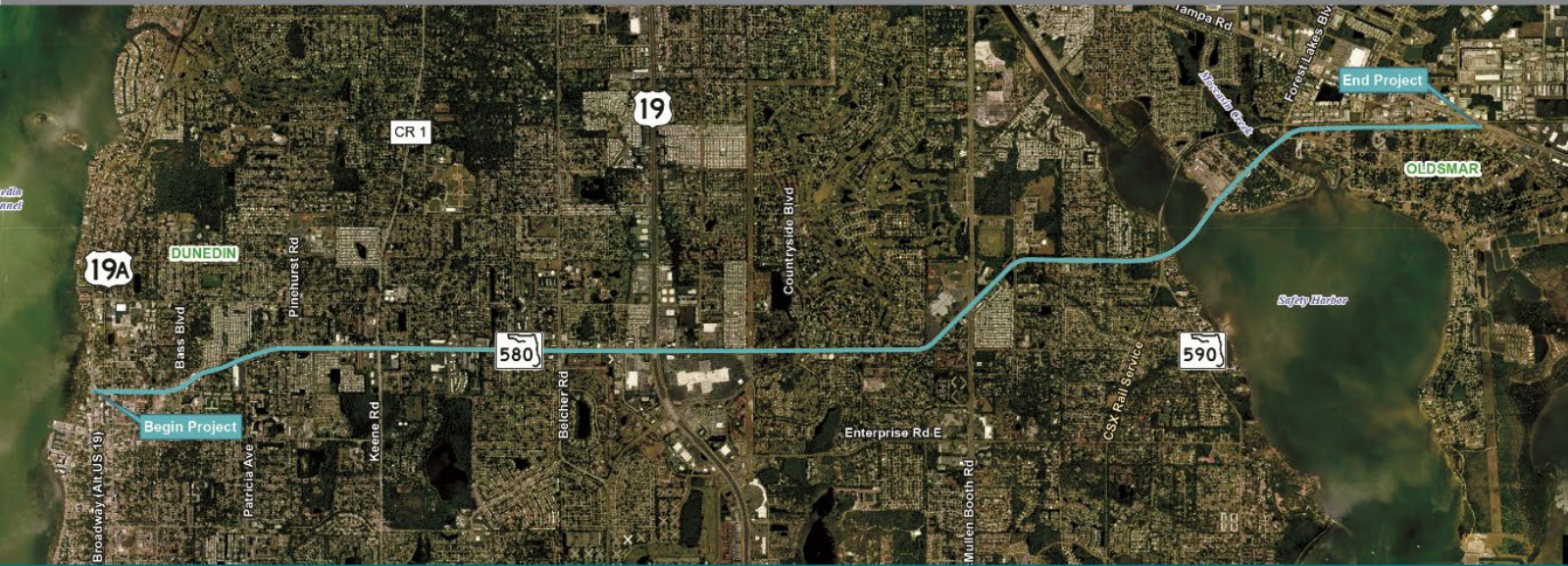




FROM ALT. US 19/SR 595/BROADWAY TO TAMPA ROAD



# NO-BUILD FUTURE CONDITIONS MEMORANDUM



Pinellas County  
FPID: 259109-1-12-27  
January 2022

# TECHNICAL MEMORANDUM

---

## MEMORANDUM PURPOSE

The purpose of this *No-Build Future Conditions Memorandum* is to identify future conditions in the No-Build scenario and identify future needs for the State Road 580 (SR 580) study corridor. This memorandum assesses the future vision, goals and objectives for the corridor, as well as future land use, travel demand and no-build traffic operations.

## PROJECT LOCATION

The study area extends 8.5 miles along SR 580 in Pinellas County Florida from Alternate US 19 to Tampa Road and has several typical sections that vary. Beginning at Alternate US 19 in the City of Dunedin and spanning the cities of Clearwater, Safety Harbor and Oldsmar, SR 580 is a four-lane divided urban arterial for almost a mile then transitions to a four-lane undivided urban arterial to the cross street of Pinehurst Road where an additional through lane is added in each direction of SR 580 for a total of six lanes. The six-lane SR 580 continues east until just west of SR 590/Philippe Parkway where a through lane drops in each direction to a four-lane facility and continues to the end of the project at Tampa Road.

Daily traffic volumes ranged from 10,800 vehicles per day to 48,500 vehicles per day along the corridor in the year 2018. The corridor features sidewalks on both sides of the roadway, with a few exceptions. Bike lanes are present on both sides of SR 580 from Alternate US 19 to Countryside Boulevard only. Vehicle traffic regularly mixes in the corridor with bicycles, pedestrians and transit users for access to community services, businesses, Countryside Mall, Countryside High School, and Oldsmar Elementary School along the corridor.

The project location map is shown on **Figure 1**.

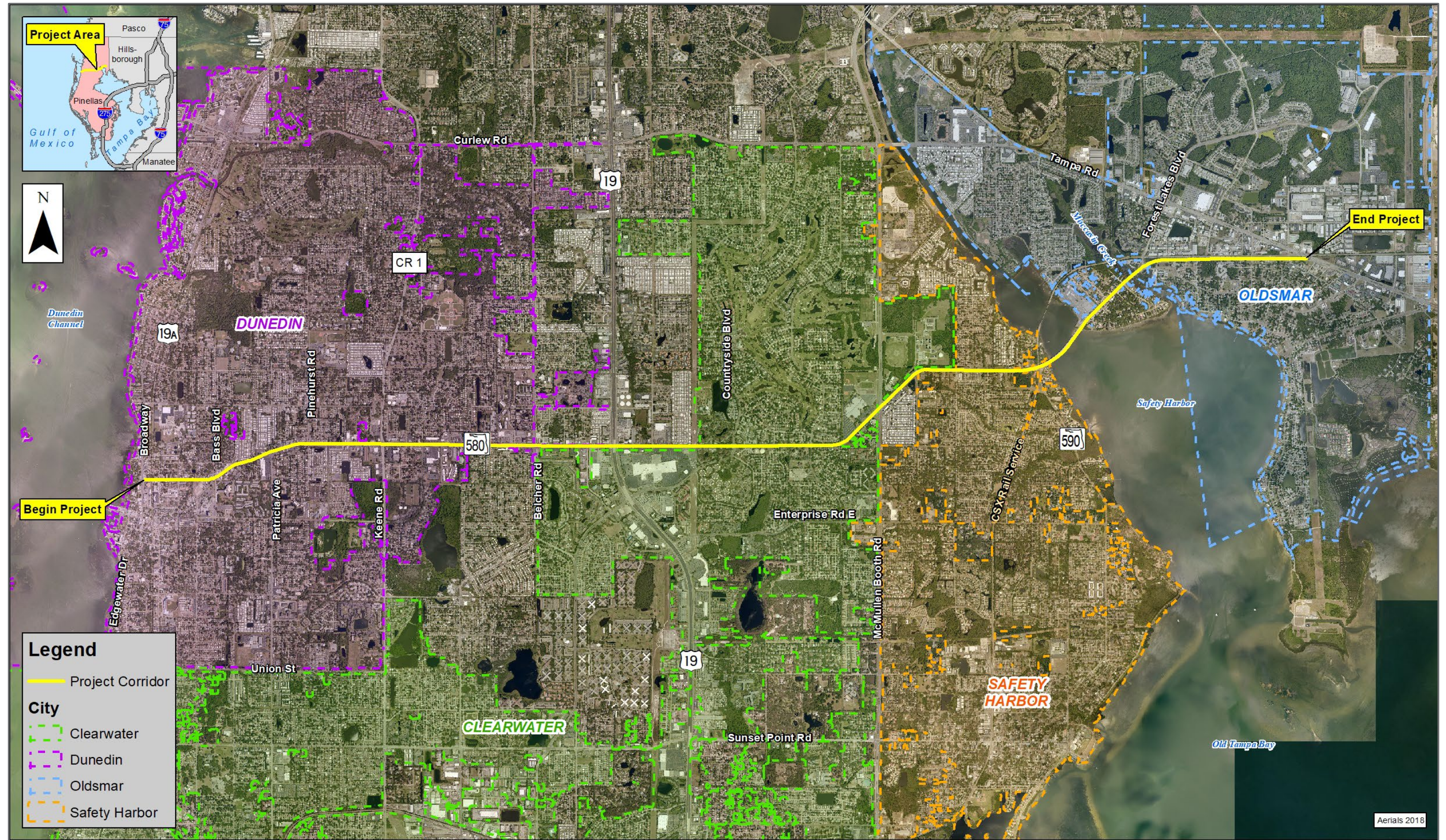


Figure 1 Project Location Map

## ***GUIDING PRINCIPLES***

The guiding principles of the study will establish the vision of the corridor, identify major users of the corridor, and define the desired role of the facility for the future conditions.

### ***Corridor Vision***

SR 580 is a major east west arterial in Pinellas County with several different adjacent land uses that attract and generate different modes of transportation users. The vision for the SR 580 corridor includes providing safe options for pedestrians, bicyclist, transit users and motorized vehicles using the corridor while not deteriorating vehicle operations or increasing delay. This vision will include providing sidewalks and bicycle facilities along the corridor and connections to other multimodal facilities and proposing transportation improvements that reduce vehicle crashes and enhance safety for all users. constraint the existing or proposed Pinellas Suncoast Transit Authority (PSTA) system.

Pinellas County and Forward Pinellas support Vision Zero for the state of Florida. Vision Zero is a policy that establishes a goal of reducing traffic fatalities and serious injuries to zero. Forward Pinellas discusses Vision Zero transportation safety philosophy in their Safe Streets Pinellas effort. All proposed concepts for the study corridor will be aligned with the corridor visions.

### ***Corridor Users***

The SR 580 study corridor major users are listed as:

- Passenger vehicles
- Freight/Heavy trucks
- Pedestrian/Bicyclist
- Transit users (PSTA)
- Local residents
- Tourists

### ***Desired Role of Facility***

The SR 580 corridor is classified as an urban arterial and is a major east/west corridor in Pinellas County, Florida. The adjacent land use is mainly residential with some mixed use, commercial, institutional, industrial and transportation/utility. This adjacent land use of the roadway generates vehicular, bicyclists, pedestrians and transit users. Proposed improvements that support the role of the facility will be developed and evaluated in a Corridor Alternatives and Strategies Report.

## ***NEEDS ASSESSMENT***

### ***Goals And Objectives for The Corridor***

Goals and objectives for the SR 580 study corridor have been established through identifying existing deficiencies of the corridor, guiding principles, developing a purpose and need statement and through

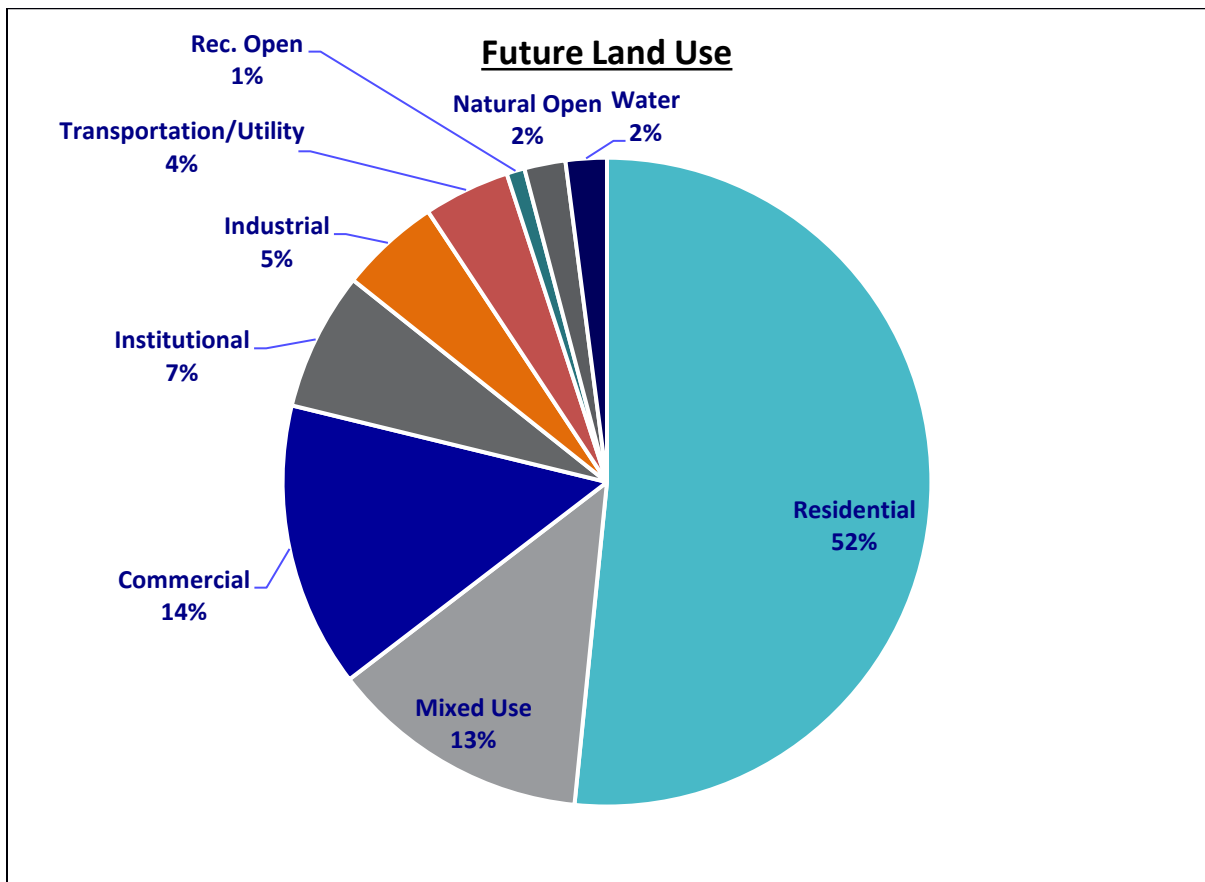
on-going coordination with the public. The future adjacent land use, population and employment growth, increased multimodal users, safety and level of service (LOS) concerns have all led to the need of multimodal solutions to address mobility and safety concerns. The overall goal of the study is to develop context sensitive improvements to help transform SR 580 into a multimodal urban corridor in keeping with the community context it traverses.

### ***Evaluation Criteria***

The evaluation criteria and measures of effectiveness to be utilized for the comparative evaluation of improvement options has been determined to ensure that the needs, goals and objectives for the study corridor are met. Traffic operational measures of effectiveness that will be used to evaluate intersection improvements will be LOS and delay (seconds) for vehicle traffic using SYNCHRO software. Segment or link evaluation will use LOS results from SYNCHRO. Bicycle and pedestrian LOS will also be determined for future conditions for the study corridor and used in the evaluation criteria.

## FUTURE LAND USE

SR 580 is located in a heavy urbanized area. Pinellas County has designated SR 580 as a Long-Range Investment Corridor due to anticipated substantial redevelopment (specifically near the Countryside Mall). Future land use maps from Pinellas County and local municipalities (City of Dunedin, Clearwater, Safety Harbor and Oldsmar) were used to evaluate future land use changes along the SR 580 corridor. Future residential land use is forecasted to drop from 58% to 52%, commercial land use is forecasted to drop from 21% to 14% and mixed land use is expected to increase from 0% to 13%. **Figure 2** shows a breakout of the future land use for the SR 580 study corridor.



**Figure 2** Future Land Use

**Figure 3** is a map of the existing and future land use map for the entire SR 580 study corridor. From this map it can be seen that the future mixed use parcels are located towards the west end in the City of Dunedin, on the north side of SR 580 near the US 19 interchange and Countryside Mall and the east end of the corridor in Oldsmar.

Changes to the corridor are supported by local municipalities through their local Comprehensive Plans, Future Land Use and Transportation elements of the plan. The City of Clearwater Future Land Use Element Objective A.6.5 states, "The City shall encourage improved land use compatibility through the

evaluation of traffic calming techniques, multi-modal transportation networks, and the use of transit oriented development planning.” One of the policies of this objective states, “All proposed development/redevelopment initiatives shall be reviewed for opportunities to improve pedestrian and bicycle access and consider the integration of bicycle and pedestrian transportation modes in all phases of transportation planning, new roadway design, roadway construction, roadway resurfacing and other capital projects consistent with the City’s *Shifting Gears Bicycle and Pedestrian Master Plan 2006*.”

At the time this memorandum was completed, the Countryside Mall is planned to remain in the future condition with no documented future land use change. The Countryside Mall contains commercial and recreational land use, generating traffic from vehicles, bicyclist, pedestrians and transit.

