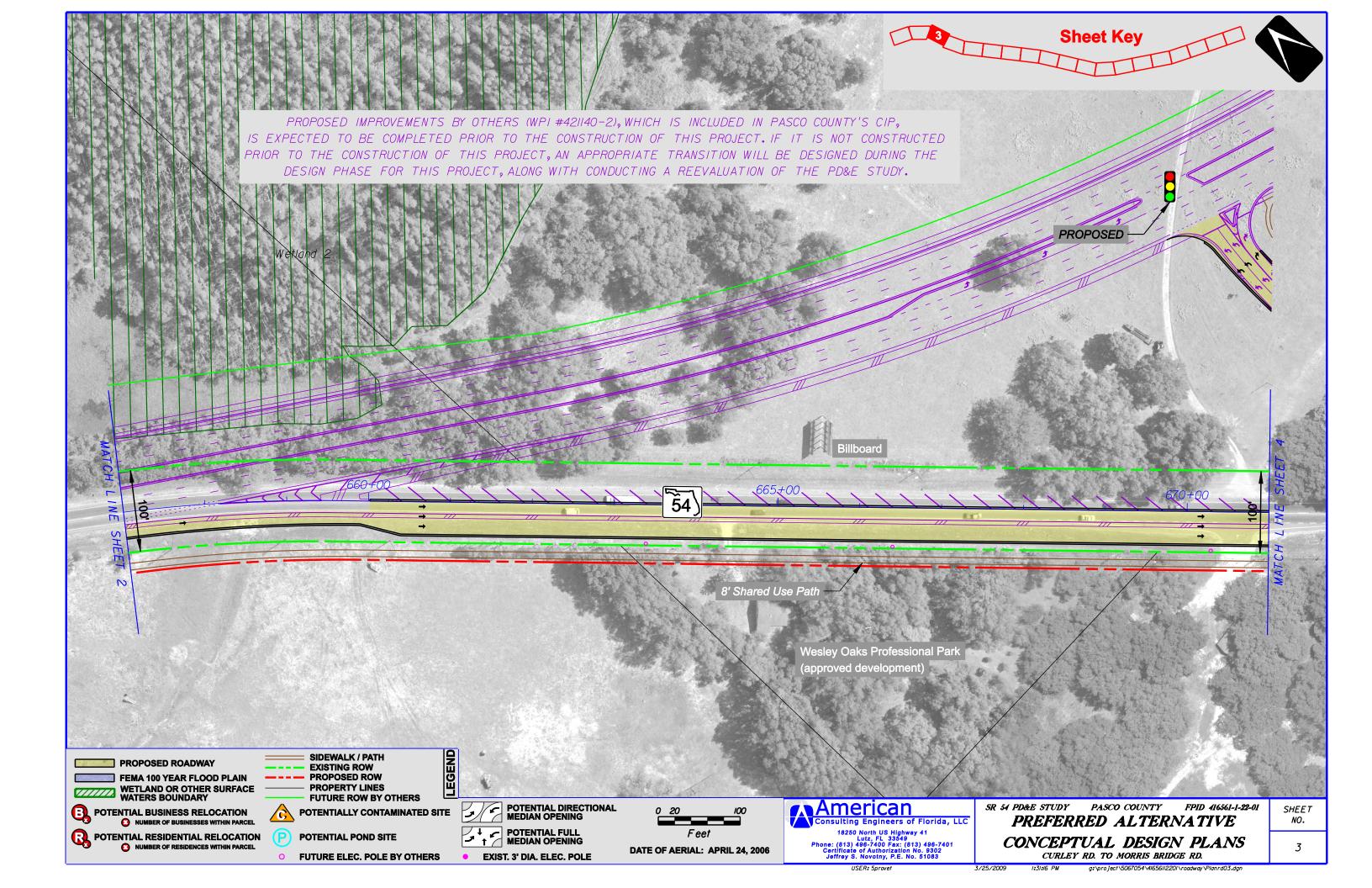
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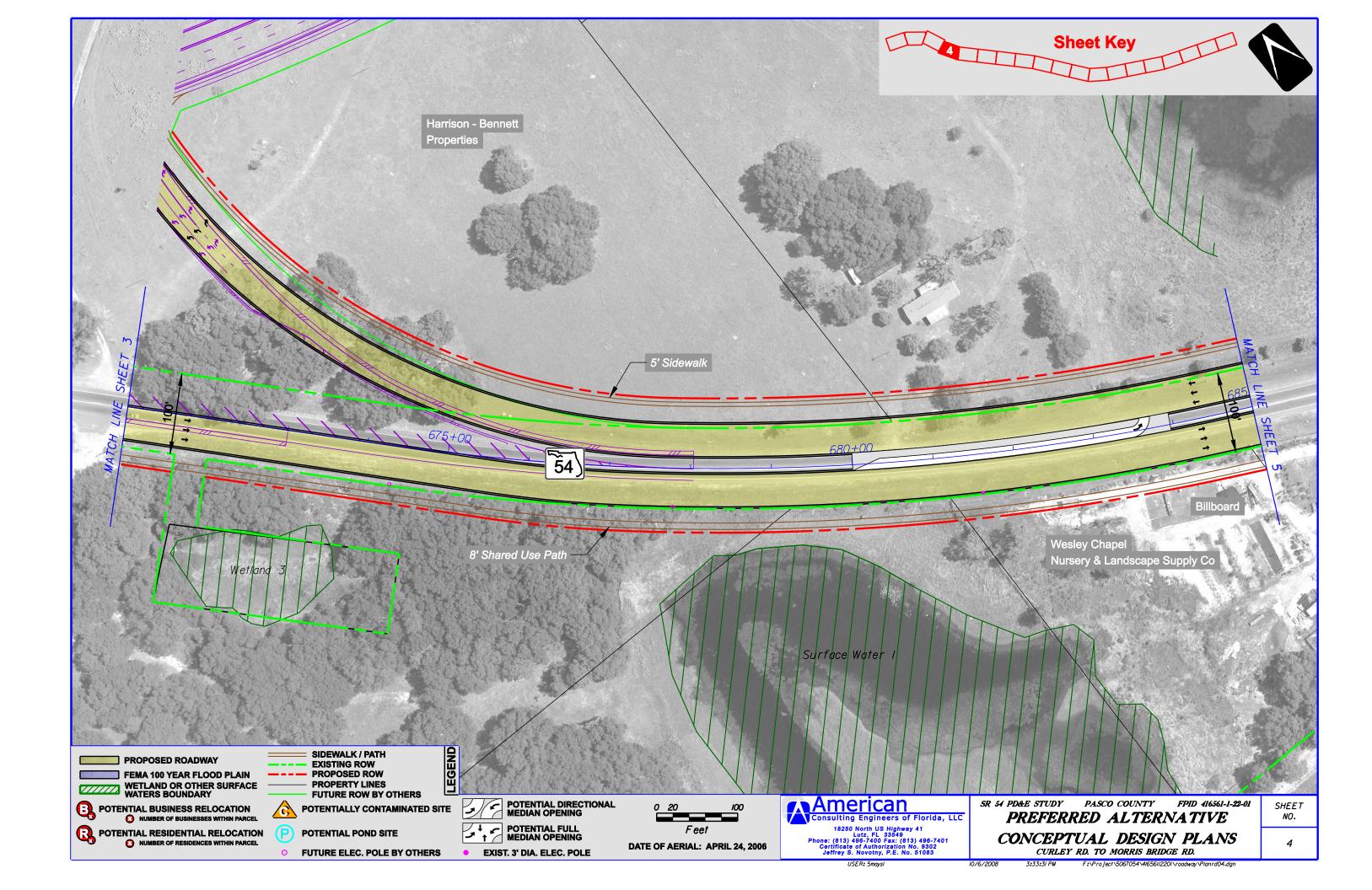
FDOT Project Manager: Manny Santos, El

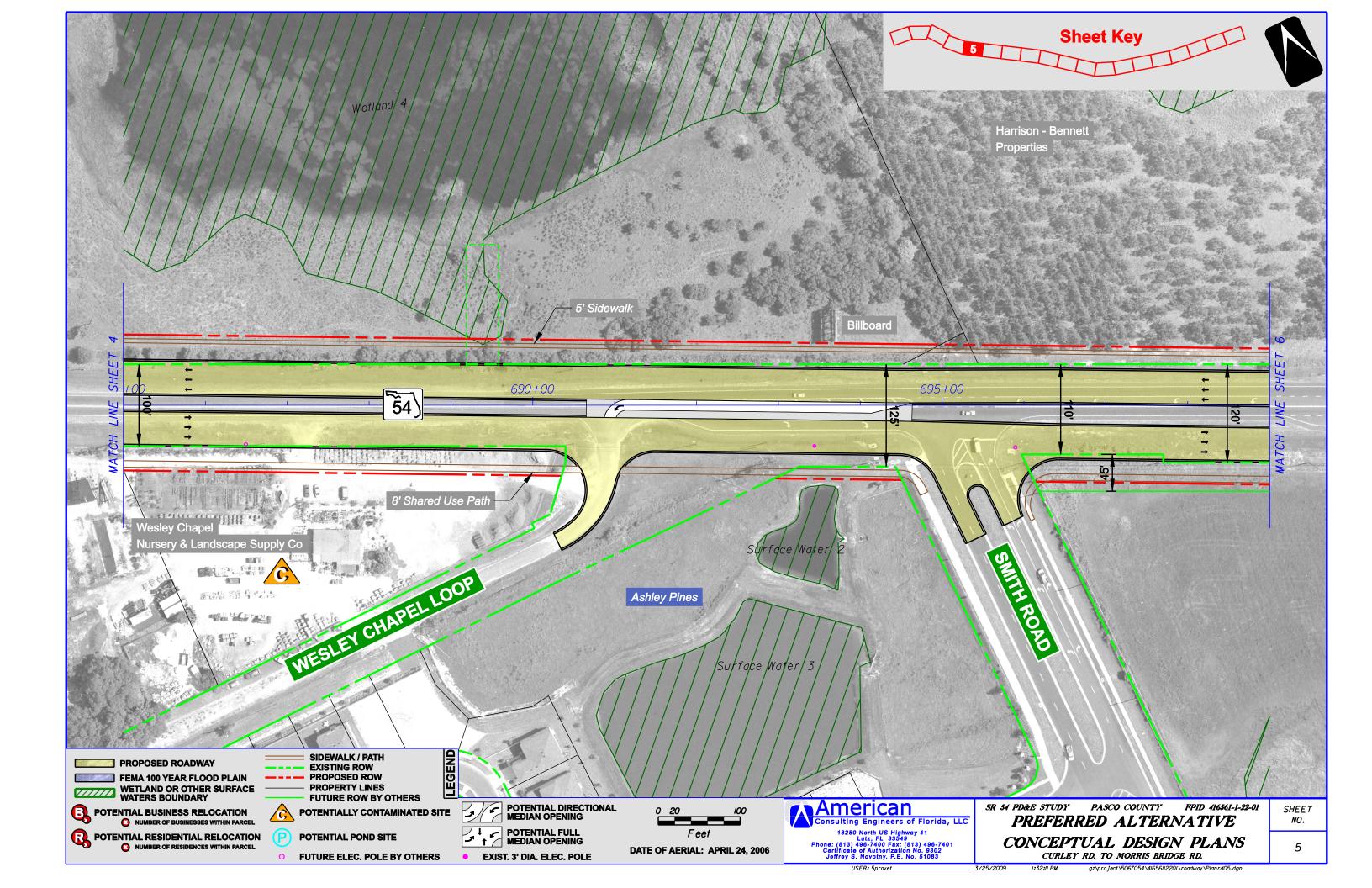


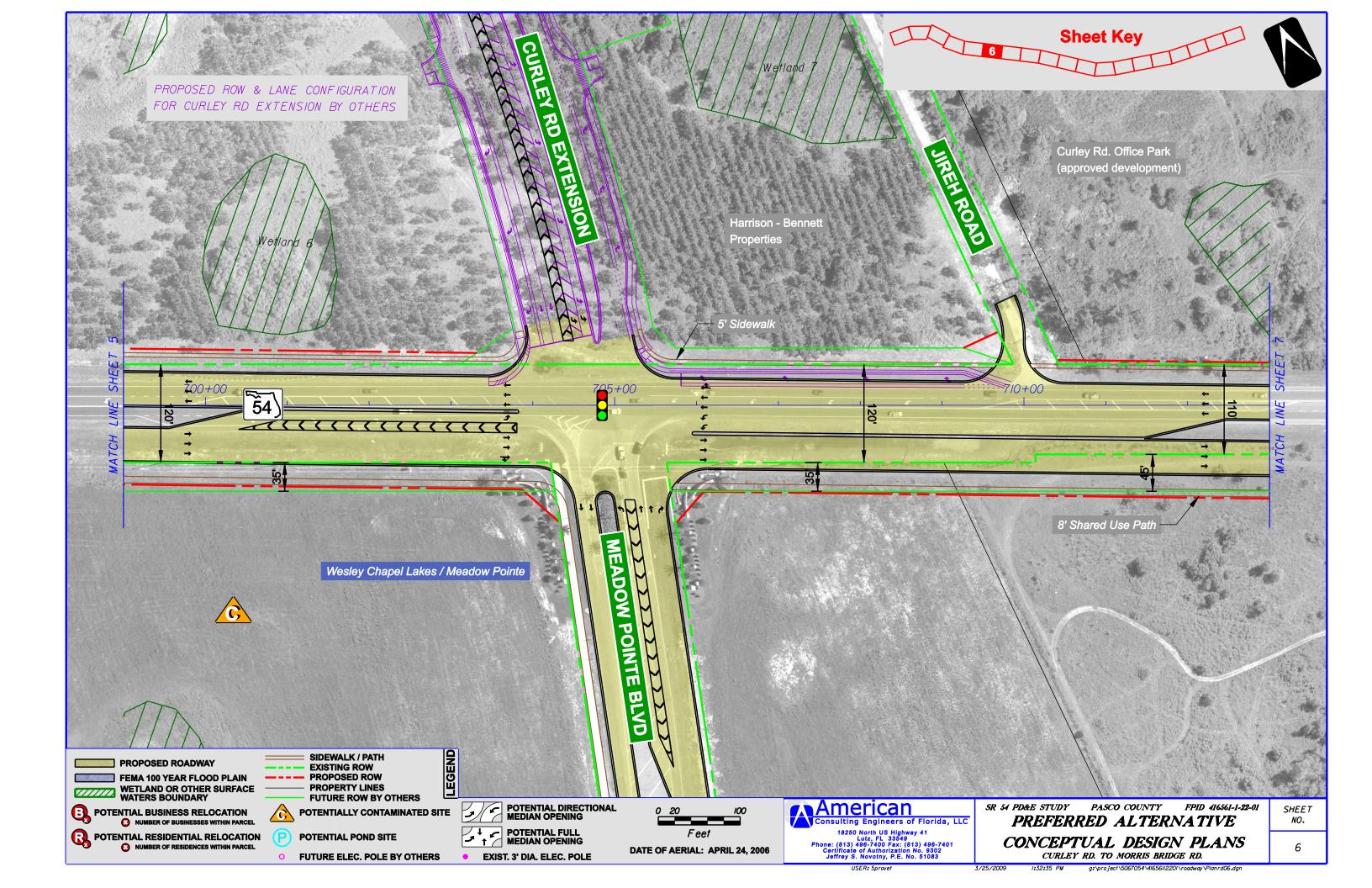


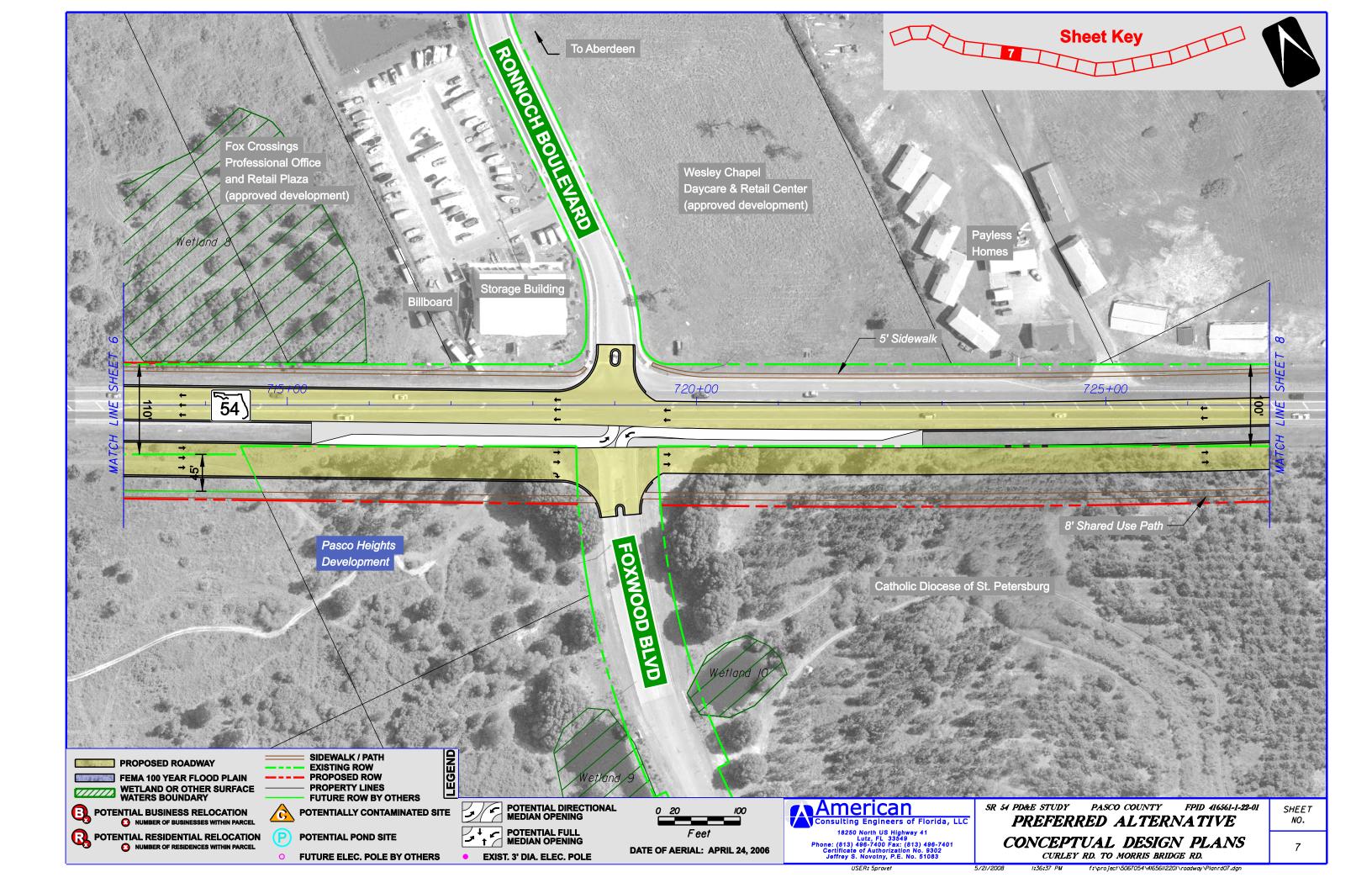


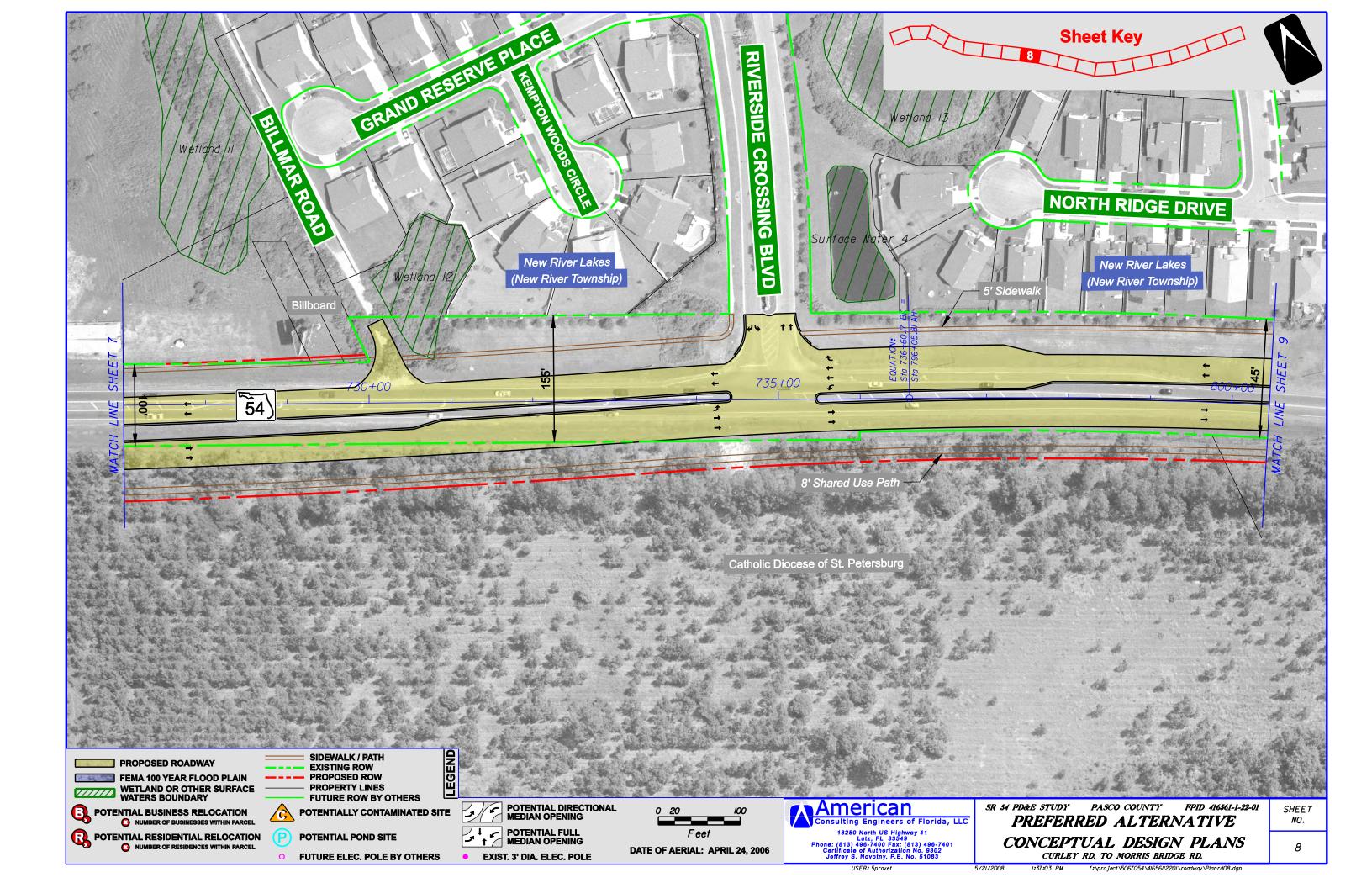


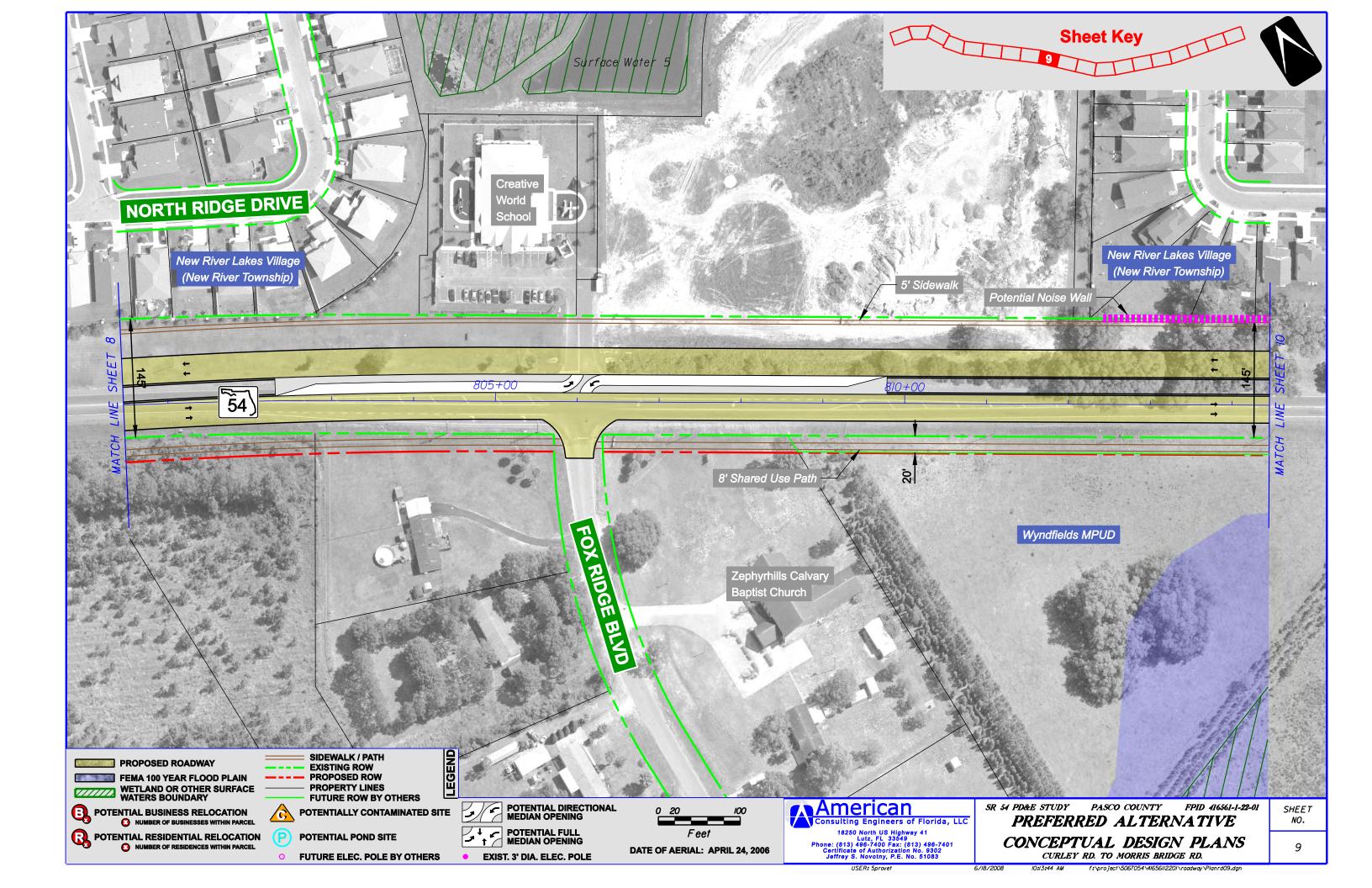


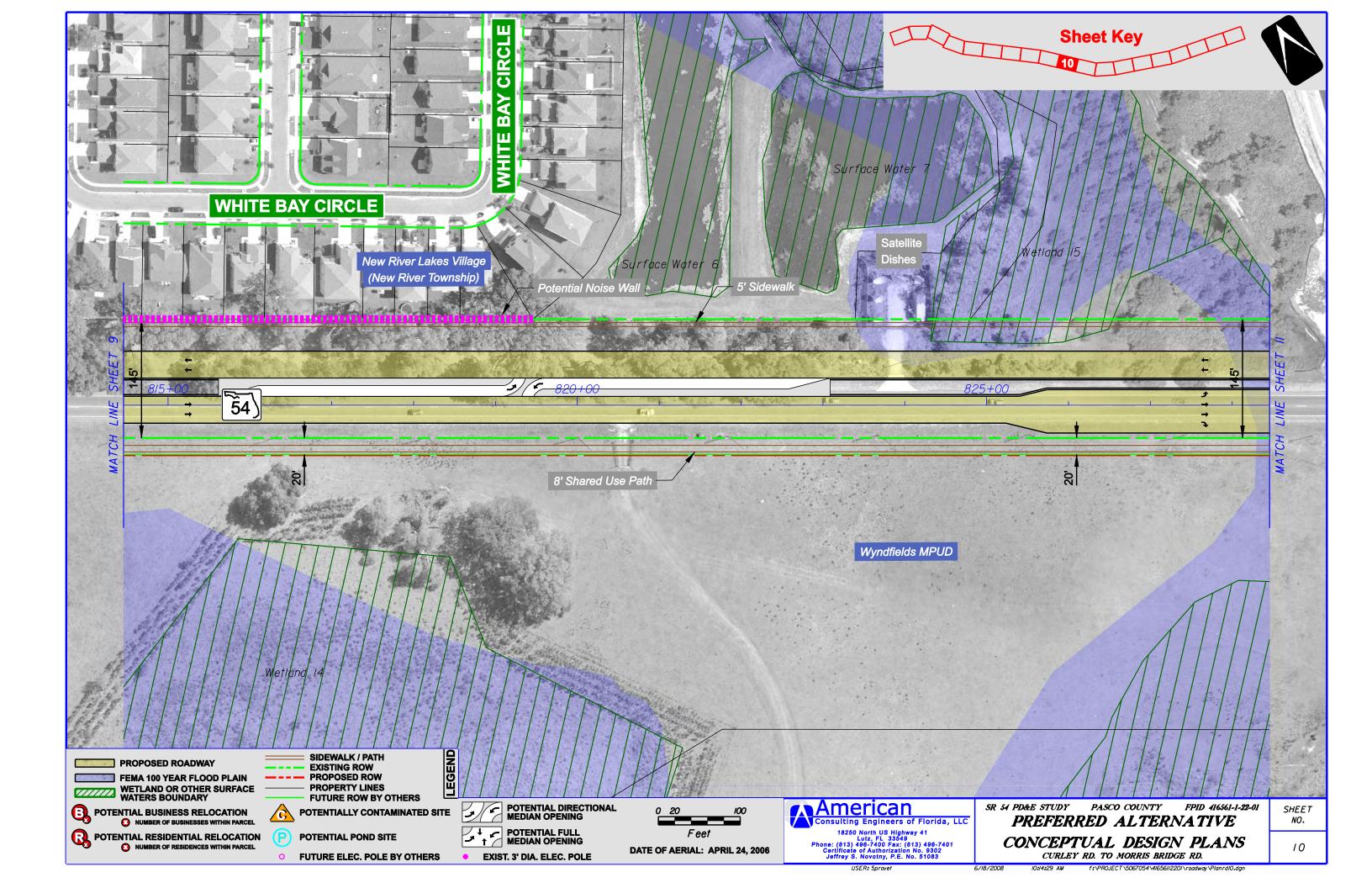


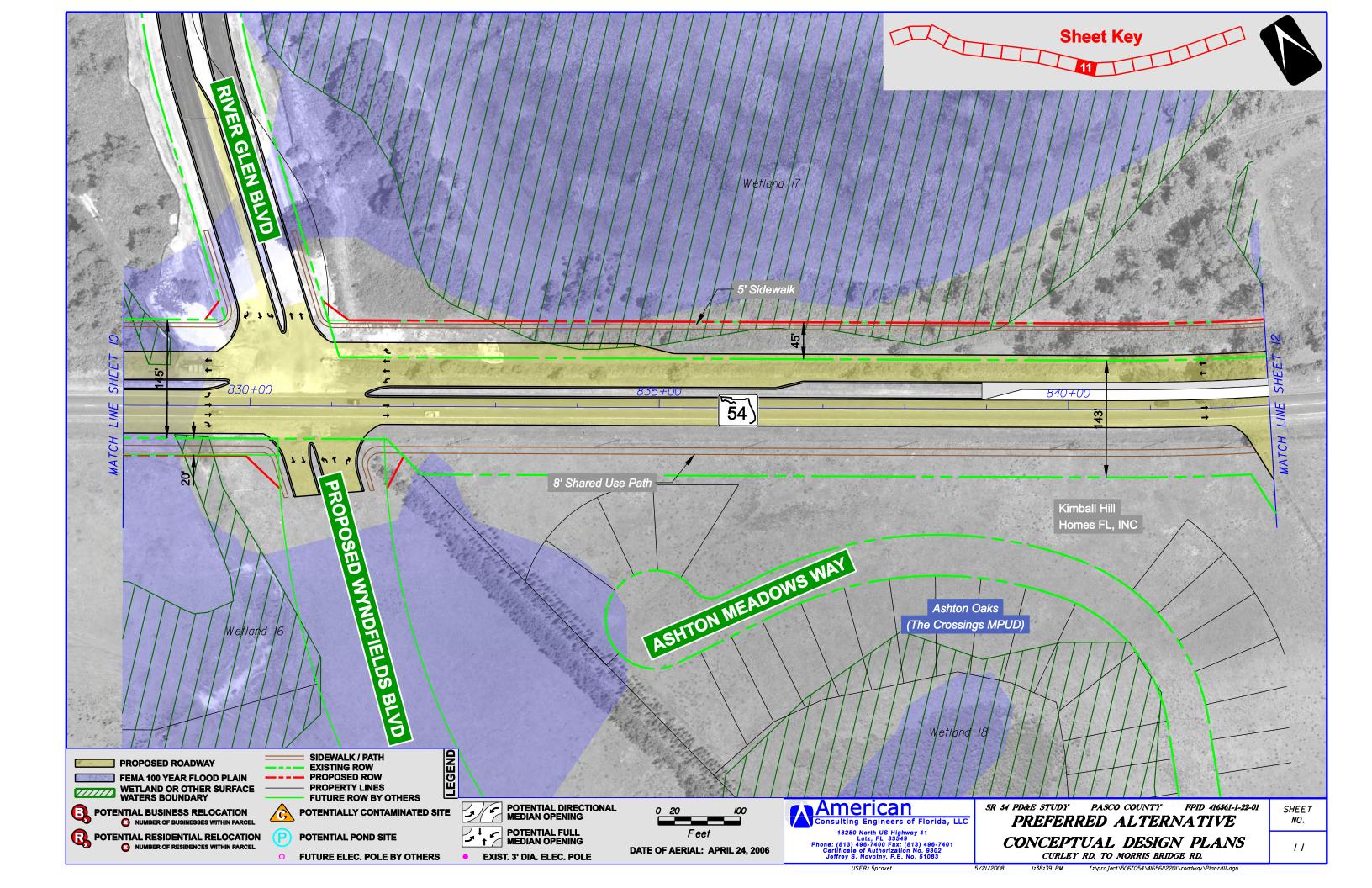


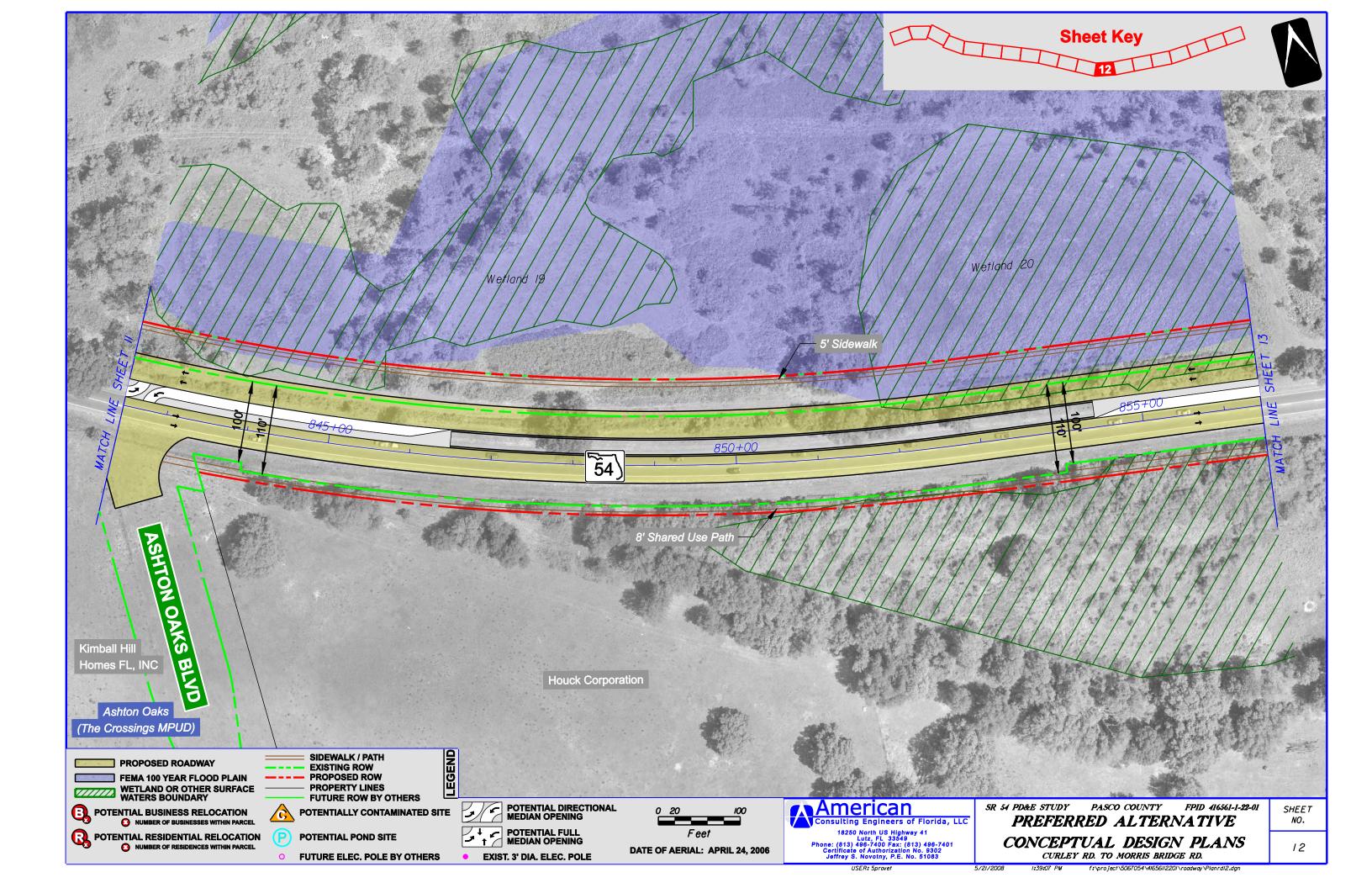


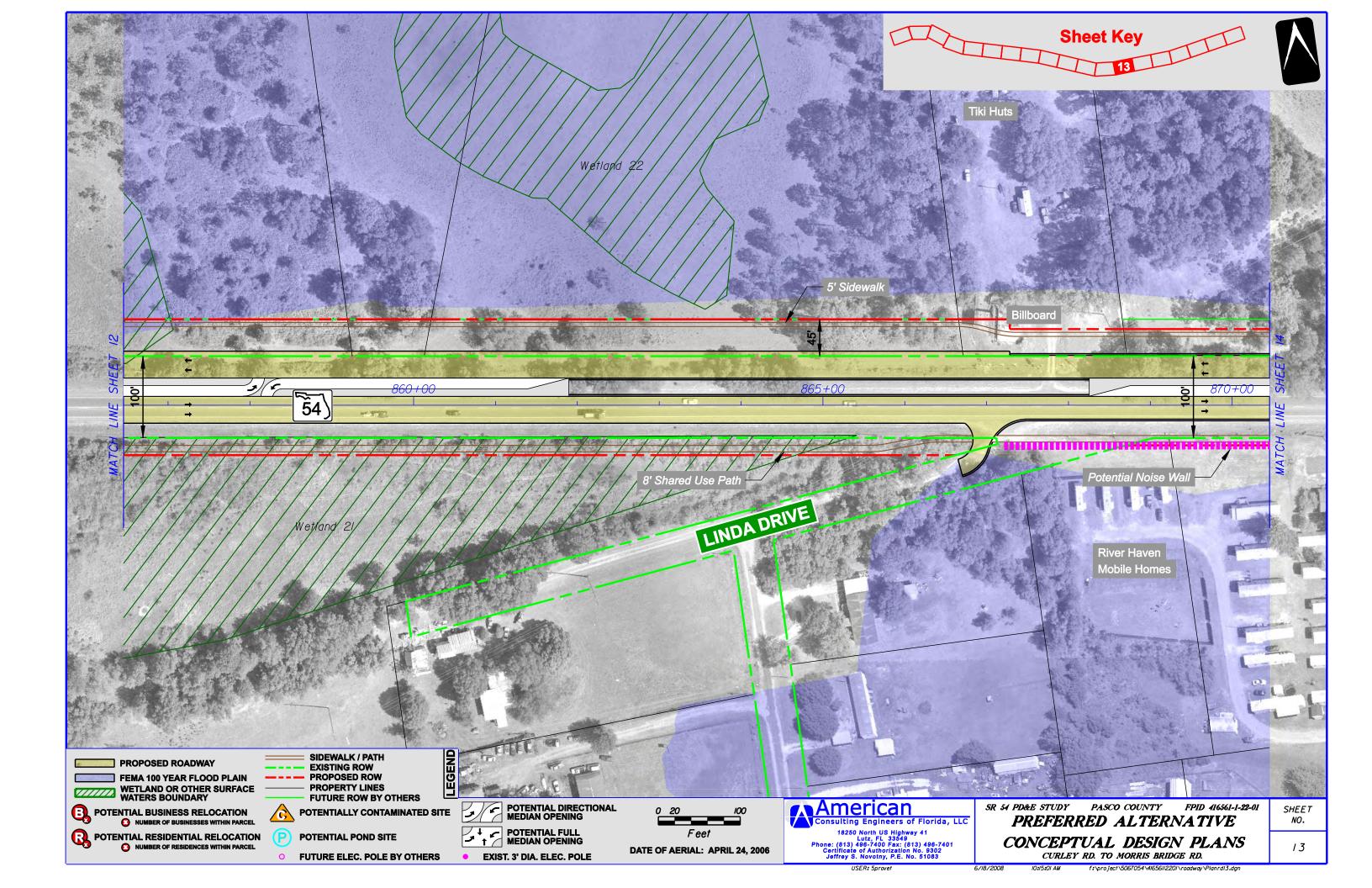


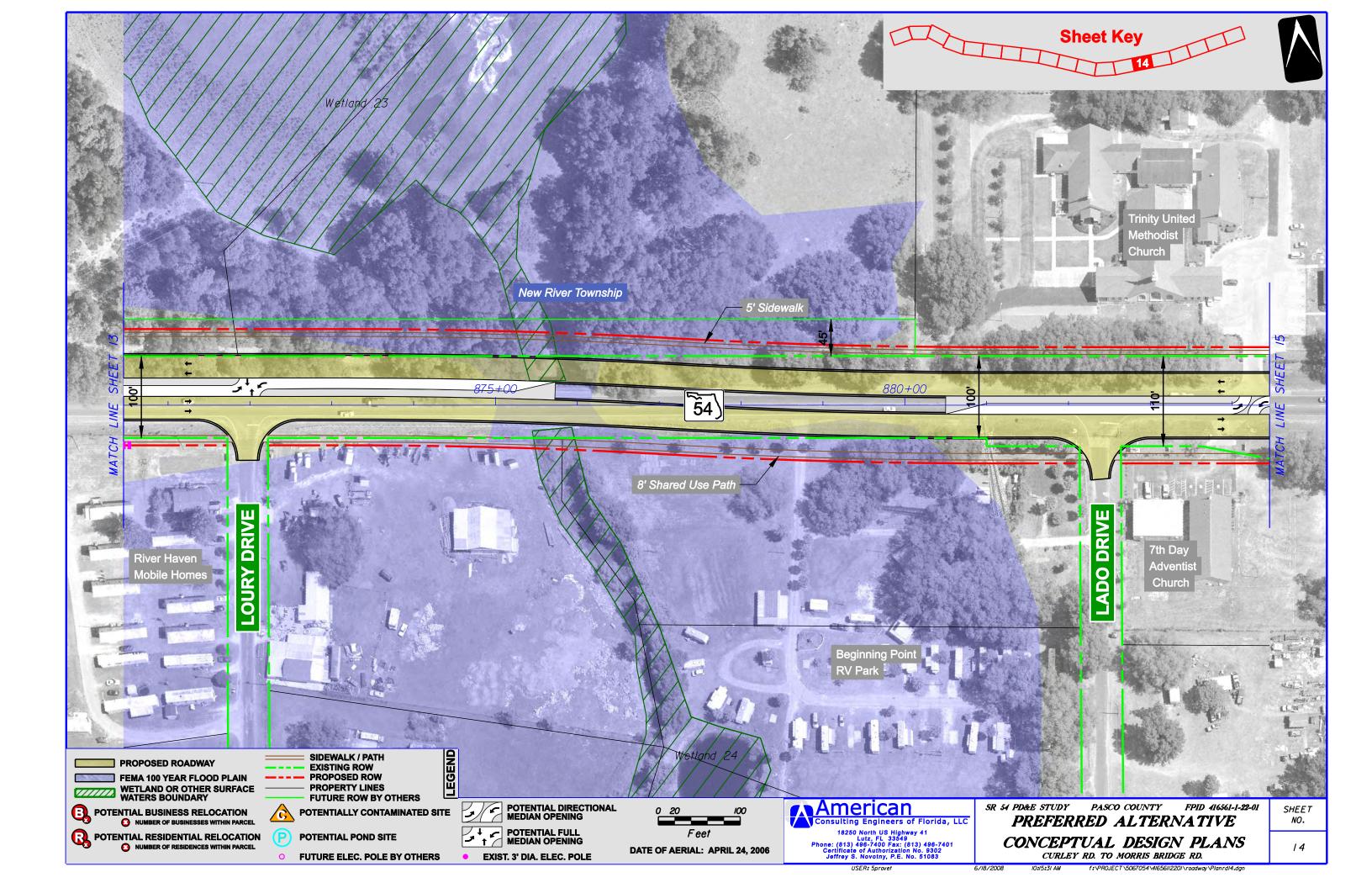


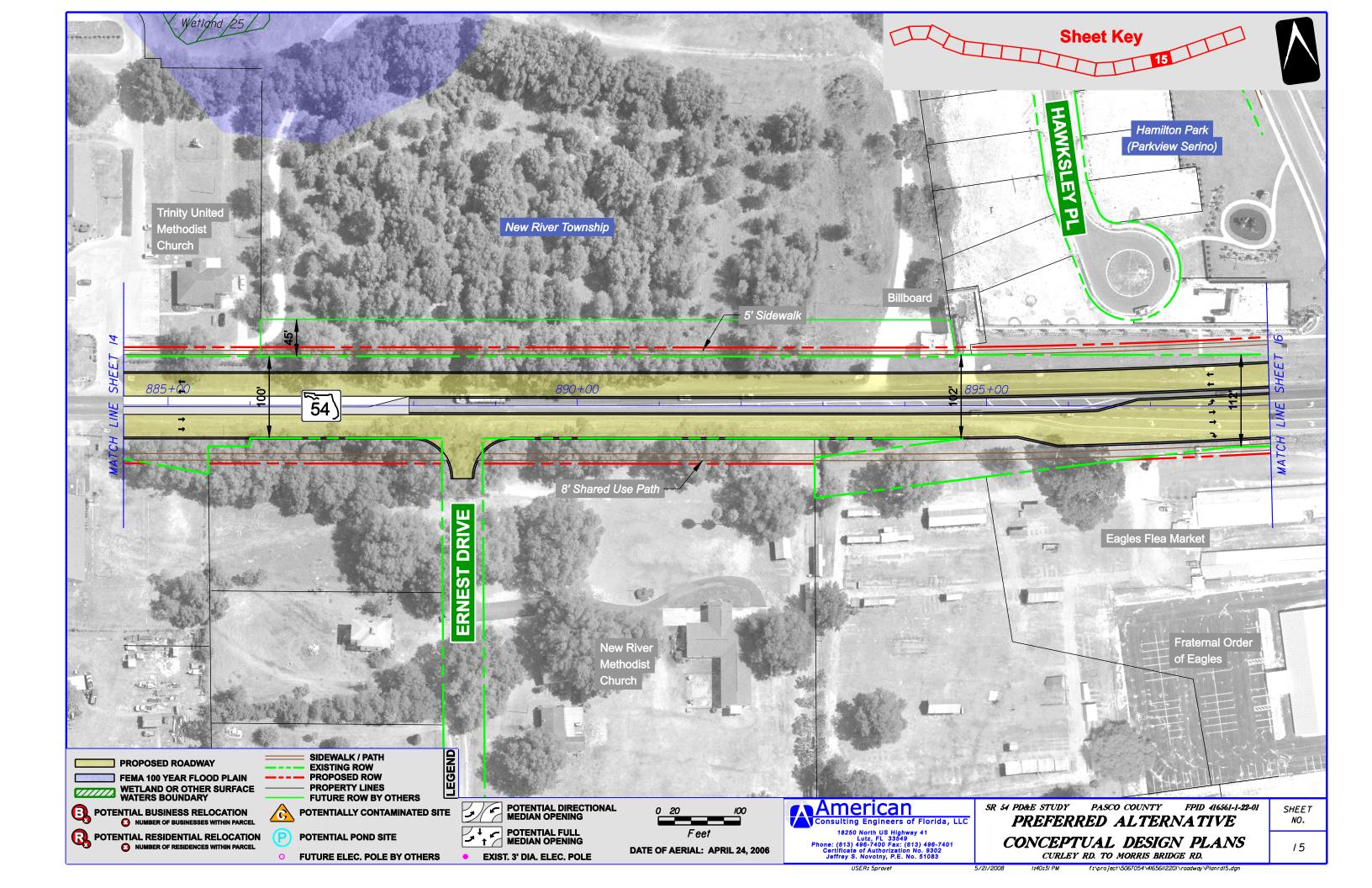


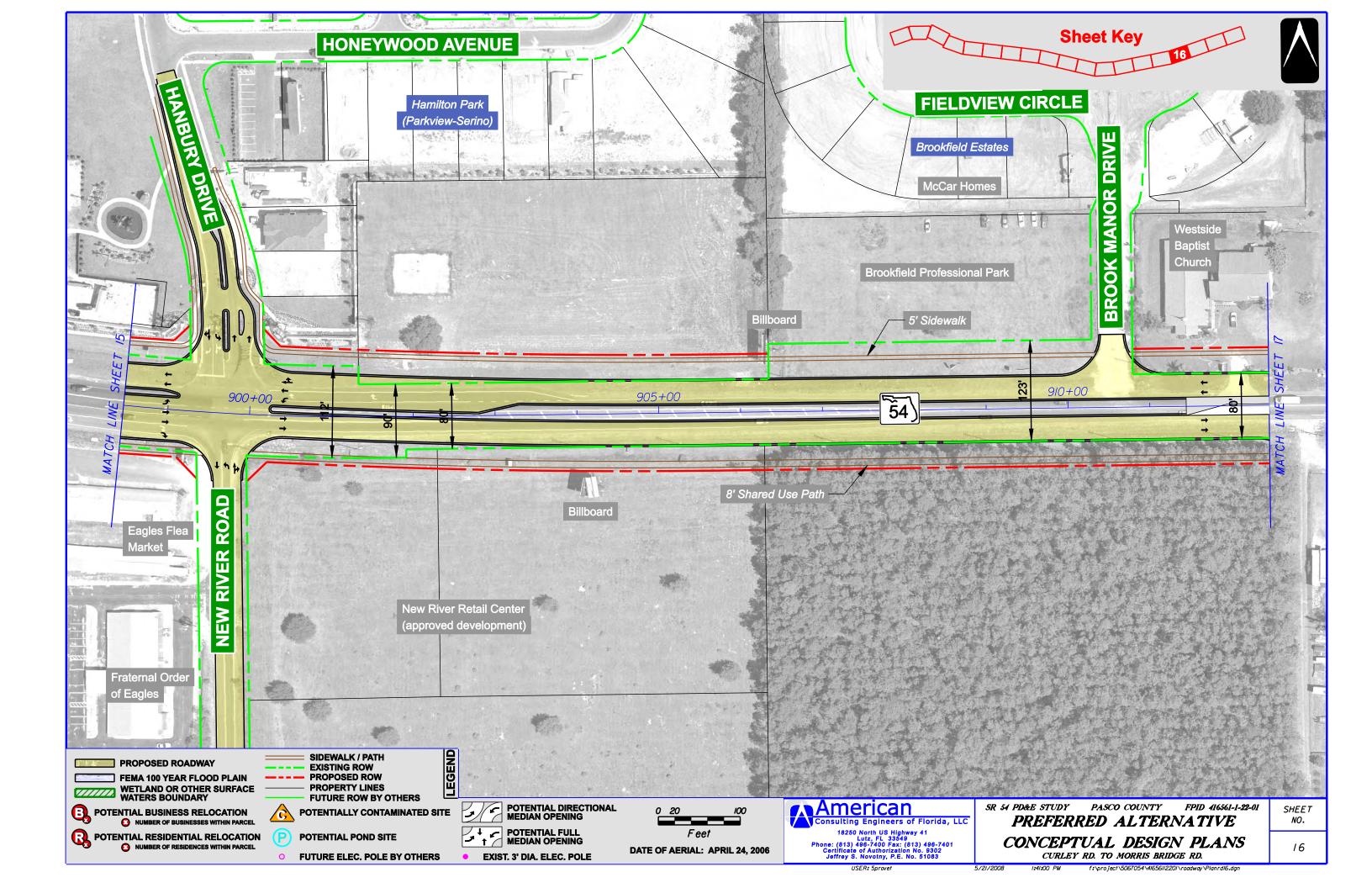


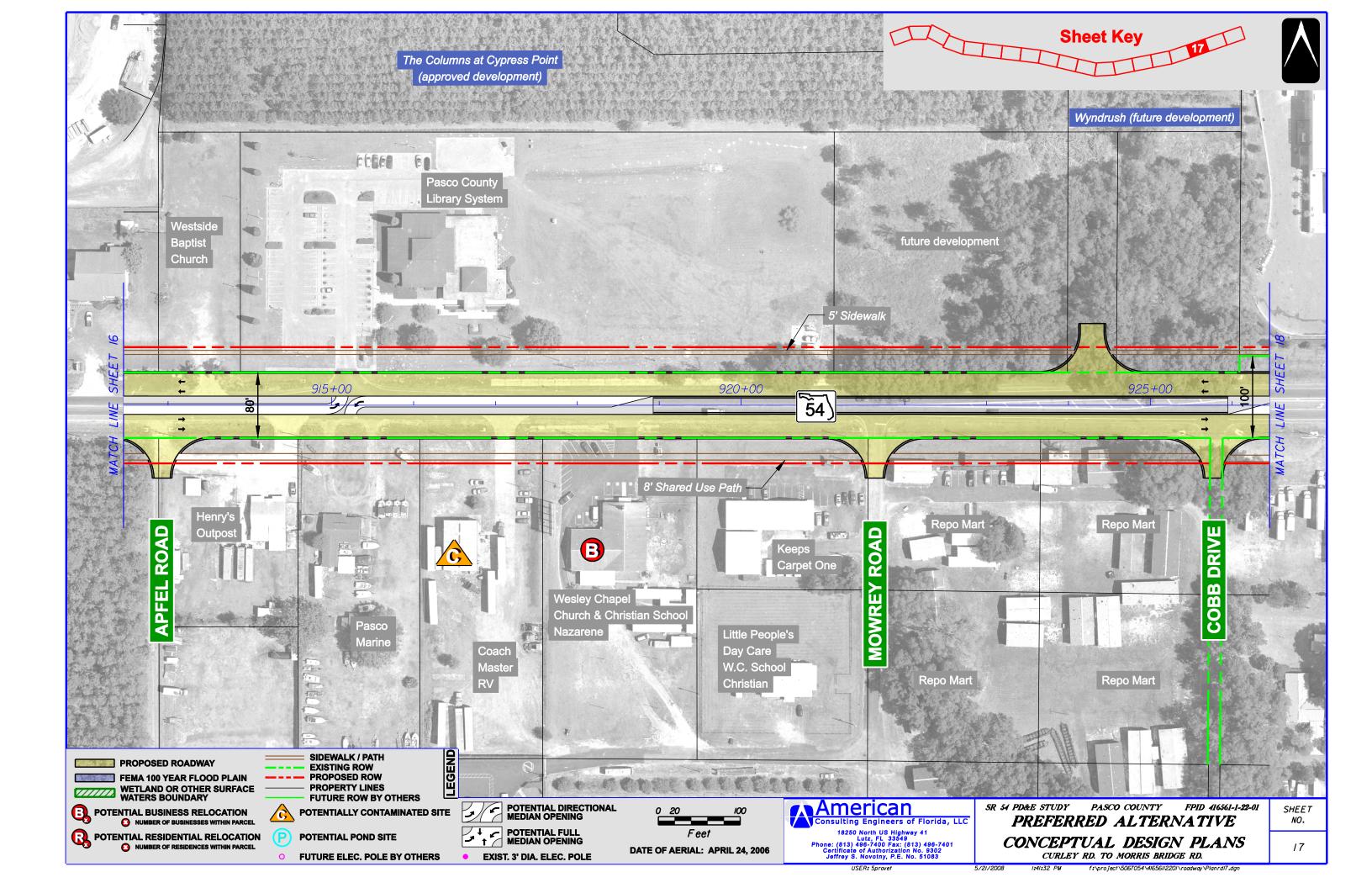


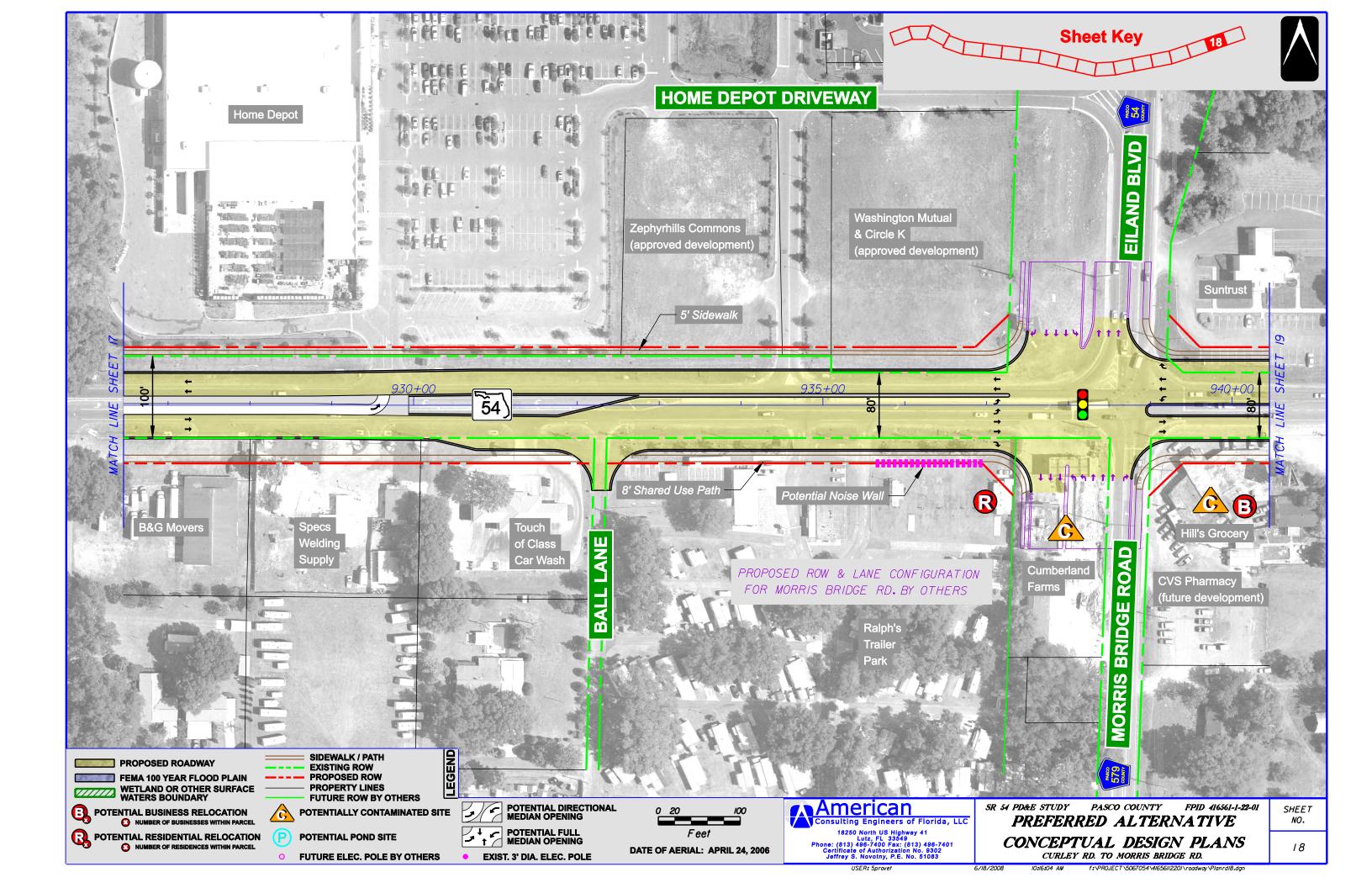


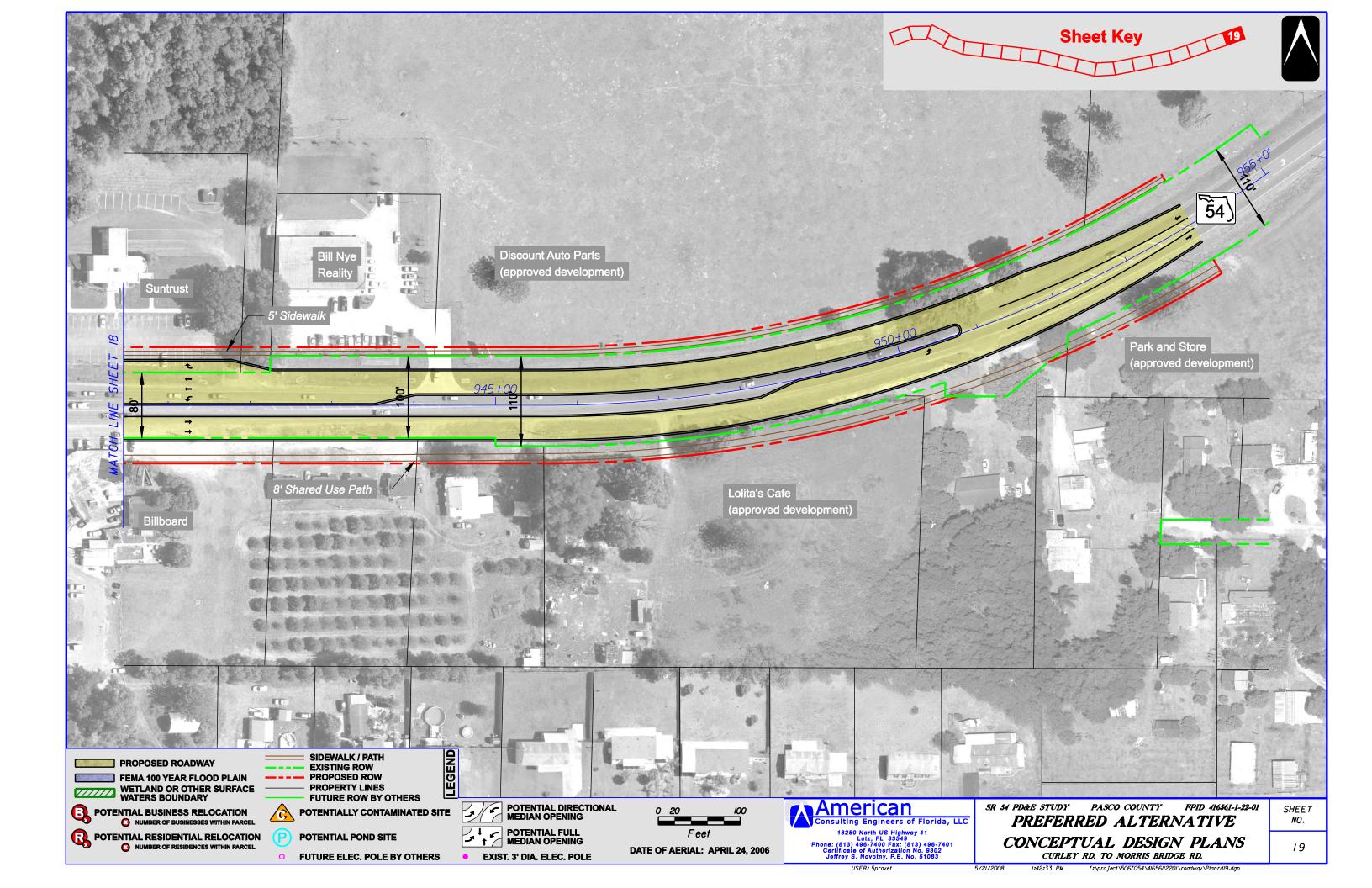












SR 54 Project Development and Environment (PD&E) Study From CR 577 (Curley Road) to CR 579/CR 54 (Morris Bridge Road)

APPENDIX

C

Design Variation for Multiuse Path Width

Final Preliminary Engineering Report

To: Ron Chin, P.E.		Date:	December 17, 20	008
District Design Engin	eer			
Financial Project ID: 416561-1-	22-01	New (Const. (X) RRR	()
Federal Aid Number:			, ,	. ,
Project Name: SR 54 Fro	m CR 577 (Curley Rd)	to CR 579/CR 54 (Me	orris Bridge Rd/Ei	land Bivd)
State Road Number: 54		Co./Sec./Sub.:	14090	
Begin Project MP: 11.611		End Project MP:	16.356	
Full Federal Oversight: Yes ()	No ()			
Request for Design Exception: (),	Design Variation: (X)		
	(For Design	Exception or Variation	ns Requiring Centra	l Office Approval)
	Re-submittal:	Yes () No(X)	Original Ref#:	
Requested for the following element	:(s):		-	
() Design Speed () L	ane Widths	() Shoulder Width	s () Bridge V	Vidths