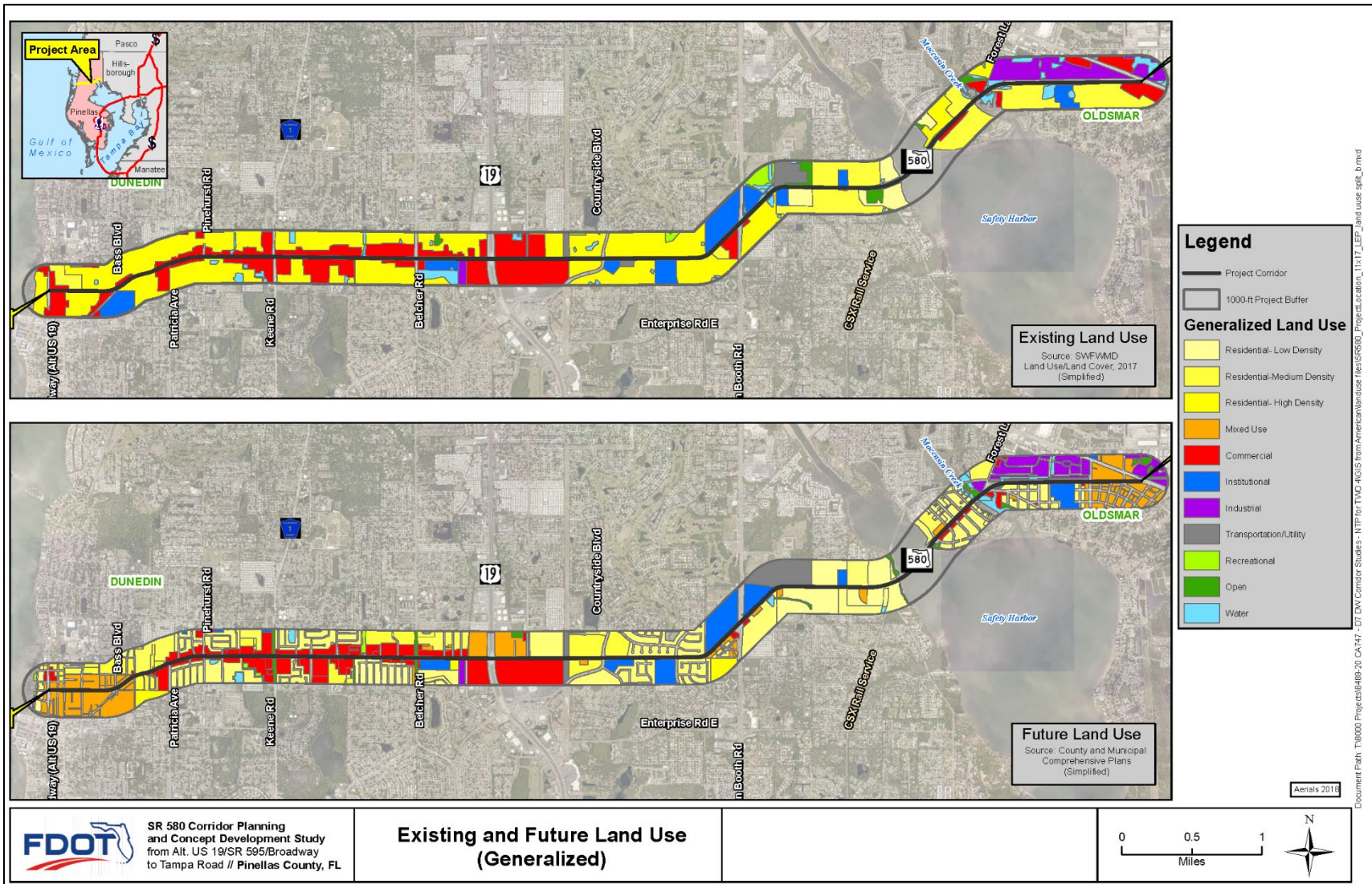


Figure 3 Existing and Future Land Use Map of SR 580



## **PROPOSED DEVELOPMENTS AND PLANNED IMPROVEMENTS**

The Forward Pinellas 2045 Long Range Transportation Plan (LRTP) is a strategic plan to improve mobility and economic opportunity countywide and is updated every five (5) years. The 2045 LRTP identifies one (1) project in the 2045 Cost Feasible Roadway Projects within the SR 580 study corridor. The county project is listed fourth and is a widening project to four (4) lanes divided for Forest Lakes Boulevard from SR 580 to SR 584. Coordination will be needed with Pinellas County for any proposed improvements at the SR 580 and Forest Lakes Boulevard intersection that arise from this study.

The Duke Energy Trail crosses SR 580 east of Countryside Mall and west of Countryside Boulevard. Pinellas County currently has a current project to provide a signalized crossing for pedestrians using this trail and crossing SR 580. This signalized trail crossing is assumed to be in place for the study analysis year of 2045.

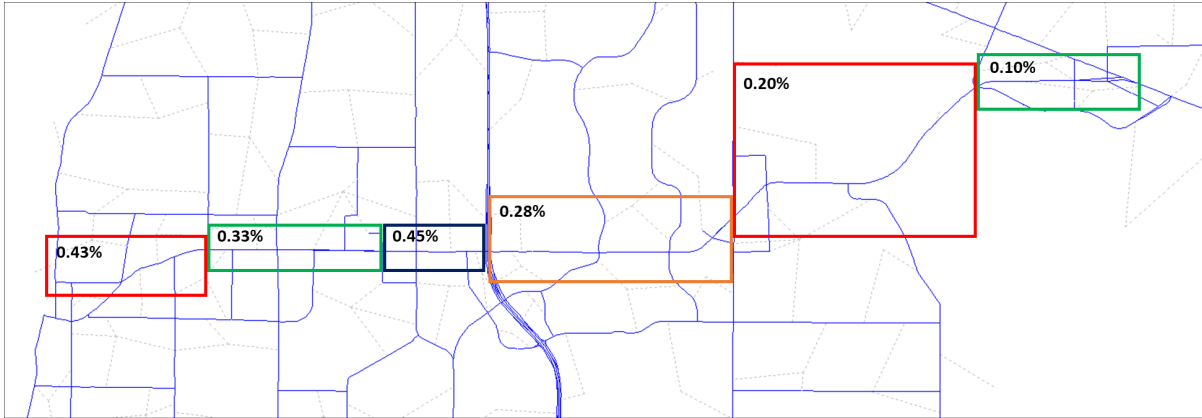
The City of Dunedin is creating a City Multimodal Transportation Plan and currently has a project at the western end of the corridor that begins at Alternate US 19 and ends at Main Street/Bass Boulevard. The project includes proposed lane elimination along SR 580/Skinner Boulevard and roundabouts at the intersection of Douglas Avenue and Highland Avenue/Martin Luther King Jr. Avenue.

## **GROWTH PROJECTIONS AND FUTURE VOLUMES**

The Tampa Bay Regional Planning Model version 2.1 (TBRPM V9.0) was utilized for the SR 580 study in order to calculate growth rates. The TBRPM V9.0 was calibrated for base year 2015 at a regional level. To improve the model performance within the SR 580 study area, further refinement and calibration was conducted based on the original model.

The growth rate was developed from the validated travel demand models, using base year 2015 and future year 2045 model volumes per segment. The original growth rate was calculated for SR 580 main street segments and all side streets of the 25 study intersections. All growth rate values were reviewed and properly adjusted. Refer to **Figure 4** for the annual growth rate used for SR 580 roadway segments.

For SR 580, 21 out of the 24 SR 580 segments in the model gave a reasonable annual growth rate for the corridor. From the model, three (3) SR 580 segments near the intersection with Tampa Road gave a negative growth rate due to the increased capacity of the nearby Forest Lake Boulevard roadway segment. The future volumes for these segments of SR 580 were attracted to the Forest Lake Boulevard segment in the validated model decreasing the volume of traffic on SR 580 in the future condition. For these three (3) segments of SR 580 it is recommended to use the minimum annual growth rate (0.01%). Refer to **Table 1** for the annual growth rate for SR 580 per segment.



**Figure 4 Annual Growth Rate for SR 580 Segments**

**Table 1 Annual Growth Rate for SR 580 per Segment**

No.	SR 580 Segment	Recommended Growth Rate
1	East of Alt. US 19	0.43%
2	East of Douglass Ave	0.43%
3	East of Bass Blvd	0.43%
4	West of Pinehurst Rd	0.33%
5	East of Pinehurst Rd	0.33%
6	East of Lake Haven	0.33%
7	East of Keene Rd	0.33%
8	East of Achieva Way	0.33%
9	East of Overcash Dr	0.33%
10	West of Belcher Rd	0.45%
11	West of Enterprise Road	0.45%
12	West of US 19 (East of Enterprise)	0.45%
13	East of US 19	0.28%
14	West of Countryside Blvd	0.28%
15	East of Countryside Blvd	0.28%
16	East of Landmark Dr	0.28%
17	Between Charles Ave and HS Bus Loop Entrance	0.28%
18	West of McMullen Booth Rd	0.28%
19	East of McMullen Booth Rd	0.20%
20	West of SR 590	0.20%
21	East of SR 590/2nd St	0.20%
22	East of Forest Blvd/St. Petersburg Drive	0.10%
23	East of S Bayview Blvd	0.10%
24	West of Tampa Rd	0.10%

The future project traffic was generated based on 2020 annual average daily traffic volumes (AADTs) and the calculated recommended growth rates. For the intersections and cross streets, the Turning Movement Volume (TMV) is calculated by the future directional design hour volume (DDHV) and 2020 turning movement splits (calculated from existing counts) for each direction. The future year 2045 DDHV for each approach of mainline and cross streets at the intersection is calculated using the 2020 AADT and the annual growth rate, K and D factors. The values were rounded according to the guidance in *FDOT 2019 Project Traffic Forecasting Handbook*.

For the cross streets, the final growth rate was determined based on model growth rate with adjustment by the population growth rate and employment growth rate of nearby Traffic Analysis Zones (TAZs). For most cross streets, the model growth rate was applied directly. For cross streets with negative or unreasonable model growth rate, the population and employment growth rate, as well as Google Earth of nearby TAZs were reviewed to determine an adjusted annual growth rate.

Future volumes for the SR 580 study corridor are provided in **Attachment A** of this memorandum.

## **NO-BUILD OPERATIONAL ANALYSIS (2045)**

LOS was used as the Measure of Effectiveness (MOE) for this study to determine the traffic operational conditions of the roadways and intersections analyzed. In this study, both the segment LOS and intersection LOS were determined using SYNCHRO 11 software, which uses Highway Capacity Manual (HCM) 2000 as the MOE criteria. The LOS output reports from SYNCHRO 11 are provided in **Attachment B**.

Existing signal timing plans were collected from Pinellas County and used in the intersection operational analysis for the future No-Build condition. The signal timing data included the phase plan, cycle length, minimum green, passage time, yellow time, all-red time etc. Out of the twenty-five study intersections, twenty are currently signalized intersections and are listed below:

- SR 580 at Alt US 19/Broadway
- SR 580 at Bass Blvd
- SR 580 at Patricia Ave
- SR 580 at Pinehurst Rd/Crosley Dr
- SR 580 at Lake Haven Road
- SR 580 at Keene Road
- SR 580 at Sunlight Dr/Achieva Way
- SR 580 at Overcash Dr
- SR 580 at King Arthur Ct/Pinewood Dr
- SR 580 at Belcher Road
- SR 580 at US 19 Frontage Roads
- SR 580 at Summerdale Dr
- SR 580 at Countryside Blvd
- SR 580 at Landmark Dr
- SR 580 at Charles Ave
- SR 580 at McMullen Booth Road
- SR 580 at SR 590/2nd St/Philippe Pkwy
- SR 580 at Forest Lakes Blvd./St Petersburg Dr W
- SR 580 at S Bayview Blvd
- SR 580 at Tampa Road – Northbound, Southbound, Eastbound

The existing five (5) unsignalized study intersections were assumed to remain unsignalized in the 2045 No-Build condition and were modeled with stops sign controls on the minor approaches. The five (5) unsignalized study intersections are listed below:

- SR 580 at Douglas Ave
- SR 580 at Enterprise Road
- SR 580 at Countryside HS bus loop entrance
- SR 580 at Rigby Lane
- SR 580 at State Street

**Table 2** lists the LOS results for the 2045 No-Build condition for the roadway segment operations along SR 580 within the study area. There were eight (8) segments with failing LOS in the AM peak period and seven (7) segments with failing LOS in the PM peak period. Failing LOS was taken to be LOS E or worse.

**Table 2 Roadway Segments with Failing LOS in 2045 No-Build**

Time Period	SR 580 Segment		
	Direction	From	To
AM	EB	Lake Haven Rd	Keene Rd
	EB	Pinewood Dr	Belcher Rd
	EB	Belcher Rd	Enterprise Rd
	EB	Enterprise Rd	US 19 Frontage Rd
	EB	Charles Ave	McMullen Booth Rd
	WB	Bayview Blvd	St Petersburg Dr
	WB	Summerdale	US 19 Frontage Rd
	WB	Achieva Way	Keene Rd
PM	EB	Lake Haven Rd	Keene Rd
	EB	Pinewood Dr	Belcher Rd
	EB	Belcher Rd	Enterprise Rd
	EB	Enterprise Rd	US 19 Frontage Rd
	EB	Charles Ave	McMullen Booth Rd
	WB	Summerdale Dr	US 19 Frontage Rd
	WB	Achieva Way	Keene Rd

**Table 3** list the LOS for the intersections along the corridor. **Figure 5** and **Figure 6** show the 2045 No-Build LOS results for study segments and intersections for AM and PM peak period, respectively. **Figure 7** shows the 2045 No-Build AM and PM peak period LOS results.

**Table 3 Intersections LOS in 2045 No-Build**

Intersection	Time Period	
	LOS AM	LOS PM
Alt US 19	C	B
Bass Blvd	C	C
Patricia Ave	C	C
Pinehurst Rd	C	B
Lake Haven Rd	A	A
Keene Rd	<b>E</b>	<b>F</b>
Achieva Way	C	C
Over Cash Dr	B	B
Pinewood Dr	B	C
Belcher Rd	<b>E</b>	<b>E</b>
US 19 Frontage Rd	<b>F</b>	<b>F</b>
Summerdale Dr	B	B
Countryside Blvd	<b>E</b>	<b>F</b>
Landmark Dr	C	C
Charles Ave	A	A
McMullen Booth Rd	<b>F</b>	<b>F</b>
SR 590	D	<b>E</b>
St. Petersburg Dr	<b>F</b>	<b>F</b>
S Bayview Blvd	B	B
Tampa Rd	B	B