() Structural Capacity () \	/ertical Clearance	() Grades	() Cross S	Slope
() Superelevation () H	Horizontal Alignment	() Vertical Alignme	ent () Stoppin	ng Sight Distance
() Horizontal Clearance (X)	Other Shared-Use P	ath Width		
SR 54 in Pasco County is currently a two-lane rural facility with one segment classified as a Rural Principal Arterial-Other and two segments classified as Urban Principal Arterial Other. The proposed improvements include widening to a four lane arterial from Curley Road to east of Morris Bridge Road. A 4-lane urban section with auxiliary lanes and 4 foot bike lanes is proposed from Curley Road to Foxwood Boulevard; a 4-lane suburban section is proposed from Foxwood Boulevard to Linda Drive, and a 4-lane urban section with 4 foot bike lanes is proposed from Linda Drive to east of Morris Bridge Road. There are currently no pedestrian facilities on SR 54. Proposed improvements include adding a 5 foot sidewalk on the north side of SR 54 and a minimum 8-foot shared use path on the south side of SR 54 for the entire length of the project. A design variation is being requested for the reduced shared-use path minimum width of 8 feet from the FDOT requirement of 12 feet. This is to be consistent with the Department's previous commitments to Pasco County to be consistent with their typical sections and the Department's unwritten understanding that this roadway will be turned over to Pasco County once SR 56 has been extended eastward to US 301.				
Recommended by: Responsible Professional Engineer Approvals:	Date: 12-18-20	08	1	
Konald El. Cler	Date: 12/27/08	N	/A	Date:
District Design Engineer	′ /	District Structures	Design Engineer	
N/FI	Date:	N	111	Date:
State Roadway Design Engineer	-	State Structures	Design Engineer	
N/A	Date:			Date:
State Chief Engineer		FHWA Division	n Administrator	

Design Variation: Shared-Use Path Width

INTRODUCTION

This Design Variation is being requested in conjunction with the PD&E Study for the widening of SR 54 between Curley Road and Morris Bridge Road. The total length of the proposed project limits is approximately 4.5 miles. The segment of SR 54 to the west, from I-75 to east of Curley Road (CR 577), is currently programmed by Pasco County for widening to six lanes. That project also includes a connection to the planned Zephyrhills West Bypass Extension.

The existing SR 54 facility is functionally classified by FDOT as:

- "Urban Principal Arterial Other" from west of the project limits to Smith Rd
- "Rural Principal Arterial Other" from Smith Rd to west of New River
- "Urban Principal Arterial Other" from west of New River to east of the project limits

Traffic signals currently exist (or will be in operation) at Curley Road, Meadow Pointe Boulevard, River Glen Boulevard/Wyndfields Boulevard, and Morris Bridge Road. The existing right-of-way typically varies between 80 ft and 100 ft. The existing highway is classified by FDOT as Access Management Class 3.

EXISTING CONDITION (Typical Section and Speed Limits)