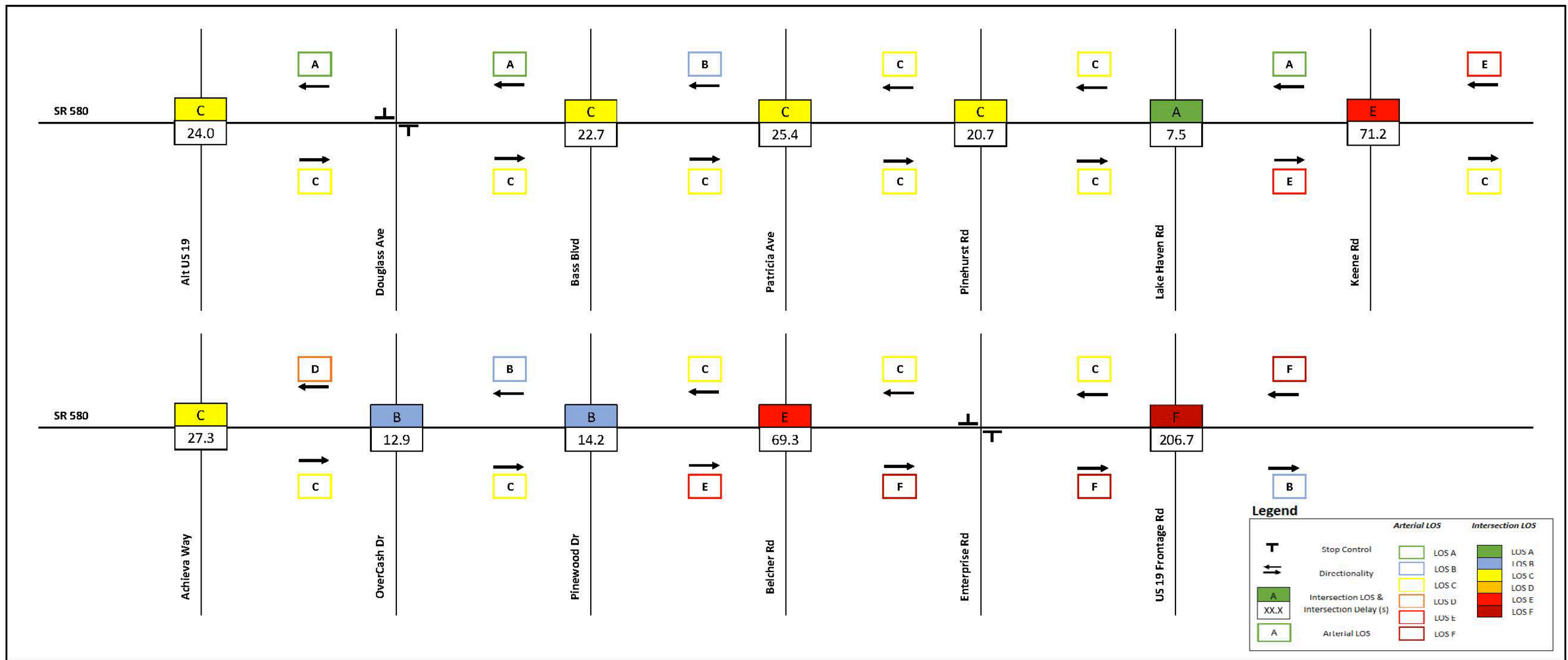


Figure 5 2045 No-Build AM Peak Hour LOS Results

1 of 2

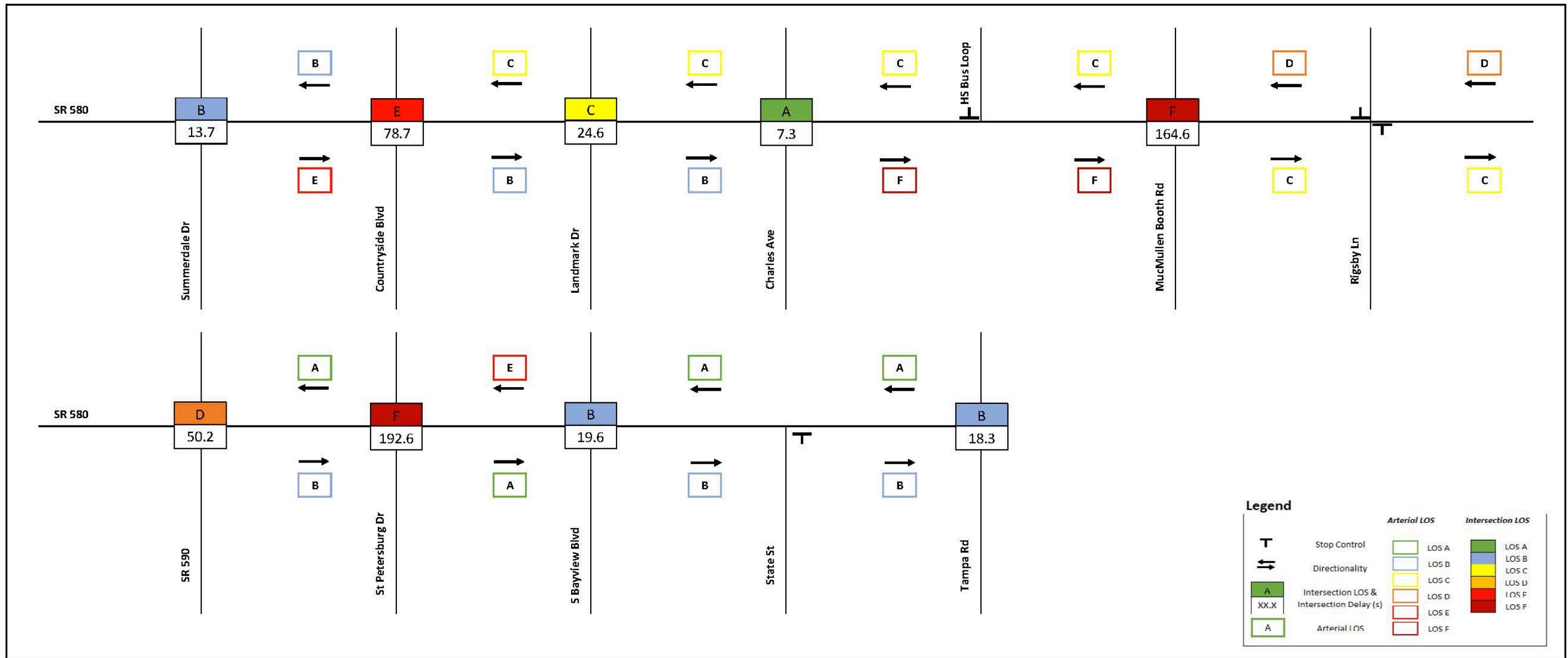


Figure 5 2045 No-Build AM Peak Hour LOS Results



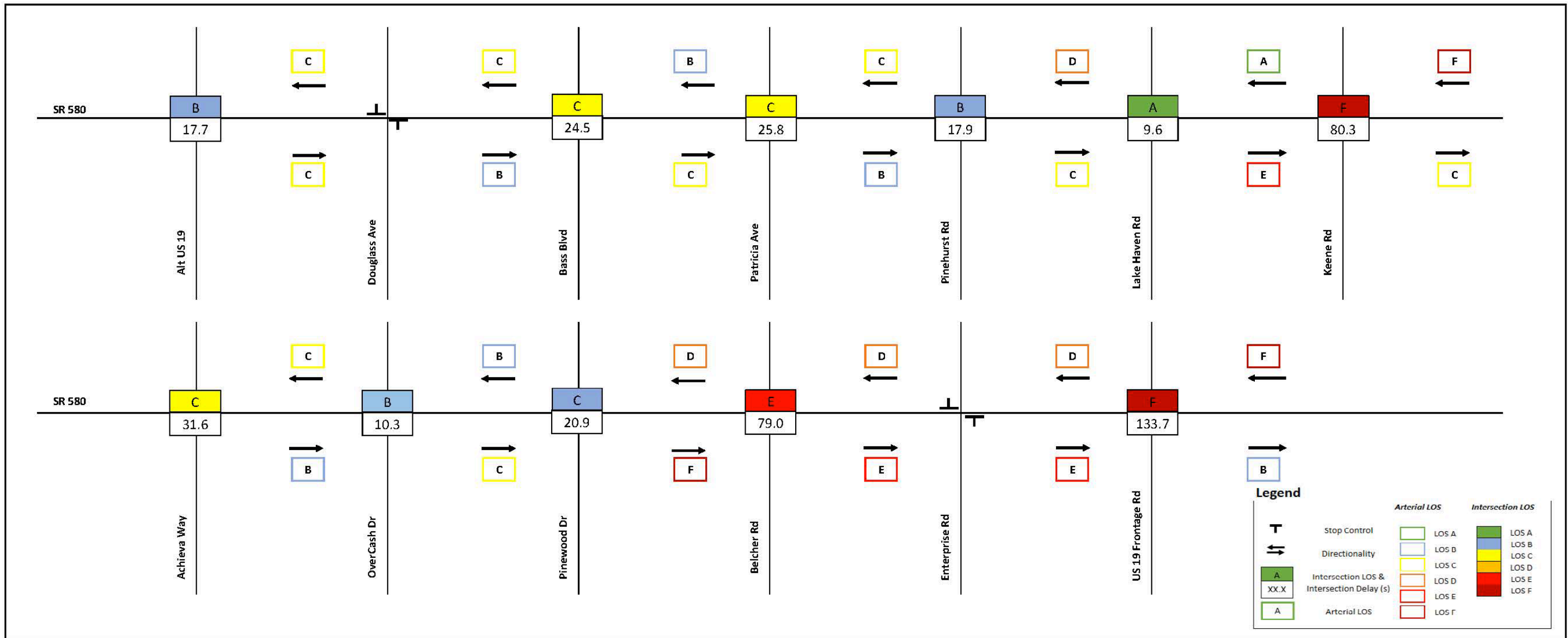


Figure 6 2045 No-Build PM Peak Hour LOS Results

1 of 2

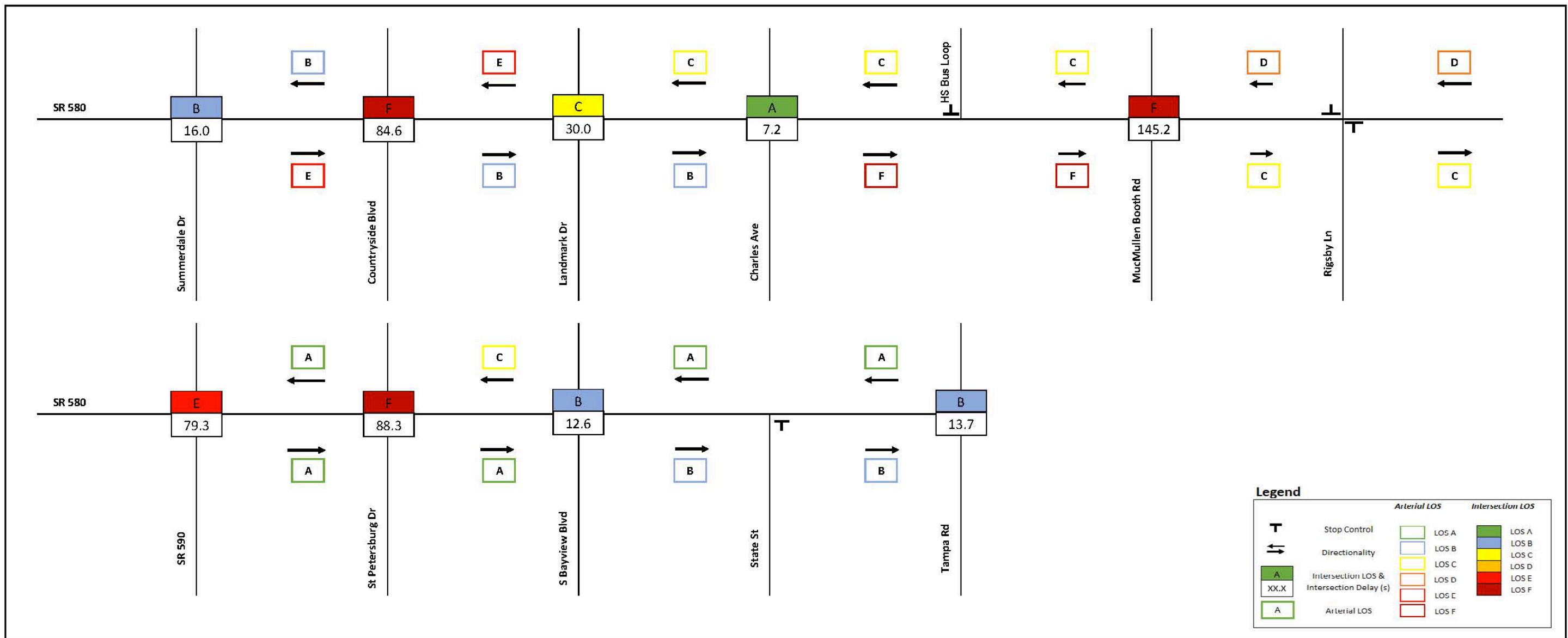


Figure 6 2045 No-Build PM Peak Hour LOS Results

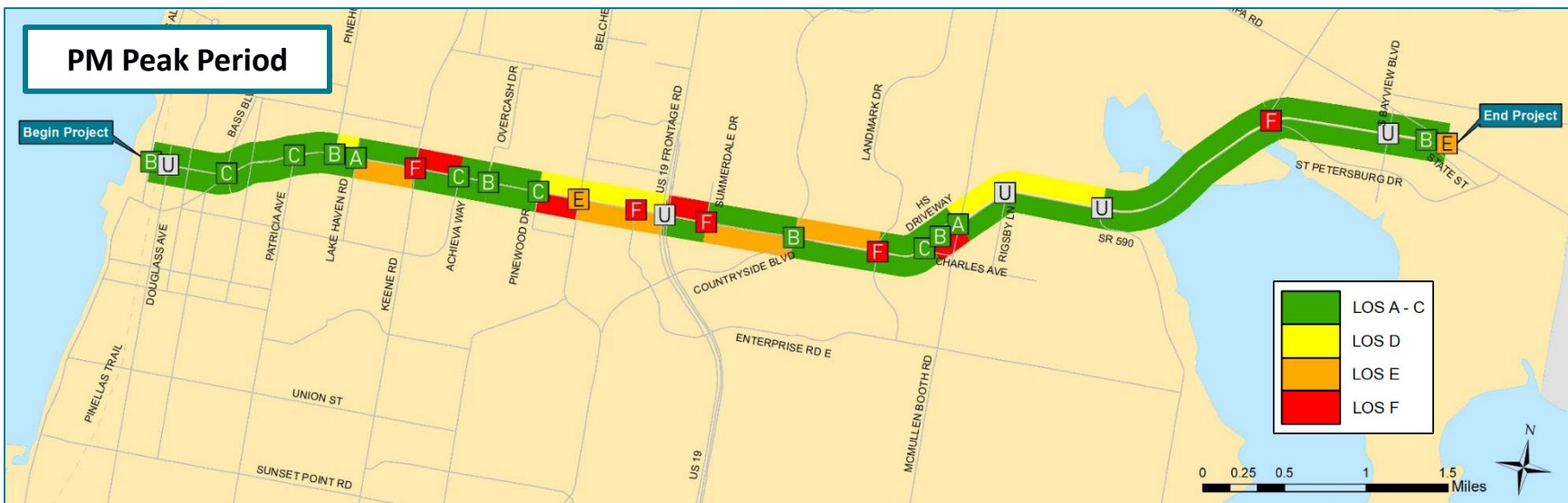
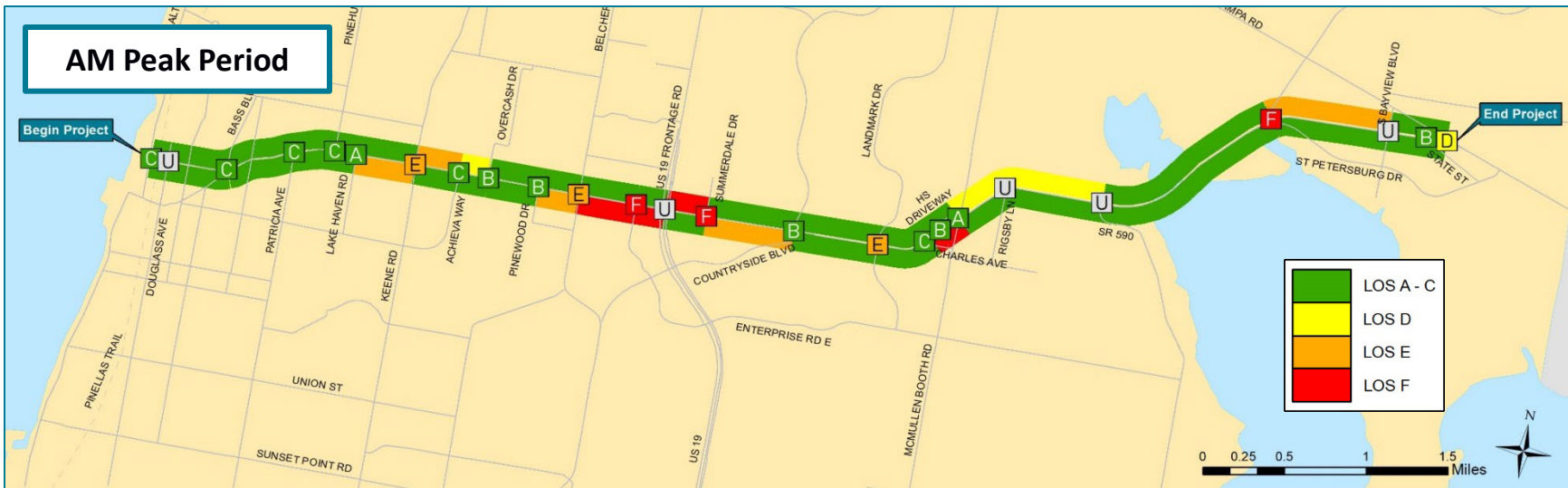


Figure 7 2045 No-Build AM and PM Peak Period LOS Results

Pedestrian and Bicycle LOS standards have been developed on a user-based perception of how safe and comfortable a roadway facility feels. Typically metropolitan planning organizations (MPO) bike plans use LOS C as a target LOS threshold for bike and pedestrian facilities. Pinellas County generally requests separated bike and pedestrian facilities to obtain optimal LOS on roadway facilities. For this study, any LOS identified as LOS D or greater was taken to be below target LOS.

The Bicycle and Pedestrian LOS for each segment was determined using the LOS equations from NCHRP Report 616. **Table 4** summarizes the bicycle and pedestrian LOS for each SR 580 segment in the 2045 No-Build condition. Each segment has two rows to represent eastbound and westbound respectively due to differences in geometry, traffic volume, etc. Pedestrian and Bicycle Level of Service are calculated numerically, then are stratified on an “A” through “F” scale, with “A” being the most accommodating. The numerical stratification for bicycle and pedestrian scores is as follows:

- A ≤ 1.5
- B >1.5 ≤ 2.5
- C >2.5 ≤ 3.5
- D >3.5 ≤ 4.5
- E >4.5 ≤ 5.5
- F >5.5

**Table 4 Bicycle and Pedestrian LOS for 2045 No-Build**

SR 580 Segments							
Road Name	Direction	From	To	Bicycle LOS		Pedestrian LOS	
				Score	Grade	Score	Grade
Skinner Blvd	Eastbound	Broadway (Alt 19)	Douglas Ave	2.36	B	2.49	B
Skinner Blvd	Westbound	Broadway (Alt 19)	Douglas Ave	2.36	B	2.49	B
Skinner Blvd	Eastbound	Howard Ave	Main St	2.05	B	2.61	C
Skinner Blvd	Westbound	Howard Ave	Main St	2.25	B	2.56	C
Main St	Eastbound	Skinner Blvd/Bass Blvd	Patricia Ave	2.59	C	3.03	C
Main St	Westbound	Skinner Blvd/Bass Blvd	Patricia Ave	2.59	C	3.03	C
Main St	Eastbound	Patricia Ave	Crosley Dr/Pinehurst Rd	2.83	C	<b>3.64</b>	<b>D</b>
Main St	Westbound	Patricia Ave	Crosley Dr/Pinehurst Rd	2.83	C	<b>3.64</b>	<b>D</b>
Main St	Eastbound	Crosley Dr/Pinehurst Rd	Keene Rd/CR 1	2.52	C	3.36	C
Main St	Westbound	Crosley Dr/Pinehurst Rd	Keene Rd/CR 1	2.52	C	3.36	C
Main St	Eastbound	Keene Rd/CR 1	Belcher Rd	2.82	C	<b>3.96</b>	<b>D</b>
Main St	Westbound	Keene Rd/CR 2	Belcher Rd	2.82	C	<b>3.96</b>	<b>D</b>