The existing typical section is a two-lane undivided rural roadway with 12-foot travel lanes and 12-foot shoulders (5-foot paved). Several areas have been widened to provide left-turn and right-turn lanes. From west to east, the posted speed limit varies from 55 miles per hour (mph) to 50 mph, changing in the vicinity of Linda Drive.

PROPOSED IMPROVEMENTS

Proposed typical sections from the PD&E Study are shown in **Figure 1**. All travel lanes are 12 feet in width, and all typical sections include a 5-foot sidewalk on the north side and a 8-foot (minimum width) multiuse trail on the south side.

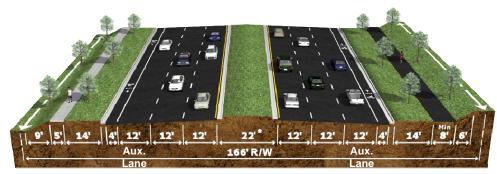
Between Curley Road and Foxwood Boulevard, the proposed typical section is a four-lane divided urban roadway with two auxiliary lanes and 4-foot bike lanes. The proposed right-of-way is 166 feet.

Between Foxwood Boulevard and Linda Drive, a 4-lane divided suburban typical section with 5-foot paved outside shoulders is proposed. The proposed right-of-way is 166 feet.

Between Linda Drive and Morris Bridge Road, a 4-lane divided urban typical section with 4-foot bike lanes is proposed. The proposed right-of-way is 142 feet.

The border areas are wider than FDOT's standard typical sections and would be generally consistent with Pasco County's standard typical section border areas (**Figure 2**) which include a 5-ft sidewalk on one side and a minimum 8-ft path on the other side.

(Looking east for all sections)



Four-Lane Divided with Auxiliary Lanes Urban Typical Section From Curley Road to Foxwood Blvd

Design Speed = 45 mph



Four-Lane Divided Suburban Typical Section From Foxwood Blvd to Linda Drive