SR 580 Segments							
Road Name	Direction	From	To	Bicycle LOS		Pedestrian LOS	
				Score	Grade	Score	Grade
Main St	Eastbound	Belcher Rd	US 19	2.84	C	<b>3.99</b>	<b>D</b>
Main St	Westbound	Belcher Rd	US 19	2.84	C	<b>3.99</b>	<b>D</b>
SR 580	Eastbound	US 19	Countryside Blvd	2.73	C	<b>3.67</b>	<b>D</b>
SR 580	Westbound	US 19	Countryside Blvd	2.73	C	<b>3.67</b>	<b>D</b>
SR 580	Eastbound	Countryside Blvd	McMullen Booth Rd	<b>3.67</b>	<b>D</b>	<b>3.65</b>	<b>D</b>
SR 580	Westbound	Countryside Blvd	McMullen Booth Rd	<b>3.67</b>	<b>D</b>	<b>3.67</b>	<b>D</b>
SR 580	Eastbound	McMullen Booth Rd	Muellers Lane	<b>3.66</b>	<b>D</b>	<b>3.73</b>	<b>D</b>
SR 580	Westbound	McMullen Booth Rd	Muellers Lane	<b>3.66</b>	<b>D</b>	<b>3.73</b>	<b>D</b>
SR 580	Eastbound	Muellers Lane	Phillippe Pkwy	3.11	C	<b>4.39</b>	<b>D</b>
SR 580	Westbound	Muellers Lane	Phillippe Pkwy	2.90	C	<b>3.72</b>	<b>D</b>
SR 580	Eastbound	Phillippe	W end bridge	3.14	C	<b>4.59</b>	<b>E</b>
SR 580	Westbound	Phillippe	W end bridge	3.14	C	<b>4.59</b>	<b>E</b>
SR 580	Eastbound	W end bridge	E end bridge	0.00	A	<b>3.58</b>	<b>D</b>
SR 580	Westbound	W end bridge	E end bridge	0.00	A	<b>3.58</b>	<b>D</b>
SR 580	Eastbound	E end bridge	St Clair Ave	3.14	C	<b>4.59</b>	<b>E</b>
SR 580	Westbound	E end bridge	St Clair Ave	3.14	C	<b>4.59</b>	<b>E</b>
SR 580	Eastbound	St Clair Ave	St Petersburg Dr/Forest Lakes Blvd	<b>3.97</b>	<b>D</b>	<b>4.65</b>	<b>E</b>
SR 580	Westbound	St Clair Ave	St Petersburg Dr/Forest Lakes Blvd	<b>3.97</b>	<b>D</b>	<b>4.65</b>	<b>E</b>
State St	Eastbound	St Petersburg Dr/Forest Lakes Blvd	Hillsborough Ave	<b>3.70</b>	<b>D</b>	<b>3.64</b>	<b>D</b>
State St	Westbound	St Petersburg Dr/Forest Lakes Blvd	Hillsborough Ave	<b>3.70</b>	<b>D</b>	<b>3.64</b>	<b>D</b>

## CONCLUSION

SR 580 from Alternative US 19 to Tampa Road is a major east-west urban arterial in Pinellas County Florida. Future residential land use adjacent to the study SR 580 corridor is forecasted to drop from 58% to 52%, commercial land use is forecasted to drop from 21% to 14% and mixed land use is expected to increase from 0% to 13%. Future mixed use land use areas along SR 580 (specifically at the west end of the corridor in the City of Dunedin, surrounding areas of the US 19 interchange and Countryside Mall and east end of the corridor in the City of Oldsmar) are expected to be future generators and attractors of multimodal traffic.

The vision for the SR 580 corridor includes providing safe options for pedestrians, bicyclist, transit users and motorized vehicles using the corridor while not deteriorating vehicle operations or increasing delay. This vision will include providing sidewalks and bicycle facilities along the corridor and connections to other multimodal facilities and proposing transportation improvements that reduce vehicle crashes and enhance safety for all users. The overall goal of the study is to develop context sensitive improvements to help transform SR 580 into a multimodal urban corridor in keeping with the community context it traverses.

The Forward Pinellas 2045 LRTP identifies one (1) project within the study corridor; Forest Lakes Boulevard from SR 580 to SR 584 is identified as a widening project to become a four-lane divided roadway. Other projects currently active along the SR 580 study corridor include the pedestrian signal crossing for the Duke Energy Trail and a complete streets project in the City of Dunedin from Alternative US 19 to Main Street/Bass Boulevard.

Future year 2045 traffic volumes were developed from growth rates established from the TBRPM V9.0. Using SYNCHRO 11 software, no-build 2045 LOS evaluation found eight (8) segments and six (6) intersections have failing LOS in the AM peak period and seven (7) segments and intersections have failing LOS in the PM peak period. Bicycle and Pedestrian LOS evaluation for the 2045 no-build condition found four (4) SR 580 segments below target LOS for bicycles and twelve (12) SR 580 segments below target LOS for pedestrians.

Due to future land use changes and the vision and goals for the SR 580 study corridor, improvements that enhance safety and operation for all users of the roadway facility are recommended.

## ATTACHMENTS

---

Attachment A 2045 Volumes

Attachment B LOS Results

# Attachment A



SR 580	11,800	14,600	19,800	31,000	34,100	34,200	46,900
Alt US 19	Douglass Ave	Bass Blvd	Patricia Ave	Pinehurst Rd	Lake Haven Rd	Keene Rd	
SR 580	52,700	53,100	53,100	54,200	53,300	42,900	
Achieva Way	Overcash Dr	Pinewood Dr	Belcher Rd	Enterprise Rd	US 19 Frontage Rd		

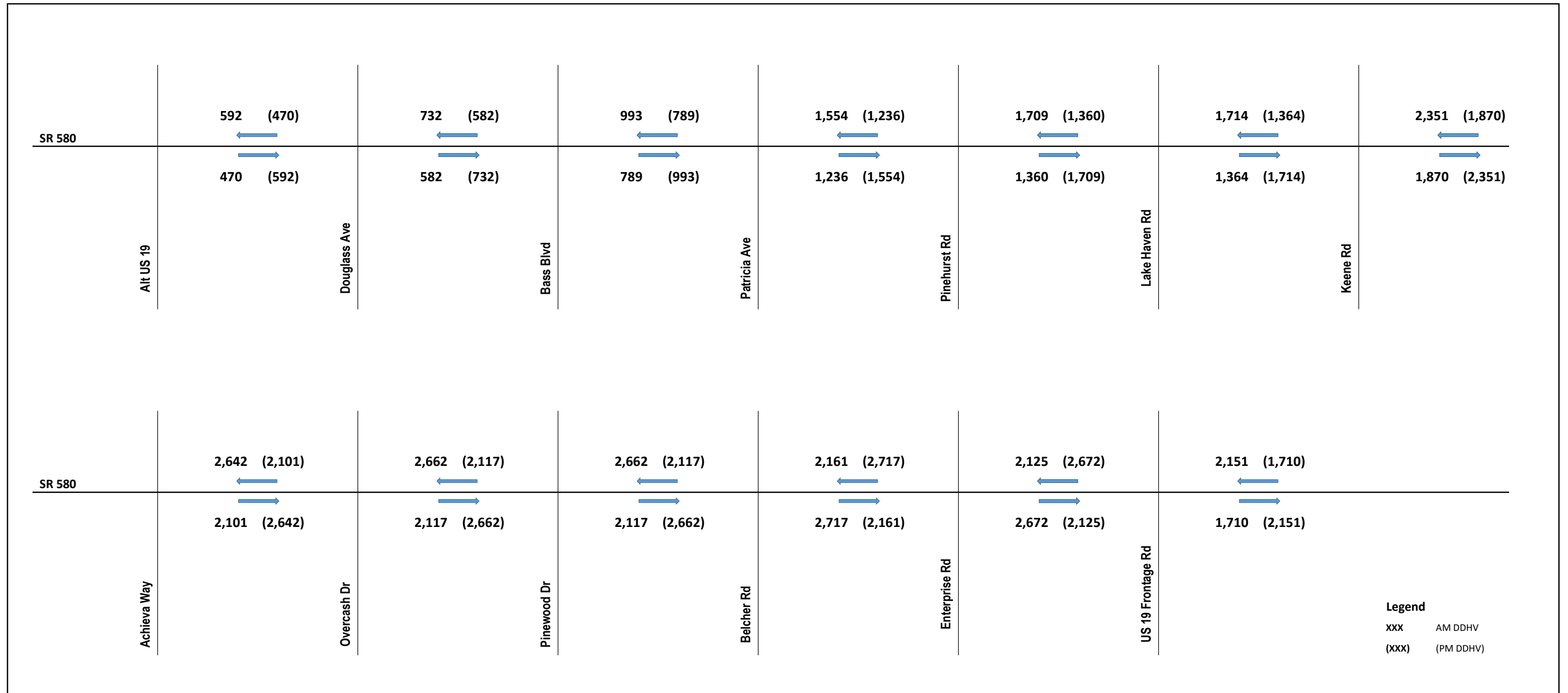
**SR 580 Corridor Planning and Concept Development Study**  
Pinellas County, Florida

*Future 2045 AADT*  
**Figure 1**

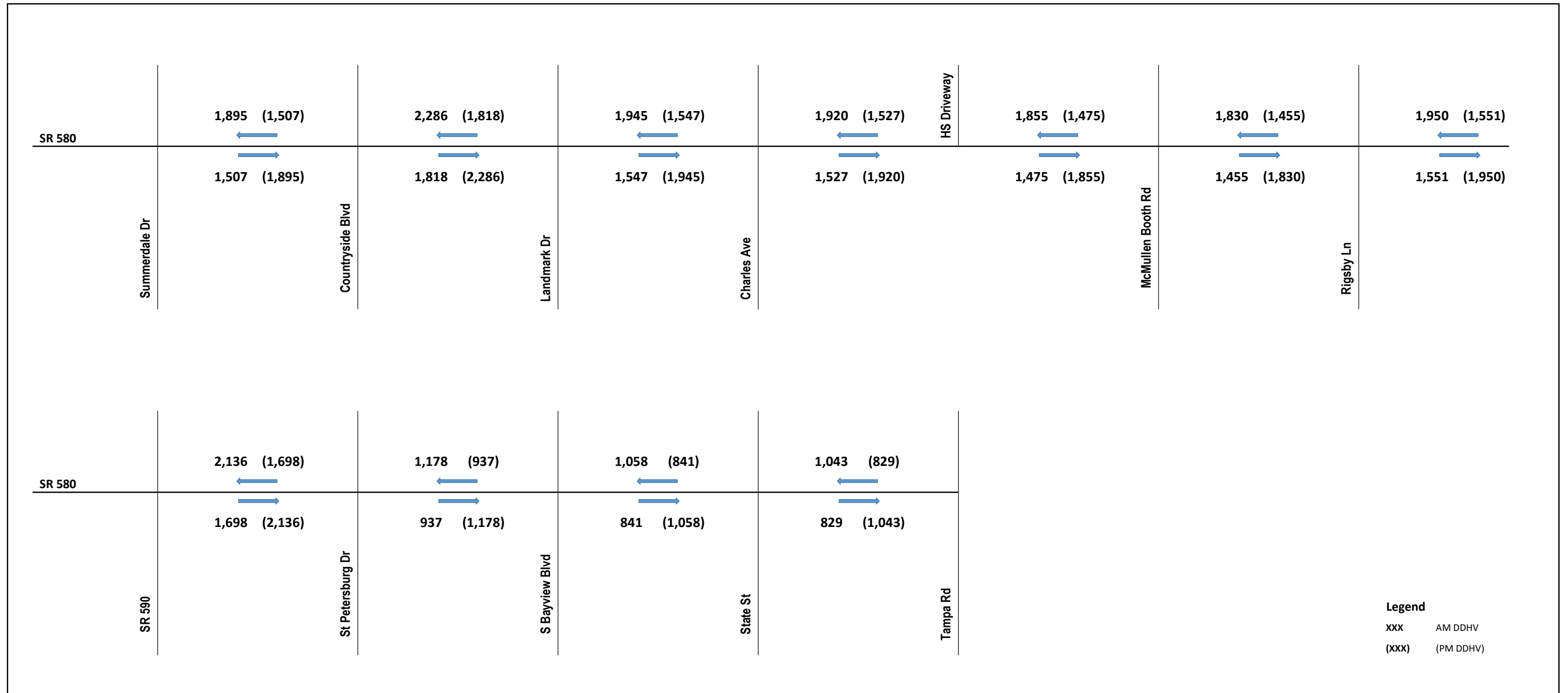
SR 580	37,800	45,600	38,800	38,300	HS Driveway	37,000	36,500	38,900
Summerdale Dr	Countryside Blvd	Landmark Dr	Charles Ave	McMullen Booth Rd	Rigsby Ln			
SR 580	42,600	23,500	21,100	20,800				
SR 590	St Petersburg Dr	S Bayview Blvd	State St	Tampa Rd				

**SR 580 Corridor Planning and Concept Development Study**  
 Pinellas County, Florida

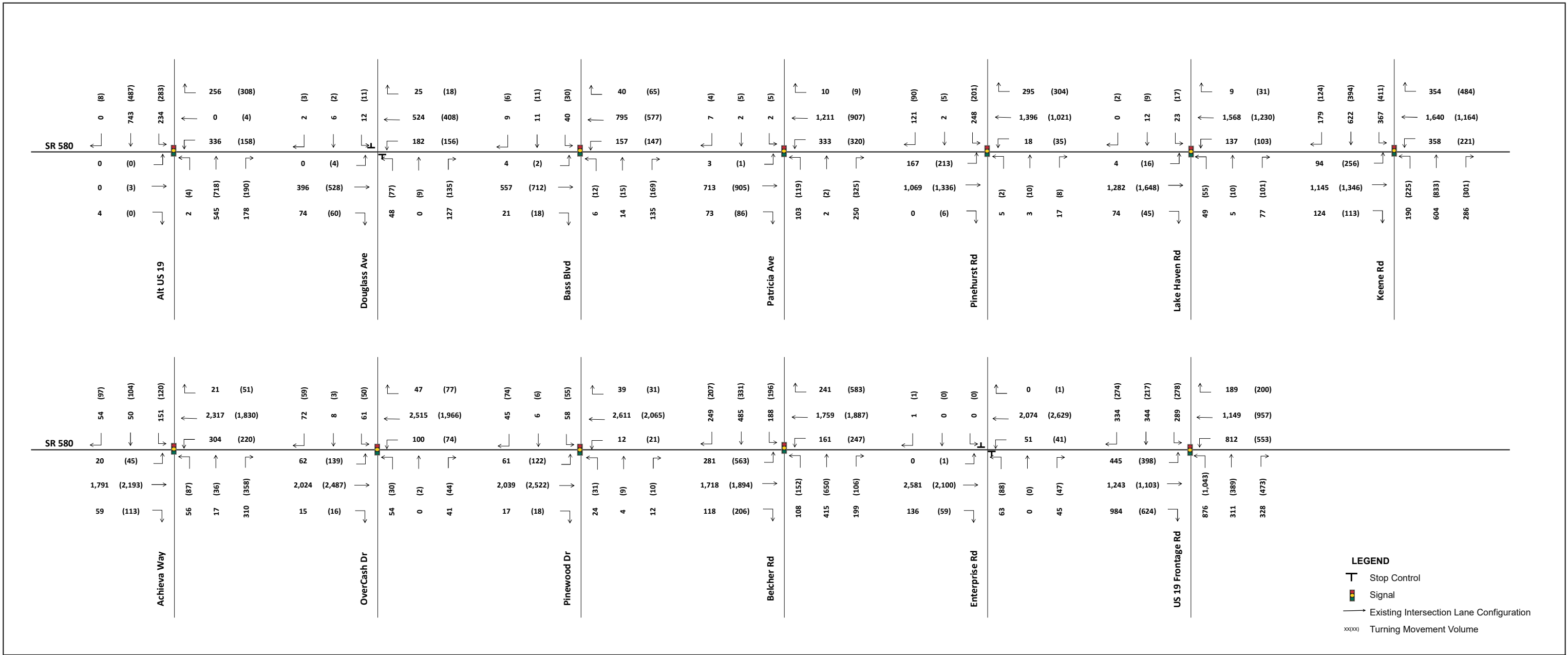
*Future 2045 AADT*  
**Figure 2**



**Legend**  
 XXX AM DDHV  
 (XXX) (PM DDHV)



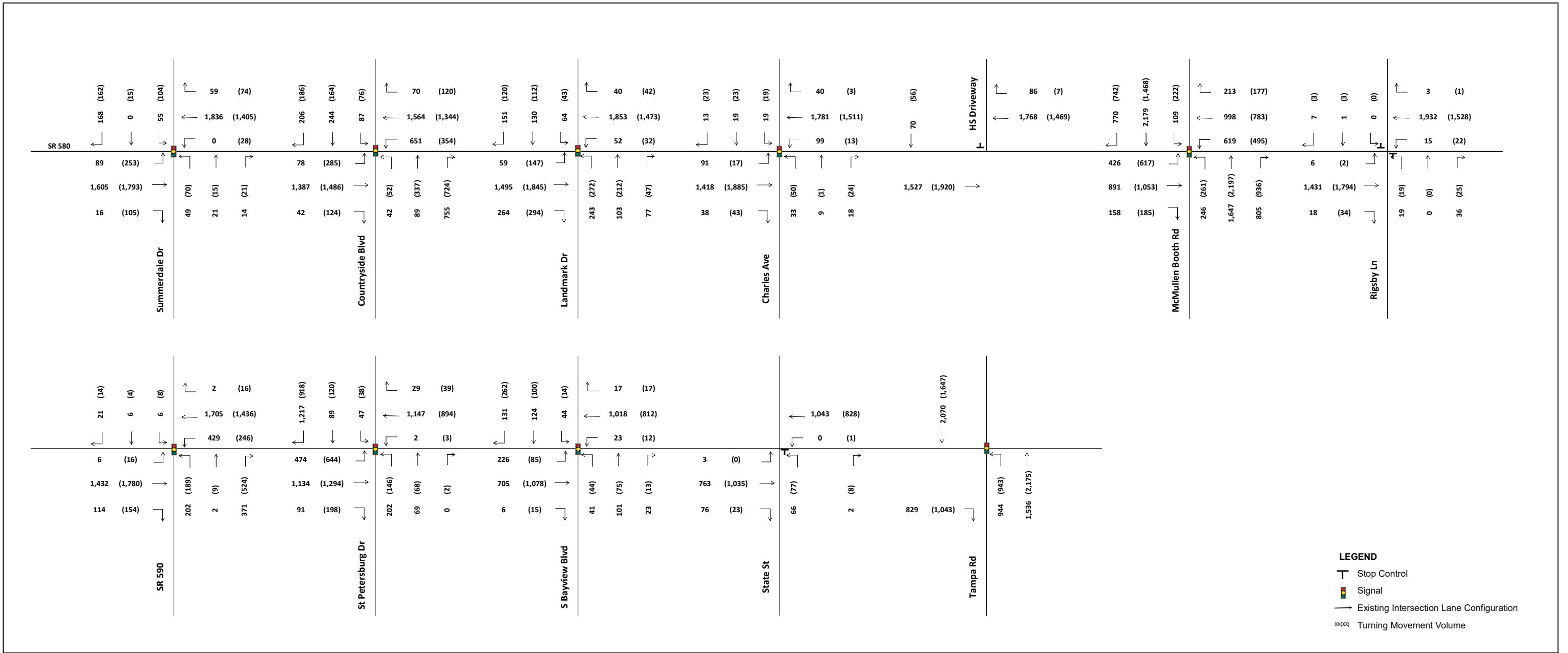
**Legend**  
 XXX AM DDHV  
 (XXX) (PM DDHV)



**SR 580 Corridor Planning and Concept Development Study**  
Pinellas County, Florida

**Future 2045 AM and PM Peak Hour Traffic Volumes**

**Figure 1**



**SR 580 Corridor Planning and Concept Development Study**  
 Pinellas County, Florida

*Future 2045 AM and PM Peak Hour Traffic Volumes*  
**Figure 2**

Roadway		2045 Peak Hour TMV for U-turn & Left-turn				
		AM		PM		
		U	L	U	L	
A	Alt US 19/Broadway	EB	-	-	-	-
		WB	2	334	-	158
		NB	2	-	1	3
		SB	-	234	-	283
B	Douglas Ave	EB	-	-	-	4
		WB	2	181	-	156
		NB	-	48	-	77
		SB	-	12	-	11
C	Bass Blvd	EB	-	4	-	2
		WB	-	157	1	145
		NB	-	6	-	12
		SB	-	40	-	30
D	Patricia Ave	EB	2	2	-	1
		WB	8	325	10	310
		NB	5	98	2	116
		SB	-	2	-	5
E	Pinehurst Rd/Crosley Dr	EB	4	162	7	205
		WB	-	18	3	32
		NB	-	5	-	2
		SB	-	248	-	201
F	Lake Haven Rd	EB	-	4	8	8
		WB	5	132	7	96
		NB	-	49	-	55
		SB	-	23	-	17
G	Keene Road	EB	4	91	7	249
		WB	-	358	1	220
		NB	2	188	-	225
		SB	-	367	-	411
H	Sunlight Dr/Achieva Way	EB	-	20	3	42
		WB	-	304	2	217
		NB	5	51	6	82
		SB	-	151	-	120
I	Overcash Dr	EB	33	30	13	125
		WB	4	96	8	65
		NB	-	54	1	29
		SB	-	61	-	50
J	King Arthur Ct/Pinewood Dr	EB	-	61	1	120
		WB	2	10	2	19
		NB	-	24	-	31
		SB	-	58	-	55
K	Belcher Road	EB	10	272	10	553
		WB	3	157	11	236
		NB	-	108	-	152
		SB	-	188	5	191
L	Enterprise Road	EB	-	-	-	1
		WB	3	48	7	34
		NB	-	63	-	88
		SB	-	-	-	-

Roadway			2045 Peak Hour TMV for U-turn & Left-turn			
			AM		PM	
			U	L	U	L
M	US 19 Frontage Roads	EB	6	439	5	393
		WB	-	812	-	553
		NB	84	791	96	947
		SB	103	186	44	234
N	Summerdale Dr	EB	-	89	3	250
		WB	-	-	-	28
		NB	-	49	-	70
		SB	-	55	-	104
O	Countryside Blvd	EB	-	78	3	282
		WB	-	651	1	352
		NB	-	42	-	52
		SB	-	87	-	76
P	Landmark Dr	EB	-	59	9	138
		WB	-	52	-	32
		NB	-	243	-	272
		SB	-	64	-	43
Q	Charles Ave	EB	1	89	7	11
		WB	65	34	8	5
		NB	-	33	-	50
		SB	-	19	-	19
R	School DW	EB	-	-	-	-
		WB	-	-	-	-
		SB	-	-	-	-
S	McMullen Booth Road	EB	7	420	3	615
		WB	3	616	1	494
		NB	17	229	16	245
		SB	-	109	2	220
T	Rigby Lane	EB	1	4	1	1
		WB	2	14	2	20
		NB	-	19	-	19
		SB	-	-	-	-
U	SR 590/2nd St	EB	-	6	-	16
		WB	-	429	-	246
		NB	2	200	4	186
		SB	-	6	-	8
V	Forest Lakes Blvd./St Petersburg Dr W	EB	-	474	1	643
		WB	-	2	-	3
		NB	-	202	-	146
		SB	-	47	-	38
W	S Bayview Blvd	EB	1	225	1	84
		WB	-	23	1	11
		NB	-	41	-	44
		SB	-	44	-	14
X	State Street	EB	3	-	-	-
		WB	-	-	1	-
		NB	-	66	-	77
Y	Tampa Rd	EB	-	-	-	-
		NB	-	944	1	942
		SB	-	-	-	-

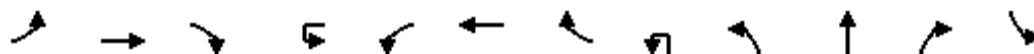


# Attachment B

# HCM Signalized Intersection Capacity Analysis

## 3: Alt 19 & SR 580

06/24/2021



Movement	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBL
Lane Configurations		↕				↕	↕			↕	↕	↕
Traffic Volume (vph)	0	0	4	2	336	0	256	2	2	545	178	234
Future Volume (vph)	0	0	4	2	336	0	256	2	2	545	178	234
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		7.3				6.7	6.7			6.7	6.7	6.5
Lane Util. Factor		1.00				1.00	1.00			0.95	1.00	1.00
Frt		0.86				1.00	0.85			1.00	0.85	1.00
Flt Protected		1.00				0.95	1.00			1.00	1.00	0.95
Satd. Flow (prot)		1611				1770	1583			3538	1583	1770
Flt Permitted		1.00				0.76	1.00			0.95	1.00	0.28
Satd. Flow (perm)		1611				1407	1583			3357	1583	520
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Adj. Flow (vph)	0	0	4	2	361	0	275	2	2	586	191	252
RTOR Reduction (vph)	0	3	0	0	0	0	196	0	0	0	132	0
Lane Group Flow (vph)	0	1	0	0	0	363	79	0	0	590	59	252
Turn Type		NA		Perm	Perm	NA	Perm	Perm	Perm	NA	Perm	pm+pt
Protected Phases		8				4				6		5
Permitted Phases	8			4	4		4	6	6		6	2
Actuated Green, G (s)		19.2				19.8	19.8			21.4	21.4	35.9
Effective Green, g (s)		19.2				19.8	19.8			21.4	21.4	35.9
Actuated g/C Ratio		0.28				0.29	0.29			0.31	0.31	0.52
Clearance Time (s)		7.3				6.7	6.7			6.7	6.7	6.5
Vehicle Extension (s)		3.0				3.0	3.0			3.0	3.0	3.0
Lane Grp Cap (vph)		447				403	453			1039	490	414
v/s Ratio Prot		0.00										0.07
v/s Ratio Perm						0.26	0.05			0.18	0.04	0.25
v/c Ratio		0.00				0.90	0.17			0.57	0.12	0.61
Uniform Delay, d1		18.0				23.7	18.5			20.0	17.1	10.3
Progression Factor		1.00				1.00	1.00			1.00	1.00	1.00
Incremental Delay, d2		0.0				22.6	0.2			2.3	0.5	2.5
Delay (s)		18.0				46.3	18.7			22.2	17.6	12.8
Level of Service		B				D	B			C	B	B
Approach Delay (s)		18.0				34.4				21.1		
Approach LOS		B				C				C		

### Intersection Summary

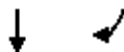
HCM 2000 Control Delay	24.0	HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio	0.98		
Actuated Cycle Length (s)	69.1	Sum of lost time (s)	20.5
Intersection Capacity Utilization	97.8%	ICU Level of Service	F
Analysis Period (min)	15		

c Critical Lane Group

# HCM Signalized Intersection Capacity Analysis

## 3: Alt 19 & SR 580

06/24/2021



Movement	SBT	SBR
Lane Configurations	⤴	
Traffic Volume (vph)	743	0
Future Volume (vph)	743	0
Ideal Flow (vphpl)	1900	1900
Total Lost time (s)	6.7	
Lane Util. Factor	1.00	
Frt	1.00	
Flt Protected	1.00	
Satd. Flow (prot)	1863	
Flt Permitted	1.00	
Satd. Flow (perm)	1863	
Peak-hour factor, PHF	0.93	0.93
Adj. Flow (vph)	799	0
RTOR Reduction (vph)	0	0
Lane Group Flow (vph)	799	0
Turn Type	NA	
Protected Phases	2	
Permitted Phases		
Actuated Green, G (s)	35.9	
Effective Green, g (s)	35.9	
Actuated g/C Ratio	0.52	
Clearance Time (s)	6.7	
Vehicle Extension (s)	3.0	
Lane Grp Cap (vph)	967	
v/s Ratio Prot	c0.43	
v/s Ratio Perm		
v/c Ratio	0.83	
Uniform Delay, d1	14.0	
Progression Factor	1.00	
Incremental Delay, d2	8.0	
Delay (s)	22.0	
Level of Service	C	
Approach Delay (s)	19.8	
Approach LOS	B	
Intersection Summary		

# HCM Signalized Intersection Capacity Analysis

## 4: Main St/Bass Blvd & SR 580

06/24/2021



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↗		↖	↗			↖	↗	↖	↗	
Traffic Volume (vph)	4	557	21	157	795	40	6	14	135	40	11	9
Future Volume (vph)	4	557	21	157	795	40	6	14	135	40	11	9
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	7.1	7.1		7.1	7.1			9.1	9.1	7.0	7.0	
Lane Util. Factor	1.00	0.95		1.00	0.95			1.00	1.00	1.00	1.00	
Frt	1.00	0.99		1.00	0.99			1.00	0.85	1.00	0.93	
Flt Protected	0.95	1.00		0.95	1.00			0.99	1.00	0.95	1.00	
Satd. Flow (prot)	1593	3168		1593	3162			1653	1425	1593	1562	
Flt Permitted	0.95	1.00		0.95	1.00			0.99	1.00	0.95	1.00	
Satd. Flow (perm)	1593	3168		1593	3162			1653	1425	1593	1562	
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Adj. Flow (vph)	4	599	23	169	855	43	6	15	145	43	12	10
RTOR Reduction (vph)	0	2	0	0	2	0	0	0	138	0	9	0
Lane Group Flow (vph)	4	620	0	169	896	0	0	21	7	43	13	0
Turn Type	Prot	NA		Prot	NA		Split	NA	Perm	Split	NA	
Protected Phases	1	6		5	2		4	4		8	8	
Permitted Phases									4			
Actuated Green, G (s)	1.0	51.3		21.9	72.2			5.6	5.6	5.9	5.9	
Effective Green, g (s)	1.0	51.3		21.9	72.2			5.6	5.6	5.9	5.9	
Actuated g/C Ratio	0.01	0.45		0.19	0.63			0.05	0.05	0.05	0.05	
Clearance Time (s)	7.1	7.1		7.1	7.1			9.1	9.1	7.0	7.0	
Vehicle Extension (s)	1.0	1.0		1.0	1.0			1.0	1.0	1.0	1.0	
Lane Grp Cap (vph)	13	1413		303	1985			80	69	81	80	
v/s Ratio Prot	0.00	0.20		c0.11	c0.28			c0.01		c0.03	0.01	
v/s Ratio Perm									0.00			
v/c Ratio	0.31	0.44		0.56	0.45			0.26	0.10	0.53	0.16	
Uniform Delay, d1	56.7	21.9		42.2	11.1			52.7	52.3	53.2	52.2	
Progression Factor	1.00	1.00		1.22	0.74			1.00	1.00	1.00	1.00	
Incremental Delay, d2	4.8	1.0		1.1	0.6			0.6	0.2	3.3	0.3	
Delay (s)	61.5	22.9		52.4	8.8			53.3	52.5	56.5	52.5	
Level of Service	E	C		D	A			D	D	E	D	
Approach Delay (s)		23.2			15.7			52.6			55.2	
Approach LOS		C			B			D			E	

### Intersection Summary

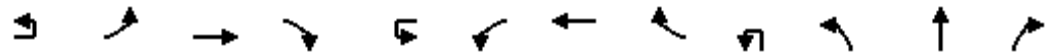
HCM 2000 Control Delay	22.7	HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio	0.50		
Actuated Cycle Length (s)	115.0	Sum of lost time (s)	30.3
Intersection Capacity Utilization	56.8%	ICU Level of Service	B
Analysis Period (min)	15		

c Critical Lane Group

# HCM Signalized Intersection Capacity Analysis

## 13: Patricia Ave & SR 580

06/24/2021



Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR
Lane Configurations		↖	↗	↘		↖	↗				↖	↗
Traffic Volume (vph)	2	3	713	73	8	325	1211	10	5	98	2	250
Future Volume (vph)	2	3	713	73	8	325	1211	10	5	98	2	250
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		6.9	6.9	6.9		6.9	6.9				6.7	6.7
Lane Util. Factor		1.00	0.95	1.00		0.97	0.95				1.00	0.88
Frt		1.00	1.00	0.85		1.00	1.00				1.00	0.85
Flt Protected		0.95	1.00	1.00		0.95	1.00				0.95	1.00
Satd. Flow (prot)		1770	3539	1583		3433	3535				1776	2787
Flt Permitted		0.95	1.00	1.00		0.95	1.00				0.72	1.00
Satd. Flow (perm)		1770	3539	1583		3433	3535				1344	2787
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Adj. Flow (vph)	2	3	767	78	9	349	1302	11	5	105	2	269
RTOR Reduction (vph)	0	0	0	58	0	0	0	0	0	0	0	239
Lane Group Flow (vph)	0	5	767	20	0	358	1313	0	0	0	112	30
Turn Type	Prot	Prot	NA	Perm	Prot	Prot	NA		Perm	Perm	NA	Perm
Protected Phases	1	1	6		5	5	2				4	
Permitted Phases				6					4	4		4
Actuated Green, G (s)		1.0	30.1	30.1		42.4	71.5				13.0	13.0
Effective Green, g (s)		1.0	30.1	30.1		42.4	71.5				13.0	13.0
Actuated g/C Ratio		0.01	0.26	0.26		0.37	0.62				0.11	0.11
Clearance Time (s)		6.9	6.9	6.9		6.9	6.9				6.7	6.7
Vehicle Extension (s)		1.0	1.0	1.0		1.0	1.0				1.0	1.0
Lane Grp Cap (vph)		15	926	414		1265	2197				151	315
v/s Ratio Prot		0.00	c0.22			0.10	c0.37					
v/s Ratio Perm				0.01							c0.08	0.01
v/c Ratio		0.33	0.83	0.05		0.28	0.60				0.74	0.10
Uniform Delay, d1		56.7	40.0	31.7		25.6	13.1				49.4	45.7
Progression Factor		0.71	0.67	1.00		0.81	1.02				1.00	1.00
Incremental Delay, d2		4.5	5.6	0.0		0.0	1.0				15.7	0.0
Delay (s)		45.0	32.6	31.8		20.9	14.3				65.1	45.8
Level of Service		D	C	C		C	B				E	D
Approach Delay (s)			32.6				15.7				51.4	
Approach LOS			C				B				D	