Design Speed = 55 mph



Four-Lane Divided Urban Typical Section From Linda Drive to Morris Bridge Road

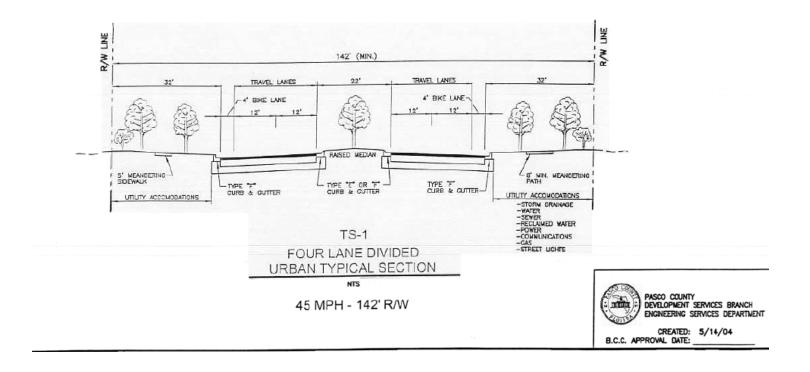
Design Speed = 45 mph

*For the few areas where a 30' median would be required for dual left turn lanes at signalized intersections, the outside border areas would be reduced by 4' on each side to provide the extra median width required.

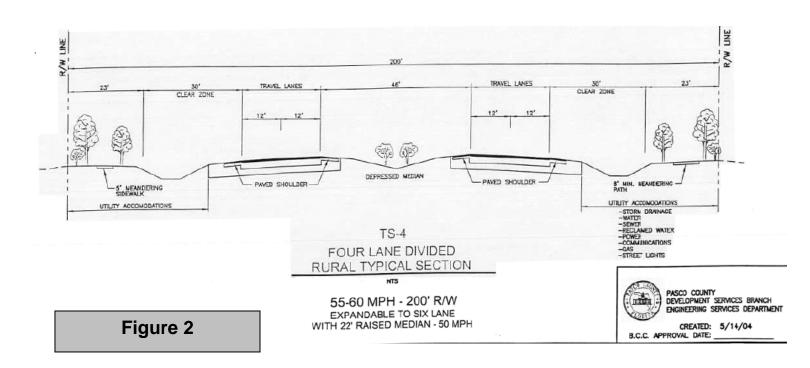
Rev. 5/13/08







(Note: the border areas for TS-2 (Six Lane Divided Urban Typical Section) look the same as those of TS-1)



Design Variation: Shared-Use Path Width

DESIGN VARIATION

This Design Variation request is for a reduction of the minimum shared-use path width (outlined in the FDOT Plans Preparation Manual - Chapter 2 and summarized in the Design Criteria section below). The proposed path width requested is (a minimum of) 8 feet for the length of this project.

DESIGN CRITERIA

FDOT Criteria for Median Width

Per the PPM Vol. I Section 8.6.2, the width of a shared use path (for two-way "traffic") shall be **12** ft.

REASON CRITERIA IS NOT APPROPRIATE

While the FDOT PPM Volume I states that a shared use path must be 12 foot wide, other applicable design criteria does allow the use of a 10 foot wide shared use path with a minimum 8 foot width. AASHTO "Guide for the Development of Bicycles Facilities", 1999 Edition, page 35 states:

"...Under most conditions, a recommended paved width for a two-directional shared use path is 3.0 m (10 ft). In rare instances, a reduced width of 2.4 m (8 ft) can be adequate".

Also, the Florida "Green Book" Chapter 9, Section C.2 Width, states:

"The paved width and operating width required for a shared use path are primary design considerations. The minimum recommended width for a paved two-way path is 10 ft. ... In a few cases, it may be acceptable to decrease the trail width to 8 feet. This width should only be used where the following conditions prevail:

- Bicycle traffic is expected to be low, even on peak days or during peak hours.
- Pedestrian use of the facility is not expected to be more than occasional.
- There will be good horizontal and vertical alignment, providing safe and frequent passing opportunities.
- During normal maintenance activities, the path will not be subjected to maintenance vehicles causing pavement edge damage."

JUSTIFICATION & BACKGROUND INFORMATION

Pasco County has adopted standard roadway typical sections to achieve uniformity for the County's multi-lane collector and arterial roadways. The Department made commitments to Pasco County previously during the PD&E Study to provide typical sections which are consistent with the county's standard typical sections. The rationale for this is that there is an unwritten understanding between the Department and the county that this segment of SR 54 will be turned over to the county at such time that SR 56 is extended eastward to US 301. This understanding was stated during the PD&E Study in several meetings by Mr. Bob Clifford, Department Head of Intermodal Systems Development at District Seven.

Design Variation: Shared-Use Path Width

Applicable criterion (both AASHTO and Florida Green Book) do allow for the use of an 8-foot path. Therefore, a variation is justified as part of 416561-1 for the reasons indicated above and below.

- (1) AASHTO "Guide for the Development of Bicycle Facilities" allows that a shared use path can be a minimum of 8 feet wide,
- (2) Florida Green Book states that it may be acceptable to decrease the trail width to 8 feet.
- (3) Pasco County Standard Roadway Typical Sections for Collector and Arterial Roadways (Resolution #04-212 adopted by the BOCC on June 29, 2004).

Since the proposed typical sections include either designated bike lanes or paved shoulders, the more experienced cyclists will be expected to use these facilities, which will result in lower bicycle volumes on the multiuse trail. Construction of a narrower path would also, of course, result in some savings in construction costs. If this narrower path were applied to the entire project, the estimated savings could be approximately \$140,000.

OTHER DESIGN VARIATIONS

Not Applicable

Recommended by:

Jeffrey Novotny, P.E. No. 51083 Responsible Professional Engineer

Date: /2/18/

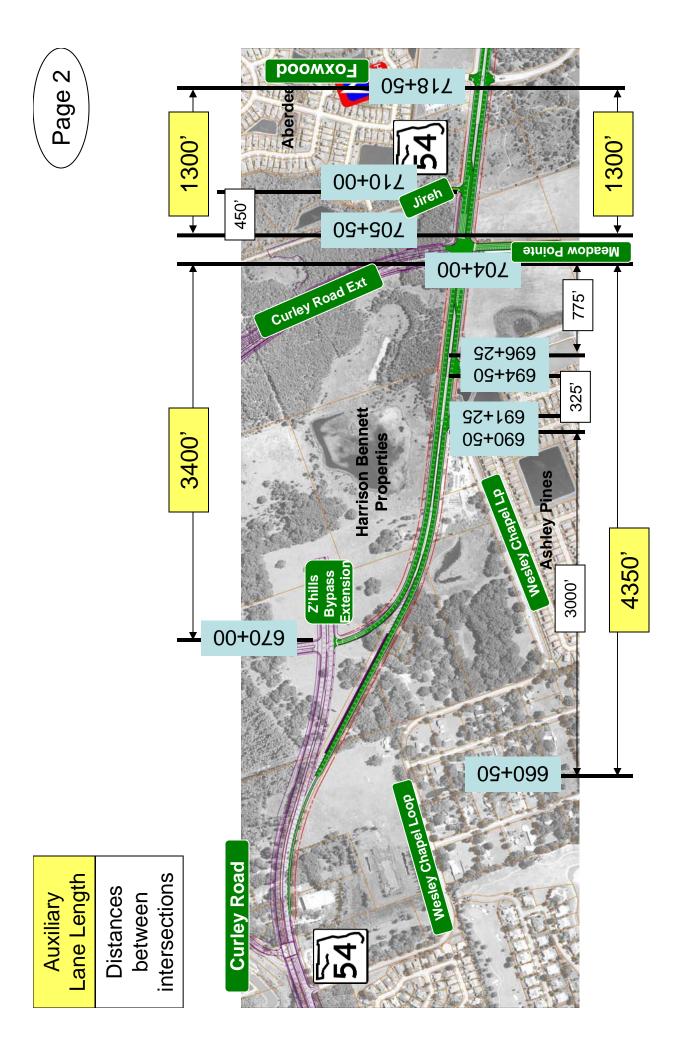
American Consulting Engineers of Florida, LLC

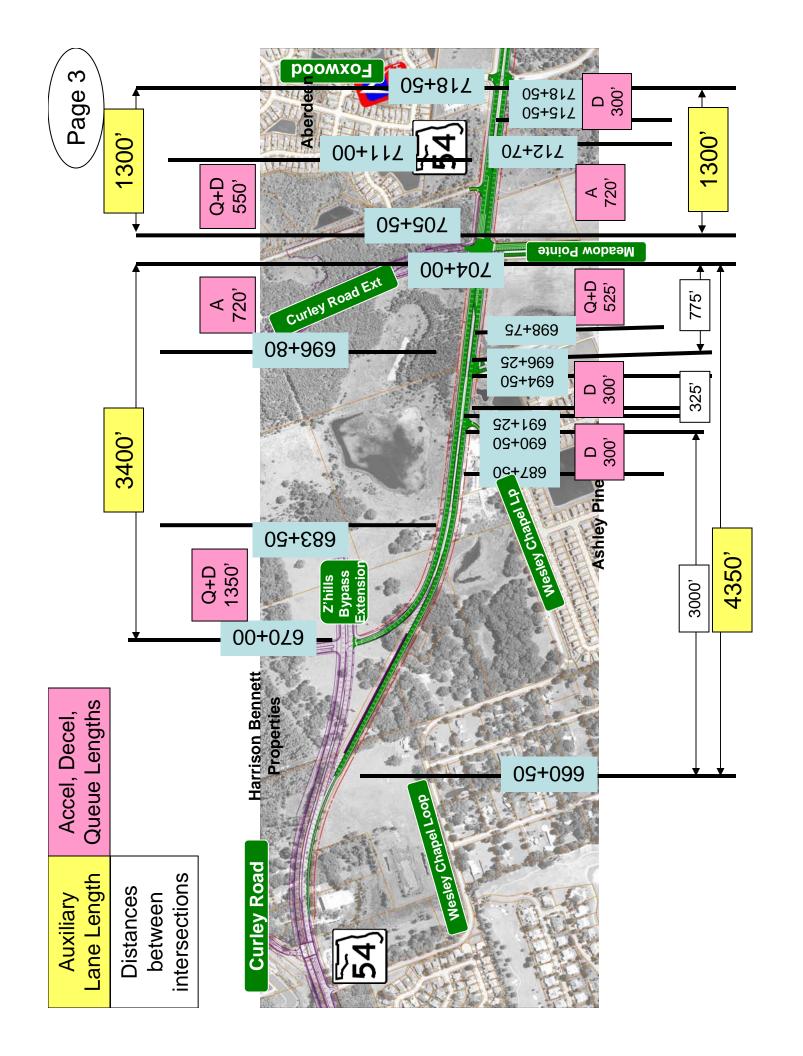
SR 54 Project Development and Environment (PD&E) Study From CR 577 (Curley Road) to CR 579/CR 54 (Morris Bridge Road)