### Intersection Summary

HCM 2000 Control Delay	25.4	HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio	0.70		
Actuated Cycle Length (s)	115.0	Sum of lost time (s)	27.5
Intersection Capacity Utilization	67.5%	ICU Level of Service	C
Analysis Period (min)	15		

c Critical Lane Group

# HCM Signalized Intersection Capacity Analysis

## 13: Patricia Ave & SR 580

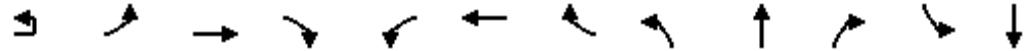
06/24/2021



Movement	SBL	SBT	SBR
Lane Configurations			
Traffic Volume (vph)	2	2	7
Future Volume (vph)	2	2	7
Ideal Flow (vphpl)	1900	1900	1900
Total Lost time (s)		7.0	
Lane Util. Factor		1.00	
Frt		0.91	
Flt Protected		0.99	
Satd. Flow (prot)		1681	
Flt Permitted		0.96	
Satd. Flow (perm)		1629	
Peak-hour factor, PHF	0.93	0.93	0.93
Adj. Flow (vph)	2	2	8
RTOR Reduction (vph)	0	6	0
Lane Group Flow (vph)	0	6	0
Turn Type	Perm	NA	
Protected Phases		3 8	
Permitted Phases	3 8		
Actuated Green, G (s)		21.7	
Effective Green, g (s)		21.7	
Actuated g/C Ratio		0.19	
Clearance Time (s)			
Vehicle Extension (s)			
Lane Grp Cap (vph)		307	
v/s Ratio Prot			
v/s Ratio Perm		c0.00	
v/c Ratio		0.02	
Uniform Delay, d1		38.0	
Progression Factor		1.00	
Incremental Delay, d2		0.0	
Delay (s)		38.0	
Level of Service		D	
Approach Delay (s)		38.0	
Approach LOS		D	
<b>Intersection Summary</b>			

HCM Signalized Intersection Capacity Analysis  
 18: Crosley Dr/Pinehurst Rd & SR 580

06/24/2021



Movement	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations		↖	↗		↖	↗	↗		↖	↗	↖	↖
Traffic Volume (vph)	4	162	1069	0	18	1396	295	5	3	17	248	2
Future Volume (vph)	4	162	1069	0	18	1396	295	5	3	17	248	2
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		7.2	7.2		6.8	7.2	7.2		7.4	7.4	7.5	7.5
Lane Util. Factor		1.00	0.95		1.00	0.95	1.00		1.00	1.00	0.95	0.95
Frt		1.00	1.00		1.00	1.00	0.85		1.00	0.85	1.00	1.00
Flt Protected		0.95	1.00		0.95	1.00	1.00		0.97	1.00	0.95	0.95
Satd. Flow (prot)		1770	3539		1770	3539	1583		1806	1583	1681	1687
Flt Permitted		0.13	1.00		0.20	1.00	1.00		0.97	1.00	0.95	0.95
Satd. Flow (perm)		246	3539		375	3539	1583		1806	1583	1681	1687
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Adj. Flow (vph)	4	174	1149	0	19	1501	317	5	3	18	267	2
RTOR Reduction (vph)	0	0	0	0	0	0	49	0	0	18	0	0
Lane Group Flow (vph)	0	178	1149	0	19	1501	268	0	8	0	133	136
Turn Type	pm+pt	pm+pt	NA		pm+pt	NA	Perm	Split	NA	Perm	Split	NA
Protected Phases	1	1	6		5	2		4	4		8	8
Permitted Phases	6	6			2		2			4		
Actuated Green, G (s)		171.7	171.7		160.5	160.5	160.5		4.3	4.3	22.0	22.0
Effective Green, g (s)		171.7	171.7		160.5	160.5	160.5		4.3	4.3	22.0	22.0
Actuated g/C Ratio		0.75	0.75		0.70	0.70	0.70		0.02	0.02	0.10	0.10
Clearance Time (s)		7.2	7.2		6.8	7.2	7.2		7.4	7.4	7.5	7.5
Vehicle Extension (s)		1.0	1.0		1.0	1.0	1.0		1.0	1.0	1.0	1.0
Lane Grp Cap (vph)		275	2641		280	2469	1104		33	29	160	161
v/s Ratio Prot		c0.04	0.32		0.00	c0.42			c0.00		0.08	c0.08
v/s Ratio Perm		c0.44			0.05		0.17			0.00		
v/c Ratio		0.65	0.44		0.07	0.61	0.24		0.24	0.01	0.83	0.84
Uniform Delay, d1		44.3	10.9		12.5	18.2	12.6		111.2	110.8	102.2	102.3
Progression Factor		0.79	0.57		0.35	0.26	0.15		1.00	1.00	1.00	1.00
Incremental Delay, d2		3.6	0.0		0.0	1.1	0.5		1.4	0.1	28.1	30.2
Delay (s)		38.7	6.3		4.5	5.8	2.4		112.6	110.8	130.3	132.5
Level of Service		D	A		A	A	A		F	F	F	F
Approach Delay (s)			10.7			5.2			111.4			119.5
Approach LOS			B			A			F			F

Intersection Summary

HCM 2000 Control Delay	20.7	HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio	0.67		
Actuated Cycle Length (s)	230.0	Sum of lost time (s)	29.3
Intersection Capacity Utilization	83.9%	ICU Level of Service	E
Analysis Period (min)	15		

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis  
 18: Crosley Dr/Pinehurst Rd & SR 580

06/24/2021

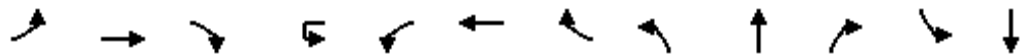
Movement	SBR
Lane Configurations	
Traffic Volume (vph)	121
Future Volume (vph)	121
Ideal Flow (vphpl)	1900
Total Lost time (s)	7.5
Lane Util. Factor	1.00
Frt	0.85
Flt Protected	1.00
Satd. Flow (prot)	1583
Flt Permitted	1.00
Satd. Flow (perm)	1583
Peak-hour factor, PHF	0.93
Adj. Flow (vph)	130
RTOR Reduction (vph)	118
Lane Group Flow (vph)	12
Turn Type	Perm
Protected Phases	
Permitted Phases	8
Actuated Green, G (s)	22.0
Effective Green, g (s)	22.0
Actuated g/C Ratio	0.10
Clearance Time (s)	7.5
Vehicle Extension (s)	1.0
Lane Grp Cap (vph)	151
v/s Ratio Prot	
v/s Ratio Perm	0.01
v/c Ratio	0.08
Uniform Delay, d1	94.8
Progression Factor	1.00
Incremental Delay, d2	0.1
Delay (s)	94.9
Level of Service	F
Approach Delay (s)	
Approach LOS	
Intersection Summary	



# HCM Signalized Intersection Capacity Analysis

## 20: Lake Haven Rd & SR 580

06/24/2021



Movement	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations	↖	↑↑↑	↗		↖	↑↑↑			↑	↗	↖	↗
Traffic Volume (vph)	4	1282	74	5	132	1568	9	49	5	77	23	12
Future Volume (vph)	4	1282	74	5	132	1568	9	49	5	77	23	12
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	6.8	6.8	6.8		6.8	6.8			8.0	8.0	8.0	8.0
Lane Util. Factor	1.00	0.91	1.00		1.00	0.91			1.00	1.00	1.00	1.00
Frt	1.00	1.00	0.85		1.00	1.00			1.00	0.85	1.00	1.00
Flt Protected	0.95	1.00	1.00		0.95	1.00			0.96	1.00	0.95	1.00
Satd. Flow (prot)	1770	5085	1583		1770	5081			1781	1583	1770	1863
Flt Permitted	0.13	1.00	1.00		0.18	1.00			0.74	1.00	0.72	1.00
Satd. Flow (perm)	235	5085	1583		337	5081			1369	1583	1340	1863
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Adj. Flow (vph)	4	1378	80	5	142	1686	10	53	5	83	25	13
RTOR Reduction (vph)	0	0	10	0	0	0	0	0	0	78	0	0
Lane Group Flow (vph)	4	1378	70	0	147	1696	0	0	58	5	25	13
Turn Type	Perm	NA	Perm	Perm	Perm	NA		Perm	NA	Perm	Perm	NA
Protected Phases		6				2			4			8
Permitted Phases	6		6	2	2			4		4	8	
Actuated Green, G (s)	202.0	202.0	202.0		202.0	202.0			13.2	13.2	13.2	13.2
Effective Green, g (s)	202.0	202.0	202.0		202.0	202.0			13.2	13.2	13.2	13.2
Actuated g/C Ratio	0.88	0.88	0.88		0.88	0.88			0.06	0.06	0.06	0.06
Clearance Time (s)	6.8	6.8	6.8		6.8	6.8			8.0	8.0	8.0	8.0
Vehicle Extension (s)	1.0	1.0	1.0		1.0	1.0			1.0	1.0	1.0	1.0
Lane Grp Cap (vph)	206	4465	1390		295	4462			78	90	76	106
v/s Ratio Prot		0.27				0.33						0.01
v/s Ratio Perm	0.02		0.04		c0.44				c0.04	0.00	0.02	
v/c Ratio	0.02	0.31	0.05		0.50	0.38			0.74	0.05	0.33	0.12
Uniform Delay, d1	1.7	2.3	1.8		3.0	2.6			106.7	102.5	104.1	102.9
Progression Factor	1.17	1.08	2.01		1.56	0.15			1.00	1.00	1.00	1.00
Incremental Delay, d2	0.0	0.0	0.0		3.6	0.1			28.0	0.1	0.9	0.2
Delay (s)	2.0	2.5	3.6		8.3	0.5			134.7	102.6	105.1	103.1
Level of Service	A	A	A		A	A			F	F	F	F
Approach Delay (s)		2.6				1.1			115.8			104.4
Approach LOS		A				A			F			F

### Intersection Summary

HCM 2000 Control Delay	7.5	HCM 2000 Level of Service	A
HCM 2000 Volume to Capacity ratio	0.51		
Actuated Cycle Length (s)	230.0	Sum of lost time (s)	14.8
Intersection Capacity Utilization	66.7%	ICU Level of Service	C
Analysis Period (min)	15		

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis  
 20: Lake Haven Rd & SR 580

06/24/2021



Movement	SBR
Lane Configurations	
Traffic Volume (vph)	0
Future Volume (vph)	0
Ideal Flow (vphpl)	1900
Total Lost time (s)	
Lane Util. Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Peak-hour factor, PHF	0.93
Adj. Flow (vph)	0
RTOR Reduction (vph)	0
Lane Group Flow (vph)	0
Turn Type	
Protected Phases	
Permitted Phases	
Actuated Green, G (s)	
Effective Green, g (s)	
Actuated g/C Ratio	
Clearance Time (s)	
Vehicle Extension (s)	
Lane Grp Cap (vph)	
v/s Ratio Prot	
v/s Ratio Perm	
v/c Ratio	
Uniform Delay, d1	
Progression Factor	
Incremental Delay, d2	
Delay (s)	
Level of Service	
Approach Delay (s)	
Approach LOS	
Intersection Summary	

# HCM Signalized Intersection Capacity Analysis

## 23: Keene Rd/CR 1 & SR 580

06/24/2021



Movement	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBL
Lane Configurations		↔↔	↔↔↔		↔↔	↔↔↔	↔		↔	↔↔	↔	↔
Traffic Volume (vph)	4	94	1145	124	358	1640	354	2	190	604	286	367
Future Volume (vph)	4	94	1145	124	358	1640	354	2	190	604	286	367
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		7.3	7.3		7.3	7.3	7.3		7.1	7.1	7.1	7.4
Lane Util. Factor		0.97	0.91		0.97	0.91	1.00		1.00	0.95	1.00	0.91
Frt		1.00	0.99		1.00	1.00	0.85		1.00	1.00	0.85	1.00
Flt Protected		0.95	1.00		0.95	1.00	1.00		0.95	1.00	1.00	0.95
Satd. Flow (prot)		3433	5011		3433	5085	1583		1770	3539	1583	1610
Flt Permitted		0.95	1.00		0.95	1.00	1.00		0.95	1.00	1.00	0.95
Satd. Flow (perm)		3433	5011		3433	5085	1583		1770	3539	1583	1610
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Adj. Flow (vph)	4	101	1231	133	385	1763	381	2	204	649	308	395
RTOR Reduction (vph)	0	0	5	0	0	0	53	0	0	0	111	0
Lane Group Flow (vph)	0	105	1359	0	385	1763	328	0	206	649	197	344
Turn Type	Prot	Prot	NA		Prot	NA	Perm	Split	Split	NA	Perm	Split
Protected Phases	1	1	6		5	2		4	4	4		8
Permitted Phases							2					4
Actuated Green, G (s)		9.7	74.4		28.9	93.6	93.6		44.9	44.9	44.9	52.7
Effective Green, g (s)		9.7	74.4		28.9	93.6	93.6		44.9	44.9	44.9	52.7
Actuated g/C Ratio		0.04	0.32		0.13	0.41	0.41		0.20	0.20	0.20	0.23
Clearance Time (s)		7.3	7.3		7.3	7.3	7.3		7.1	7.1	7.1	7.4
Vehicle Extension (s)		1.0	1.0		1.0	1.0	1.0		1.0	1.0	1.0	1.0
Lane Grp Cap (vph)		144	1620		431	2069	644		345	690	309	368
v/s Ratio Prot		0.03	0.27		c0.11	c0.35			0.12	c0.18		c0.21
v/s Ratio Perm							0.21					0.12
v/c Ratio		0.73	0.84		0.89	0.85	0.51		0.60	0.94	0.64	0.93
Uniform Delay, d1		108.9	72.2		99.0	61.9	51.0		84.3	91.2	85.1	87.0
Progression Factor		0.98	0.75		0.72	0.62	0.46		1.00	1.00	1.00	1.00
Incremental Delay, d2		14.0	5.2		16.1	3.6	2.2		1.9	20.7	3.2	30.2
Delay (s)		120.3	59.1		87.3	41.9	25.7		86.2	112.0	88.2	117.2
Level of Service		F	E		F	D	C		F	F	F	F
Approach Delay (s)			63.5			46.4				101.1		
Approach LOS			E			D				F		

### Intersection Summary

HCM 2000 Control Delay	71.2	HCM 2000 Level of Service	E
HCM 2000 Volume to Capacity ratio	0.92		
Actuated Cycle Length (s)	230.0	Sum of lost time (s)	29.1
Intersection Capacity Utilization	95.4%	ICU Level of Service	F
Analysis Period (min)	15		

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis  
 23: Keene Rd/CR 1 & SR 580

06/24/2021



Movement	SBT	SBR
Lane Configurations	↕↕	↗
Traffic Volume (vph)	622	179
Future Volume (vph)	622	179
Ideal Flow (vphpl)	1900	1900
Total Lost time (s)	7.4	7.4
Lane Util. Factor	0.91	1.00
Frt	1.00	0.85
Flt Protected	1.00	1.00
Satd. Flow (prot)	3378	1583
Flt Permitted	1.00	1.00
Satd. Flow (perm)	3378	1583
Peak-hour factor, PHF	0.93	0.93
Adj. Flow (vph)	669	192
RTOR Reduction (vph)	0	140
Lane Group Flow (vph)	720	52
Turn Type	NA	Perm
Protected Phases	8	
Permitted Phases		8
Actuated Green, G (s)	52.7	52.7
Effective Green, g (s)	52.7	52.7
Actuated g/C Ratio	0.23	0.23
Clearance Time (s)	7.4	7.4
Vehicle Extension (s)	1.0	1.0
Lane Grp Cap (vph)	774	362
v/s Ratio Prot	0.21	
v/s Ratio Perm		0.03
v/c Ratio	0.93	0.14
Uniform Delay, d1	86.8	70.6
Progression Factor	1.00	1.00
Incremental Delay, d2	17.4	0.1
Delay (s)	104.3	70.7
Level of Service	F	E
Approach Delay (s)	102.7	
Approach LOS	F	
Intersection Summary		

# HCM Signalized Intersection Capacity Analysis

## 26: Achieva Way/Sunlight Dr & SR 580

06/24/2021



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBL	SBT
Lane Configurations	↗	↑↑↑		↖	↑↑↑			↖	↑	↗	↖	↘
Traffic Volume (vph)	20	1791	59	304	2317	21	5	51	17	310	151	50
Future Volume (vph)	20	1791	59	304	2317	21	5	51	17	310	151	50
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	7.2	7.2		7.2	7.2			7.6	7.6	7.6	7.6	7.6
Lane Util. Factor	1.00	0.91		1.00	0.91			1.00	1.00	1.00	1.00	1.00
Frt	1.00	1.00		1.00	1.00			1.00	1.00	0.85	1.00	0.92
Flt Protected	0.95	1.00		0.95	1.00			0.95	1.00	1.00	0.95	1.00
Satd. Flow (prot)	1770	5061		1770	5078			1770	1863	1583	1770	1718
Flt Permitted	0.05	1.00		0.05	1.00			0.56	1.00	1.00	0.75	1.00
Satd. Flow (perm)	87	5061		100	5078			1039	1863	1583	1389	1718
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Adj. Flow (vph)	22	1926	63	327	2491	23	5	55	18	333	162	54
RTOR Reduction (vph)	0	1	0	0	0	0	0	0	0	289	0	18
Lane Group Flow (vph)	22	1988	0	327	2514	0	0	60	18	44	162	94
Turn Type	pm+pt	NA		pm+pt	NA		Perm	Perm	NA	Perm	Perm	NA
Protected Phases	1	6		5	2				4			8
Permitted Phases	6			2			4	4		4	8	
Actuated Green, G (s)	142.4	136.8		184.8	172.0			30.4	30.4	30.4	30.4	30.4
Effective Green, g (s)	142.4	136.8		184.8	172.0			30.4	30.4	30.4	30.4	30.4
Actuated g/C Ratio	0.62	0.59		0.80	0.75			0.13	0.13	0.13	0.13	0.13
Clearance Time (s)	7.2	7.2		7.2	7.2			7.6	7.6	7.6	7.6	7.6
Vehicle Extension (s)	1.0	1.0		1.0	1.0			1.0	1.0	1.0	1.0	1.0
Lane Grp Cap (vph)	94	3010		376	3797			137	246	209	183	227
v/s Ratio Prot	0.01	0.39		c0.15	0.49				0.01			0.05
v/s Ratio Perm	0.14			c0.54				0.06		0.03	c0.12	
v/c Ratio	0.23	0.66		0.87	0.66			0.44	0.07	0.21	0.89	0.41
Uniform Delay, d1	17.8	31.1		71.8	14.5			91.9	87.5	89.1	98.1	91.6
Progression Factor	0.41	0.30		0.75	1.05			1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.3	0.7		14.1	0.7			0.8	0.0	0.2	35.3	0.4
Delay (s)	7.6	10.1		67.9	15.8			92.7	87.5	89.3	133.4	92.1
Level of Service	A	B		E	B			F	F	F	F	F
Approach Delay (s)		10.1			21.8				89.7			116.5
Approach LOS		B			C				F			F

### Intersection Summary

HCM 2000 Control Delay	27.3	HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio	0.88		
Actuated Cycle Length (s)	230.0	Sum of lost time (s)	22.0
Intersection Capacity Utilization	86.1%	ICU Level of Service	E
Analysis Period (min)	15		

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis  
 26: Achieva Way/Sunlight Dr & SR 580

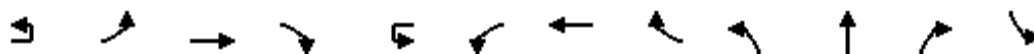
06/24/2021

Movement	SBR
Lane Configurations	
Traffic Volume (vph)	54
Future Volume (vph)	54
Ideal Flow (vphpl)	1900
Total Lost time (s)	
Lane Util. Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Peak-hour factor, PHF	0.93
Adj. Flow (vph)	58
RTOR Reduction (vph)	0
Lane Group Flow (vph)	0
Turn Type	
Protected Phases	
Permitted Phases	
Actuated Green, G (s)	
Effective Green, g (s)	
Actuated g/C Ratio	
Clearance Time (s)	
Vehicle Extension (s)	
Lane Grp Cap (vph)	
v/s Ratio Prot	
v/s Ratio Perm	
v/c Ratio	
Uniform Delay, d1	
Progression Factor	
Incremental Delay, d2	
Delay (s)	
Level of Service	
Approach Delay (s)	
Approach LOS	
Intersection Summary	

# HCM Signalized Intersection Capacity Analysis

## 29: Overcash Dr & SR 580

06/24/2021



Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL
Lane Configurations		↖	↑↑↑			↖	↑↑↑	↗		↑	↗	
Traffic Volume (vph)	33	30	2024	15	4	96	2515	47	54	0	41	61
Future Volume (vph)	33	30	2024	15	4	96	2515	47	54	0	41	61
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		7.2	7.2			7.2	7.2	7.2		7.6	7.6	
Lane Util. Factor		1.00	0.91			1.00	0.91	1.00		1.00	1.00	
Frt		1.00	1.00			1.00	1.00	0.85		1.00	0.85	
Flt Protected		0.95	1.00			0.95	1.00	1.00		0.95	1.00	
Satd. Flow (prot)		1770	5080			1770	5085	1583		1770	1583	
Flt Permitted		0.03	1.00			0.06	1.00	1.00		0.64	1.00	
Satd. Flow (perm)		61	5080			109	5085	1583		1185	1583	
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Adj. Flow (vph)	35	32	2176	16	4	103	2704	51	58	0	44	66
RTOR Reduction (vph)	0	0	0	0	0	0	0	12	0	0	41	0
Lane Group Flow (vph)	0	67	2192	0	0	107	2704	39	0	58	3	0
Turn Type	pm+pt	pm+pt	NA		pm+pt	pm+pt	NA	Perm	Perm	NA	Perm	Perm
Protected Phases	1	1	6		5	5	2			4		
Permitted Phases	6	6			2	2		2	4		4	8
Actuated Green, G (s)		181.3	181.3			176.7	176.7	176.7		16.5	16.5	
Effective Green, g (s)		181.3	181.3			176.7	176.7	176.7		16.5	16.5	
Actuated g/C Ratio		0.79	0.79			0.77	0.77	0.77		0.07	0.07	
Clearance Time (s)		7.2	7.2			7.2	7.2	7.2		7.6	7.6	
Vehicle Extension (s)		1.0	1.0			1.0	1.0	1.0		1.0	1.0	
Lane Grp Cap (vph)		158	4004			157	3906	1216		85	113	
v/s Ratio Prot		0.03	c0.43			0.03	c0.53					
v/s Ratio Perm		0.30				0.49		0.02		0.05	0.00	
v/c Ratio		0.42	0.55			0.68	0.69	0.03		0.68	0.03	
Uniform Delay, d1		42.4	9.1			19.8	13.2	6.3		104.2	99.3	
Progression Factor		0.60	0.74			1.73	0.46	0.61		1.00	1.00	
Incremental Delay, d2		0.5	0.4			5.9	0.6	0.0		16.5	0.0	
Delay (s)		26.0	7.1			40.3	6.7	3.9		120.7	99.3	
Level of Service		C	A			D	A	A		F	F	
Approach Delay (s)			7.7				7.9			111.5		
Approach LOS			A				A			F		

### Intersection Summary

HCM 2000 Control Delay	12.9	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.70		
Actuated Cycle Length (s)	230.0	Sum of lost time (s)	22.0
Intersection Capacity Utilization	90.8%	ICU Level of Service	E
Analysis Period (min)	15		

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis  
 29: Overcash Dr & SR 580

06/24/2021

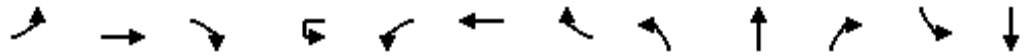


Movement	SBT	SBR
Lane Configurations	↕	↗
Traffic Volume (vph)	8	72
Future Volume (vph)	8	72
Ideal Flow (vphpl)	1900	1900
Total Lost time (s)	7.6	7.6
Lane Util. Factor	1.00	1.00
Frt	1.00	0.85
Flt Protected	0.96	1.00
Satd. Flow (prot)	1784	1583
Flt Permitted	0.71	1.00
Satd. Flow (perm)	1323	1583
Peak-hour factor, PHF	0.93	0.93
Adj. Flow (vph)	9	77
RTOR Reduction (vph)	0	71
Lane Group Flow (vph)	75	6
Turn Type	NA	Perm
Protected Phases	8	
Permitted Phases		8
Actuated Green, G (s)	16.5	16.5
Effective Green, g (s)	16.5	16.5
Actuated g/C Ratio	0.07	0.07
Clearance Time (s)	7.6	7.6
Vehicle Extension (s)	1.0	1.0
Lane Grp Cap (vph)	94	113
v/s Ratio Prot		
v/s Ratio Perm	c0.06	0.00
v/c Ratio	0.80	0.05
Uniform Delay, d1	105.1	99.4
Progression Factor	1.00	1.00
Incremental Delay, d2	34.1	0.1
Delay (s)	139.2	99.5
Level of Service	F	F
Approach Delay (s)	119.1	
Approach LOS	F	
Intersection Summary		



HCM Signalized Intersection Capacity Analysis  
 32: Pinewood Dr/King Arthur Ct & SR 580

06/24/2021



Movement	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations	↗	↑↑↑			↖	↑↑↑			↕			↙
Traffic Volume (vph)	61	2039	17	2	10	2611	39	24	4	12	58	6
Future Volume (vph)	61	2039	17	2	10	2611	39	24	4	12	58	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	7.4	7.4			7.4	7.4			7.2			7.2
Lane Util. Factor	1.00	0.91			1.00	0.91			1.00			1.00
Frt	1.00	1.00			1.00	1.00			0.96			1.00
Flt Protected	0.95	1.00			0.95	1.00			0.97			0.96
Satd. Flow (prot)	1770	5079			1770	5074			1734			1782
Flt Permitted	0.95	1.00			0.95	1.00			0.78			0.71
Satd. Flow (perm)	1770	5079			1770	5074			1386			1329
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Adj. Flow (vph)	66	2192	18	2	11	2808	42	26	4	13	62	6
RTOR Reduction (vph)	0	1	0	0	0	1	0	0	12	0	0	0
Lane Group Flow (vph)	66	2209	0	0	13	2849	0	0	31	0	0	68
Turn Type	Prot	NA		Prot	Prot	NA		Perm	NA		Perm	NA
Protected Phases	1	6		5	5	2			4			8
Permitted Phases								4			8	
Actuated Green, G (s)	9.2	81.7			2.8	75.3			8.5			8.5
Effective Green, g (s)	9.2	81.7			2.8	75.3			8.5			8.5
Actuated g/C Ratio	0.08	0.71			0.02	0.65			0.07			0.07
Clearance Time (s)	7.4	7.4			7.4	7.4			7.2			7.2
Vehicle Extension (s)	1.0	1.0			1.0	1.0			1.0			1.0
Lane Grp Cap (vph)	141	3608			43	3322			102			98
v/s Ratio Prot	0.04	c0.44			0.01	c0.56						
v/s Ratio Perm									0.02			c0.05
v/c Ratio	0.47	0.61			0.30	0.86			0.30			0.69
Uniform Delay, d1	50.6	8.5			55.1	15.6			50.4			52.0
Progression Factor	0.97	1.28			1.35	0.68			1.00			1.00
Incremental Delay, d2	0.8	0.7			1.0	2.1			0.6			15.8
Delay (s)	49.6	11.6			75.6	12.7			51.1			67.7
Level of Service	D	B			E	B			D			E
Approach Delay (s)		12.7				13.0			51.1			60.2
Approach LOS		B				B			D			E

Intersection Summary

HCM 2000 Control Delay	14.2	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.83		
Actuated Cycle Length (s)	115.0	Sum of lost time (s)	22.0
Intersection Capacity Utilization	81.1%	ICU Level of Service	D
Analysis Period (min)	15		

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis  
 32: Pinewood Dr/King Arthur Ct & SR 580

06/24/2021

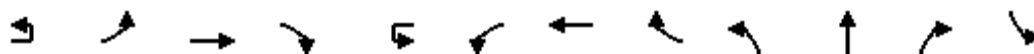


Movement	SBR
Lane Configurations	7
Traffic Volume (vph)	45
Future Volume (vph)	45
Ideal Flow (vphpl)	1900
Total Lost time (s)	7.2
Lane Util. Factor	1.00
Frt	0.85
Flt Protected	1.00
Satd. Flow (prot)	1583
Flt Permitted	1.00
Satd. Flow (perm)	1583
Peak-hour factor, PHF	0.93
Adj. Flow (vph)	48
RTOR Reduction (vph)	44
Lane Group Flow (vph)	4
Turn Type	Perm
Protected Phases	
Permitted Phases	8
Actuated Green, G (s)	8.5
Effective Green, g (s)	8.5
Actuated g/C Ratio	0.07
Clearance Time (s)	7.2
Vehicle Extension (s)	1.0
Lane Grp Cap (vph)	117
v/s Ratio Prot	
v/s Ratio Perm	0.00
v/c Ratio	0.03
Uniform Delay, d1	49.4
Progression Factor	1.00
Incremental Delay, d2	0.0
Delay (s)	49.5
Level of Service	D
Approach Delay (s)	
Approach LOS	
Intersection Summary	

# HCM Signalized Intersection Capacity Analysis

## 35: Belcher Rd & SR 580

06/24/2021



Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL
Lane Configurations		↘↘	↑↑↑	↗		↘↘	↑↑↑	↗	↘	↑↑	↗	↘
Traffic Volume (vph)	10	272	1718	118	3	157	1759	241	108	415	199	188
Future Volume (vph)	10	272	1718	118	3	157	1759	241	108	415	199	188
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		7.4	7.4	7.4		7.4	7.4	7.4	7.4	7.4	7.4	7.4
Lane Util. Factor		0.97	0.91	1.00		0.97	0.91	1.00	1.00	0.95	1.00	1.00
Frt		1.00	1.00	0.85		1.00	1.00	0.85	1.00	1.00	0.85	1.00
Flt Protected		0.95	1.00	1.00		0.95	1.00	1.00	0.95	1.00	1.00	0.95
Satd. Flow (prot)		3433	5085	1583		3433	5085	1583	1770	3539	1583	1770
Flt Permitted		0.95	1.00	1.00		0.95	1.00	1.00	0.95	1.00	1.00	0.95
Satd. Flow (perm)		3433	5085	1583		3433	5085	1583	1770	3539	1583	1770
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Adj. Flow (vph)	11	292	1847	127	3	169	1891	259	116	446	214	202
RTOR Reduction (vph)	0	0	0	76	0	0	0	161	0	0	183	0
Lane Group Flow (vph)	0	303	1847	51	0	172	1891	98	116	446	31	202
Turn Type	Prot	Prot	NA	Perm	Prot	Prot	NA	Perm	Prot	NA	Perm	Prot
Protected Phases	1	1	6		5	5	2		7	4		3
Permitted Phases				6				2			4	
Actuated Green, G (s)		10.9	46.6	46.6		7.6	43.3	43.3	9.7	16.7	16.7	14.5
Effective Green, g (s)		10.9	46.6	46.6		7.6	43.3	43.3	9.7	16.7	16.7	14.5
Actuated g/C Ratio		0.09	0.41	0.41		0.07	0.38	0.38	0.08	0.15	0.15	0.13
Clearance Time (s)		7.4	7.4	7.4		7.4	7.4	7.4	7.4	7.4	7.4	7.4
Vehicle Extension (s)		1.0	1.0	1.0		1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lane Grp Cap (vph)		325	2060	641		226	1914	596	149	513	229	223
v/s Ratio Prot		0.09	c0.36			0.05	c0.37		0.07	c0.13		0.11
v/s Ratio Perm				0.03				0.06			0.02	
v/c Ratio		0.93	0.90	0.08		0.76	0.99	0.16	0.78	0.87	0.14	0.91
Uniform Delay, d1		51.7	31.9	21.0		52.8	35.6	23.8	51.6	48.1	42.9	49.6
Progression Factor		0.99	1.01	61.93		0.77	0.83	0.69	1.00	1.00	1.00	1.00
Incremental Delay, d2		28.9	5.7	0.2		3.3	7.7	0.1	20.4	14.1	0.1	34.9
Delay (s)		80.3	37.8	1302.4		43.9	37.3	16.5	72.0	62.2	43.0	84.5
Level of Service		F	D	F		D	D	B	E	E	D	F
Approach Delay (s)			114.0			35.5			58.3			
Approach LOS			F			D			E			

### Intersection Summary

HCM 2000 Control Delay	69.3	HCM 2000 Level of Service	E
HCM 2000 Volume to Capacity ratio	1.01		
Actuated Cycle Length (s)	115.0	Sum of lost time (s)	29.6
Intersection Capacity Utilization	88.6%	ICU Level of Service	E
Analysis Period (min)	15		

c Critical Lane Group

# HCM Signalized Intersection Capacity Analysis

## 35: Belcher Rd & SR 580

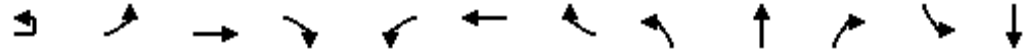
06/24/2021



Movement	SBT	SBR
Lane Configurations	↑↑	↑
Traffic Volume (vph)	485	249
Future Volume (vph)	485	249
Ideal Flow (vphpl)	1900	1900
Total Lost time (s)	7.4	7.4
Lane Util. Factor	0.95	1.00
Frt	1.00	0.85
Flt Protected	1.00	1.00
Satd. Flow (prot)	3539	1583
Flt Permitted	1.00	1.00
Satd. Flow (perm)	3539	1583
Peak-hour factor, PHF	0.93	0.93
Adj. Flow (vph)	522	268
RTOR Reduction (vph)	0	205
Lane Group Flow (vph)	522	63
Turn Type	NA	Perm
Protected Phases	8	
Permitted Phases		8
Actuated Green, G (s)	21.5	21.5
Effective Green, g (s)	21.5	21.5
Actuated g/C Ratio	0.19	0.19
Clearance Time (s)	7.4	7.4
Vehicle Extension (s)	1.0	1.0
Lane Grp Cap (vph)	661	295
v/s Ratio Prot	c0.15	
v/s Ratio Perm		0.04
v/c Ratio	0.79	0.21
Uniform Delay, d1	44.6	39.6
Progression Factor	1.00	1.00
Incremental Delay, d2	5.8	0.1
Delay (s)	50.4	39.7
Level of Service	D	D
Approach Delay (s)	54.4	
Approach LOS	D	
Intersection Summary		

HCM Signalized Intersection Capacity Analysis  
 38: US 19 Frontage & SR 580

06/24/2021



Movement	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations		↔↔	↔↔↔		↔↔	↔↔↔		↔	↔↔	↔	↔	↔↔
Traffic Volume (vph)	6	439	1243	984	812	1149	189	876	311	328	289	344
Future Volume (vph)	6	439	1243	984	812	1149	189	876	311	328	289	344
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		10.5	10.5		10.5	10.5		10.1	10.1	10.1	10.1	10.1
Lane Util. Factor		0.97	0.91		0.97	0.91		0.91	0.91	1.00	0.91	0.91
Frt		1.00	0.93		1.00	0.98		1.00	1.00	0.85	1.00	1.00
Flt Protected		0.95	1.00		0.95	1.00		0.95	0.97	1.00	0.95	0.99
Satd. Flow (prot)		3433	4748		3433	4978		1610	3294	1583	1610	3357
Flt Permitted		0.95	1.00		0.95	1.00		0.95	0.97	1.00	0.95	0.99
Satd. Flow (perm)		3433	4748		3433	4978		1610	3294	1583	1610	3357
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Adj. Flow (vph)	6	472	1337	1058	873	1235	203	942	334	353	311	370
RTOR Reduction (vph)	0	0	62	0	0	10	0	0	0	146	0	0
Lane Group Flow (vph)	0	478	2333	0	873	1428	0	471	805	207	221	460
Turn Type	Prot	Prot	NA		Prot	NA		Split	NA	Perm	Split	NA
Protected Phases	1	1	6		5	2		4	4		8	8
Permitted Phases										4		
Actuated Green, G (s)		38.8	80.5		42.5	84.2		45.9	45.9	45.9	19.9	19.9
Effective Green, g (s)		38.8	80.5		42.5	84.2		45.9	45.9	45.9	19.9	19.9
Actuated g/C Ratio		0.17	0.35		0.18	0.37		0.20	0.20	0.20	0.09	0.09
Clearance Time (s)		10.5	10.5		10.5	10.5		10.1	10.1	10.1	10.1	10.1
Vehicle Extension (s)		1.0	1.0		1.0	1.0		1.0	1.0	1.0	1.0	1.0
Lane Grp Cap (vph)		579	1661		634	1822		321	657	315	139	290
v/s Ratio Prot		0.14	c0.49		c0.25	0.29		c0.29	0.24		c0.14	0.14
v/s Ratio Perm										0.13		
v/c Ratio		0.83	1.72dr		1.38	0.78		1.47	1.41dl	0.66	1.59	1.59
Uniform Delay, d1		92.3	74.8		93.8	64.8		92.0	92.0	84.8	105.0	105.0
Progression Factor		0.83	0.78		0.75	0.83		1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2		7.0	185.0		178.0	2.9		226.6	114.6	3.7	296.7	279.6
Delay (s)		83.7	243.6		247.9	56.5		318.7	206.6	88.5	401.7	384.7
Level of Service		F	F		F	E		F	F	F	F	F
Approach Delay (s)			217.0			128.8			213.4			339.7
Approach LOS			F			F			F			F

Intersection Summary

HCM 2000 Control Delay	206.7	HCM 2000 Level of Service	F
HCM 2000 Volume to Capacity ratio	1.43		
Actuated Cycle Length (s)	230.0	Sum of lost time (s)	41.2
Intersection Capacity Utilization	139.8%	ICU Level of Service	H
Analysis Period (min)	15		

dl Defacto Left Lane. Recode with 1 though lane as a left lane.

dr Defacto Right Lane. Recode with 1 though lane as a right lane.