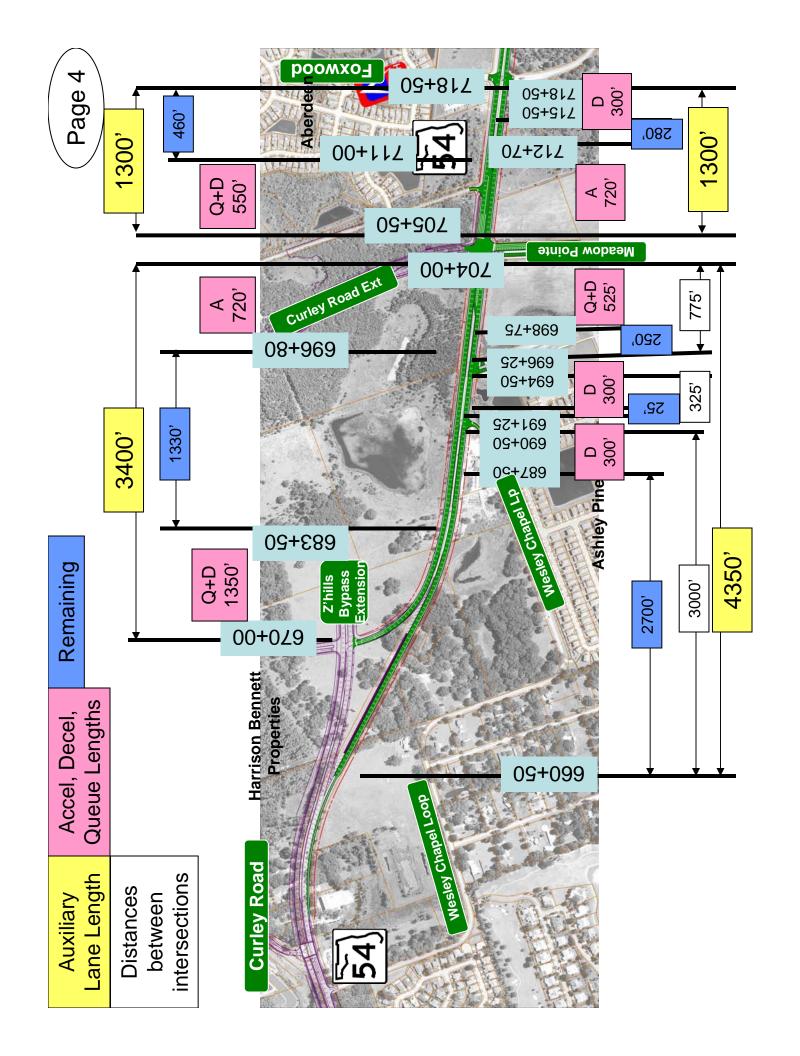
APPENDIX D

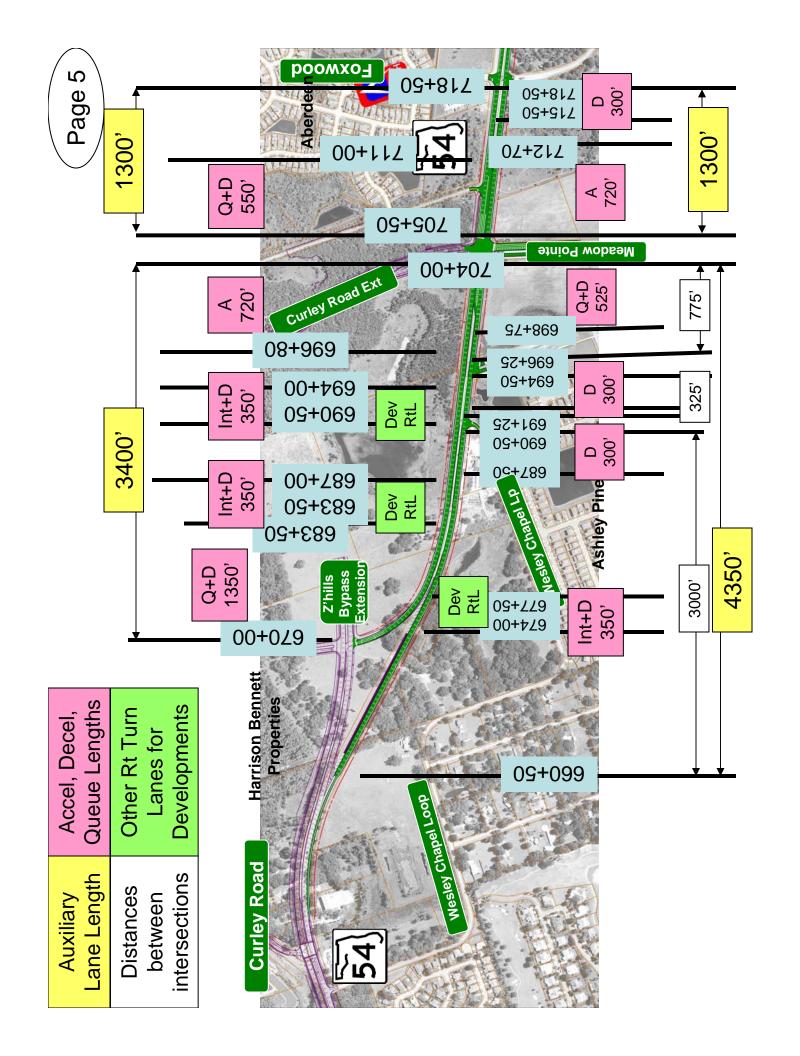
Auxiliary Lane Length Documentation

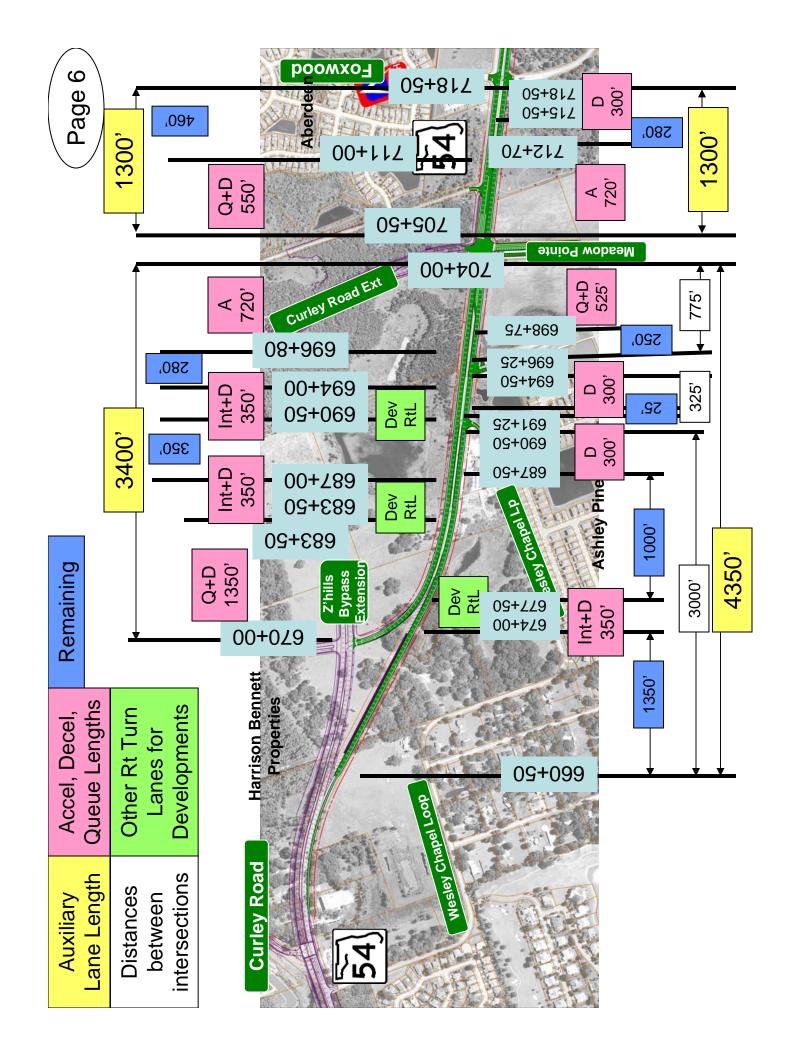
Final Preliminary Engineering Report



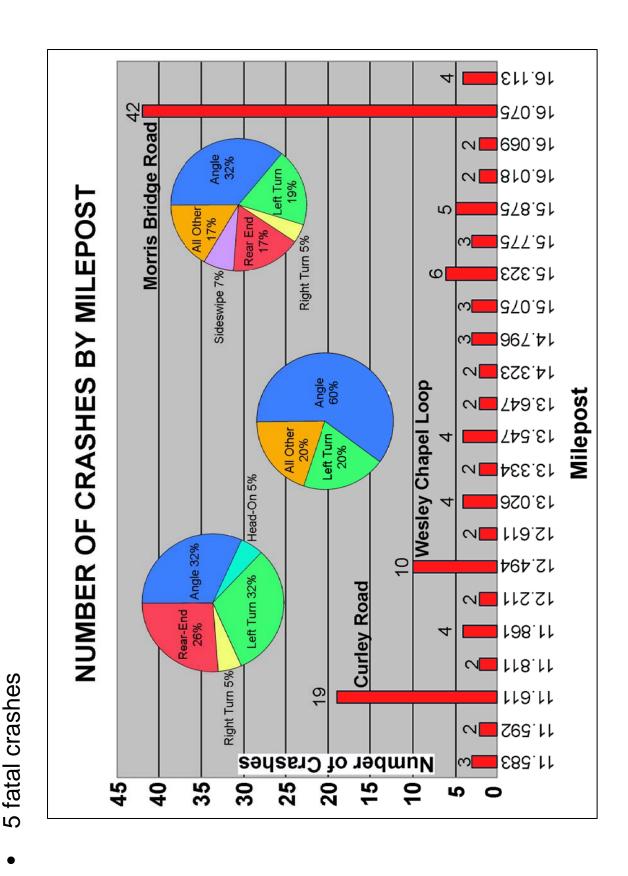








5-year number of crashes = 155



SR 54/Meadow Pointe Intersection Level of Service (2030) am/pm

		ا ــا	North Leg	South	South Overall Leg Inter-
Aux Lane	(VVD)	(СБ)	(90)	(IND)	
	D/D	D /E	E/D	D / E	D/D
2Th-1Rt	F/D	E/F	E/D	D/E	E/E

	Metric								US Customary					
Design		Stoppi	ng sigh	t dista	nce (m	1)	Design	Stopping sight distance (ft))		
speed	Do	wngra	des	U	pgrad	es	speed	Do	owngra	des	U	pgrade	9 S	
(km/h)	3 %	6 %	9 %	3 %	6 %	9 %	(mph)	3 %	6 %	9 %	3 %	6 %	9 %	
20	20	20	20	19	18	18	15	80	82	85	75	74	73	
30	32	35	35	31	30	29	20	116	120	126	109	107	104	
40	50	50	53	45	44	43	25	158	165	173	147	143	140	
50	66	70	74	61	59	58	30	205	215	227	200	184	179	
60	87	92	97	80	77	75	35	257	271	287	237	229	222	
70	110	116	124	100	97	93	40	315	333	354	289	278	269	
80	136	144	154	123	118	114	45	378	400	427	344	331	320	
90	164	174	187	148	141	136	50	446	474	507	405	388	375	
100	194	207	223	174	167	160	55	520	553	593	469	450	433	
110	227	243	262	203	194	186	60~	598	638⊱	686	538	515	495	
120	263	281	304	234	223	214	65	682	728	785	612	584	561	
130	302	323	350	267	254	243	70	771	825	891	690	658	631	
							75	866	927	1003	772	736	704	
-		***					80	965	1035	1121	859	817	782	

Exhibit 3-2. Stopping Sight Distance on Grades

Decision Sight Distance

Stopping sight distances are usually sufficient to allow reasonably competent and alert drivers to come to a hurried stop under ordinary circumstances. However, these distances are often inadequate when drivers must make complex or instantaneous decisions, when information is difficult to perceive, or when unexpected or unusual maneuvers are required. Limiting sight distances to those needed for stopping may preclude drivers from performing evasive maneuvers, which often involve less risk and are otherwise preferable to stopping. Even with an appropriate complement of standard traffic control devices in accordance with the MUTCD (6), stopping sight distances may not provide sufficient visibility distances for drivers to corroborate advance warning and to perform the appropriate maneuvers. It is evident that there are many locations where it would be prudent to provide longer sight distances. In these circumstances, decision sight distance provides the greater visibility distance that drivers need.

Decision sight distance is the distance needed for a driver to detect an unexpected or otherwise difficult-to-perceive information source or condition in a roadway environment that may be visually cluttered, recognize the condition or its potential threat, select an appropriate speed and path, and initiate and complete the maneuver safely and efficiently (7). Because decision sight distance offers drivers additional margin for error and affords them sufficient length to maneuver their vehicles at the same or reduced speed, rather than to just stop, its values are substantially greater than stopping sight distance.

Drivers need decision sight distances whenever there is a likelihood for error in either information reception, decision making, or control actions (8). Examples of critical locations where these kinds of errors are likely to occur, and where it is desirable to provide decision sight distance include interchange and intersection locations where unusual or unexpected maneuvers are required, changes in cross section such as toll plazas and lane drops, and areas of concentrated

demand where there is apt to be "visual noise" from competing sources of information, such as roadway elements, traffic, traffic control devices, and advertising signs.

The decision sight distances in Exhibit 3-3 (1) provide values for sight distances that may be appropriate at critical locations, and (2) serve as criteria in evaluating the suitability of the available sight distances at these locations. Because of the additional safety and maneuvering space provided, it is recommended that decision sight distances be provided at critical locations or that critical decision points be moved to locations where sufficient decision sight distance is available. If it is not practical to provide decision sight distance because of horizontal or vertical curvature or if relocation of decision points is not practical, special attention should be given to the use of suitable traffic control devices for providing advance warning of the conditions that are likely to be encountered.

		Met	ic					US Cus	tomary		
Design	sight di	stance	(m)	Design		Decisior	sight dis	stance (f	t)		
speed		Avoida	nce ma	aneuvei	•	speed		Avoid	ance ma	neuver	
(km/h)	Α	В	С	D	Е	(mph)	Α	В	C	D	/E
50	70	155	145	170	195	30	220	490	450	535	620
60	95	195	170	205	235	35	275	590	525	625	720
70	115	235	200	235	275	40	330	690	600	715	825
80	140	280	230	270	315	45	395	800	675	800	930
90	170	325	270	315	360	50	465	910	750	890	1030
100	200	370	315	355	400	55	535	1030	865	980	1135
110	235	420	330	380	430	60	610	1150	990	1125	1280
120	265	470	360	415	470	65	695	1275	1050	1220	1365
130	305	525	390	450	510	70	780	1410	1105	1275	1445
						75	875	1545	1180	1365	1545
Assides	. 14	Δ.				80	970	1685	1260	1455	1650

Avoidance Maneuver A: Stop on rural road—t = 3.0 sAvoidance Maneuver B: Stop on urban road—t = 9.1 s

Avoidance Maneuver C: Speed/path/direction change on rural road—t varies between 10.2 and 11.2 s

Avoidance Maneuver D: Speed/path/direction change on suburban road—/ varies between 12.1 and 12.9 s

Avoidance Maneuver E: Speed/path/direction change on urban road—/ varies between 14.0 and 14.5 s

Exhibit 3-3. Decision Sight Distance

Decision sight distance criteria that are applicable to most situations have been developed from empirical data. The decision sight distances vary depending on whether the location is on a rural or urban road and on the type of avoidance maneuver required to negotiate the location properly. Exhibit 3-3 shows decision sight distance values for various situations rounded for design. As can be seen in the exhibit, shorter distances are generally needed for rural roads and for locations where a stop is the appropriate maneuver.

For the avoidance maneuvers identified in Exhibit 3-3, the pre-maneuver time is increase above the brake reaction time for stopping sight distance to allow the driver additional time to detect and recognize the roadway or traffic situation, identify alternative maneuvers, and initiate

response at critical locations on the highway (9). The pre-maneuver component of decision sight distance uses a value ranging between 3.0 and 9.1 s (10).

The braking distance from the design speed is added to the pre-maneuver component for avoidance maneuvers A and B as shown in Equation (3-4). The braking component is replaced in avoidance maneuvers C, D, and E with a maneuver distance based on maneuver times between 3.5 and 4.5 s, that decrease with increasing speed (9) in accordance with Equation (3-5).

The decision sight distances for avoidance maneuvers A and B are determined as:

Metric	US Customary
$d = 0.278Vt + 0.039\frac{V^2}{a}$	$d = 1.47 Vt + 1.075 \frac{V^2}{a} $ (3-4)
where:	where:
t = pre-maneuver time, s (see notes in Exhibit 3-3); V = design speed, km/h; a = driver deceleration, m/s ²	 t = pre-maneuver time, s (see notes in Exhibit 3-3); V = design speed, mph; a = driver deceleration, ft/s²

The decision sight distances for avoidance maneuvers C, D, and E are determined as:

Metric	US Customary						
d = 0.278Vt	d = 1.47Vt (3-5)						
where:	where:						
 t = total pre-maneuver and maneuver time, s (see notes in Exhibit 3-3); V = design speed, km/h 	 t = total pre-maneuver and maneuver time, s (see notes in Exhibit 3-3); V = design speed, mph 						

In computing and measuring decision sight distances, the same 1 080-mm [3.5-ft] eye-height and 600-mm [2.0-ft] object-height criteria used for stopping sight distance have been adopted. Although drivers may have to be able to see the entire roadway situation, including the road surface, the rationale for the 600-mm [2.0-ft] object height is as applicable to decision sight distance as it is to stopping sight distance.

William .

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