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis  
 38: US 19 Frontage & SR 580

06/24/2021

Movement	SBR
Lane Configurations	
Traffic Volume (vph)	344
Future Volume (vph)	344
Ideal Flow (vphpl)	1900
Total Lost time (s)	10.1
Lane Util. Factor	1.00
Frt	0.85
Flt Protected	1.00
Satd. Flow (prot)	1583
Flt Permitted	1.00
Satd. Flow (perm)	1583
Peak-hour factor, PHF	0.93
Adj. Flow (vph)	370
RTOR Reduction (vph)	206
Lane Group Flow (vph)	164
Turn Type	Perm
Protected Phases	
Permitted Phases	8
Actuated Green, G (s)	19.9
Effective Green, g (s)	19.9
Actuated g/C Ratio	0.09
Clearance Time (s)	10.1
Vehicle Extension (s)	1.0
Lane Grp Cap (vph)	136
v/s Ratio Prot	
v/s Ratio Perm	0.10
v/c Ratio	1.20
Uniform Delay, d1	105.0
Progression Factor	1.00
Incremental Delay, d2	141.7
Delay (s)	246.8
Level of Service	F
Approach Delay (s)	
Approach LOS	
Intersection Summary	

# HCM Signalized Intersection Capacity Analysis

## 41: Summerdale Dr & SR 580

06/24/2021



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↗↖↗		↖	↗↖↗		↖	↗		↖	↗	
Traffic Volume (vph)	89	1605	16	0	1836	59	49	21	14	55	0	168
Future Volume (vph)	89	1605	16	0	1836	59	49	21	14	55	0	168
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	6.8	7.0			7.0		7.5	7.5		7.5	7.5	
Lane Util. Factor	1.00	0.91			0.91		1.00	1.00		1.00	1.00	
Frt	1.00	1.00			1.00		1.00	0.94		1.00	0.85	
Flt Protected	0.95	1.00			1.00		0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1770	5078			5062		1770	1752		1770	1583	
Flt Permitted	0.06	1.00			1.00		0.41	1.00		0.73	1.00	
Satd. Flow (perm)	118	5078			5062		768	1752		1364	1583	
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Adj. Flow (vph)	96	1726	17	0	1974	63	53	23	15	59	0	181
RTOR Reduction (vph)	0	0	0	0	2	0	0	14	0	0	166	0
Lane Group Flow (vph)	96	1743	0	0	2035	0	53	24	0	59	15	0
Turn Type	pm+pt	NA		pm+pt	NA		Perm	NA		Perm	NA	
Protected Phases	1	6		5	2			4			8	
Permitted Phases	6			2			4			8		
Actuated Green, G (s)	90.8	90.8			78.2		9.7	9.7		9.7	9.7	
Effective Green, g (s)	90.8	90.8			78.2		9.7	9.7		9.7	9.7	
Actuated g/C Ratio	0.79	0.79			0.68		0.08	0.08		0.08	0.08	
Clearance Time (s)	6.8	7.0			7.0		7.5	7.5		7.5	7.5	
Vehicle Extension (s)	1.0	1.0			1.0		1.0	1.0		1.0	1.0	
Lane Grp Cap (vph)	176	4009			3442		64	147		115	133	
v/s Ratio Prot	0.03	c0.34			c0.40			0.01			0.01	
v/s Ratio Perm	0.40						c0.07			0.04		
v/c Ratio	0.55	0.43			0.59		0.83	0.17		0.51	0.11	
Uniform Delay, d1	9.4	3.9			9.8		51.8	48.9		50.4	48.7	
Progression Factor	3.45	0.29			1.59		1.00	1.00		1.00	1.00	
Incremental Delay, d2	0.2	0.0			0.6		54.0	0.2		1.6	0.1	
Delay (s)	32.6	1.2			16.3		105.8	49.1		52.0	48.8	
Level of Service	C	A			B		F	D		D	D	
Approach Delay (s)		2.8			16.3			82.1			49.6	
Approach LOS		A			B			F			D	

### Intersection Summary

HCM 2000 Control Delay	13.7	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.63		
Actuated Cycle Length (s)	115.0	Sum of lost time (s)	21.4
Intersection Capacity Utilization	80.3%	ICU Level of Service	D
Analysis Period (min)	15		

c Critical Lane Group

# HCM Signalized Intersection Capacity Analysis

## 44: Countryside Blvd & SR 580

06/24/2021



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↗	↗↗↗		↗	↗↗↗		↗	↗↗	↗	↗	↗↗	↗
Traffic Volume (vph)	78	1387	42	651	1564	70	42	89	755	87	244	206
Future Volume (vph)	78	1387	42	651	1564	70	42	89	755	87	244	206
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	6.9	7.2		7.0	7.2		6.9	7.3	7.3	6.7	7.3	7.3
Lane Util. Factor	1.00	0.91		1.00	0.91		1.00	0.95	1.00	1.00	0.95	1.00
Frt	1.00	1.00		1.00	0.99		1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00		0.95	1.00		0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1770	5063		1770	5053		1770	3539	1583	1770	3539	1583
Flt Permitted	0.12	1.00		0.07	1.00		0.51	1.00	1.00	0.67	1.00	1.00
Satd. Flow (perm)	229	5063		131	5053		948	3539	1583	1257	3539	1583
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Adj. Flow (vph)	84	1491	45	700	1682	75	45	96	812	94	262	222
RTOR Reduction (vph)	0	1	0	0	2	0	0	0	496	0	0	111
Lane Group Flow (vph)	84	1535	0	700	1755	0	45	96	316	94	262	111
Turn Type	pm+pt	NA		pm+pt	NA		pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases	1	6		5	2		7	4		3	8	
Permitted Phases	6			2			4		4	8		8
Actuated Green, G (s)	69.8	69.5		135.9	135.9		51.7	47.6	47.6	54.3	48.8	48.8
Effective Green, g (s)	69.8	69.5		135.9	135.9		51.7	47.6	47.6	54.3	48.8	48.8
Actuated g/C Ratio	0.30	0.30		0.59	0.59		0.22	0.21	0.21	0.24	0.21	0.21
Clearance Time (s)	6.9	7.2		7.0	7.2		6.9	7.3	7.3	6.7	7.3	7.3
Vehicle Extension (s)	1.0	1.0		1.0	1.0		1.0	1.0	1.0	1.0	1.0	1.0
Lane Grp Cap (vph)	155	1529		641	2985		227	732	327	309	750	335
v/s Ratio Prot	0.03	c0.30		c0.38	0.35		0.00	0.03		c0.01	0.07	
v/s Ratio Perm	0.13			c0.27			0.04		c0.20	0.06		0.07
v/c Ratio	0.54	1.00		1.09	0.59		0.20	0.13	0.96	0.30	0.35	0.33
Uniform Delay, d1	76.1	80.2		68.5	29.5		71.7	74.3	90.4	71.6	77.1	76.8
Progression Factor	0.85	0.83		0.93	1.01		1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	1.9	23.0		57.1	0.6		0.2	0.0	39.9	0.2	0.1	0.2
Delay (s)	66.4	89.7		120.6	30.2		71.8	74.4	130.3	71.8	77.2	77.0
Level of Service	E	F		F	C		E	E	F	E	E	E
Approach Delay (s)		88.5			56.0			121.9			76.2	
Approach LOS		F			E			F			E	

### Intersection Summary

HCM 2000 Control Delay	78.7	HCM 2000 Level of Service	E
HCM 2000 Volume to Capacity ratio	1.02		
Actuated Cycle Length (s)	230.0	Sum of lost time (s)	28.4
Intersection Capacity Utilization	98.4%	ICU Level of Service	F
Analysis Period (min)	15		

c Critical Lane Group



# HCM Signalized Intersection Capacity Analysis

## 47: Landmark Dr & SR 580

06/24/2021



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↑↑↑		↖	↑↑↑		↖	↑↑		↖	↑↑	
Traffic Volume (vph)	59	1495	264	52	1853	40	243	103	77	64	130	151
Future Volume (vph)	59	1495	264	52	1853	40	243	103	77	64	130	151
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	6.9	6.9		6.9	6.9		6.2	6.8		6.2	6.8	
Lane Util. Factor	1.00	0.91		1.00	0.91		1.00	0.95		1.00	0.95	
Frt	1.00	0.98		1.00	1.00		1.00	0.94		1.00	0.92	
Flt Protected	0.95	1.00		0.95	1.00		0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1770	4971		1770	5069		1770	3312		1770	3254	
Flt Permitted	0.07	1.00		0.08	1.00		0.26	1.00		0.63	1.00	
Satd. Flow (perm)	137	4971		140	5069		475	3312		1173	3254	
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Adj. Flow (vph)	63	1608	284	56	1992	43	261	111	83	69	140	162
RTOR Reduction (vph)	0	19	0	0	1	0	0	68	0	0	112	0
Lane Group Flow (vph)	63	1873	0	56	2034	0	261	126	0	69	190	0
Turn Type	pm+pt	NA		pm+pt	NA		pm+pt	NA		pm+pt	NA	
Protected Phases	1	6		5	2		7	4		3	8	
Permitted Phases	6			2			4			8		
Actuated Green, G (s)	59.4	59.4		58.6	58.6		30.9	20.1		15.6	11.0	
Effective Green, g (s)	59.4	59.4		58.6	58.6		30.9	20.1		15.6	11.0	
Actuated g/C Ratio	0.52	0.52		0.51	0.51		0.27	0.17		0.14	0.10	
Clearance Time (s)	6.9	6.9		6.9	6.9		6.2	6.8		6.2	6.8	
Vehicle Extension (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lane Grp Cap (vph)	140	2567		129	2582		281	578		183	311	
v/s Ratio Prot	0.02	c0.38		0.02	c0.40		c0.11	0.04		0.02	0.06	
v/s Ratio Perm	0.21			0.21			c0.14			0.04		
v/c Ratio	0.45	0.73		0.43	0.79		0.93	0.22		0.38	0.61	
Uniform Delay, d1	21.8	21.6		31.9	23.1		37.1	40.7		44.7	49.9	
Progression Factor	1.67	0.85		0.66	0.64		1.00	1.00		1.00	1.00	
Incremental Delay, d2	0.3	0.6		0.7	2.1		34.5	0.1		0.5	2.5	
Delay (s)	36.6	19.0		21.8	17.0		71.6	40.8		45.2	52.4	
Level of Service	D	B		C	B		E	D		D	D	
Approach Delay (s)		19.6			17.1			58.5			51.1	
Approach LOS		B			B			E			D	

### Intersection Summary

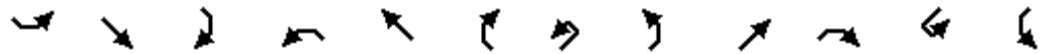
HCM 2000 Control Delay	24.6	HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio	0.90		
Actuated Cycle Length (s)	115.0	Sum of lost time (s)	26.8
Intersection Capacity Utilization	85.1%	ICU Level of Service	E
Analysis Period (min)	15		

c Critical Lane Group

# HCM Signalized Intersection Capacity Analysis

## 51: SR 580 & Charles Ave

06/24/2021



Movement	SEL	SET	SER	NWL	NWT	NWR	NEU	NEL	NET	NER	SWU	SWL
Lane Configurations		↕	↕		↕			↕	↕↕↕			↕
Traffic Volume (vph)	19	19	13	33	9	18	1	89	1418	38	65	34
Future Volume (vph)	19	19	13	33	9	18	1	89	1418	38	65	34
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		7.6	7.6		7.6			6.9	6.9			7.1
Lane Util. Factor		1.00	1.00		1.00			1.00	0.91			1.00
Frt		1.00	0.85		0.96			1.00	1.00			1.00
Flt Protected		0.98	1.00		0.97			0.95	1.00			0.95
Satd. Flow (prot)		1817	1583		1740			1770	5065			1770
Flt Permitted		0.83	1.00		0.81			0.09	1.00			0.11
Satd. Flow (perm)		1549	1583		1444			171	5065			196
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Adj. Flow (vph)	20	20	14	35	10	19	1	96	1525	41	70	37
RTOR Reduction (vph)	0	0	13	0	15	0	0	0	2	0	0	0
Lane Group Flow (vph)	0	40	1	0	49	0	0	97	1564	0	0	107
Turn Type	Perm	NA	Perm	Perm	NA		pm+pt	pm+pt	NA		pm+pt	pm+pt
Protected Phases		8			4		1	1	6		5	5
Permitted Phases	8		8	4			6	6			2	2
Actuated Green, G (s)		9.0	9.0		9.0			74.8	74.8			72.5
Effective Green, g (s)		9.0	9.0		9.0			74.8	74.8			72.5
Actuated g/C Ratio		0.08	0.08		0.08			0.65	0.65			0.63
Clearance Time (s)		7.6	7.6		7.6			6.9	6.9			7.1
Vehicle Extension (s)		4.0	4.0		4.0			4.0	5.0			4.0
Lane Grp Cap (vph)		121	123		113			279	3294			254
v/s Ratio Prot								0.04	c0.31			0.04
v/s Ratio Perm		0.03	0.00		c0.03			0.19				0.23
v/c Ratio		0.33	0.01		0.44			0.35	0.47			0.42
Uniform Delay, d1		50.1	48.9		50.6			18.3	10.2			10.9
Progression Factor		1.00	1.00		1.00			0.35	0.28			1.61
Incremental Delay, d2		2.2	0.0		3.6			0.8	0.4			0.7
Delay (s)		52.3	48.9		54.2			7.1	3.2			18.2
Level of Service		D	D		D			A	A			B
Approach Delay (s)		51.5			54.2			3.5				
Approach LOS		D			D			A				

### Intersection Summary

HCM 2000 Control Delay	7.3	HCM 2000 Level of Service	A
HCM 2000 Volume to Capacity ratio	0.59		
Actuated Cycle Length (s)	115.0	Sum of lost time (s)	21.6
Intersection Capacity Utilization	72.8%	ICU Level of Service	C
Analysis Period (min)	15		

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis  
 51: SR 580 & Charles Ave

06/24/2021

























Movement	SWT	SWR
Lane Configurations	↑↑↑	
Traffic Volume (vph)	1781	40
Future Volume (vph)	1781	40
Ideal Flow (vphpl)	1900	1900
Total Lost time (s)	6.9	
Lane Util. Factor	0.91	
Frt	1.00	
Flt Protected	1.00	
Satd. Flow (prot)	5069	
Flt Permitted	1.00	
Satd. Flow (perm)	5069	
Peak-hour factor, PHF	0.93	0.93
Adj. Flow (vph)	1915	43
RTOR Reduction (vph)	1	0
Lane Group Flow (vph)	1957	0
Turn Type	NA	
Protected Phases	2	
Permitted Phases		
Actuated Green, G (s)	72.5	
Effective Green, g (s)	72.5	
Actuated g/C Ratio	0.63	
Clearance Time (s)	6.9	
Vehicle Extension (s)	5.0	
Lane Grp Cap (vph)	3195	
v/s Ratio Prot	c0.39	
v/s Ratio Perm		
v/c Ratio	0.61	
Uniform Delay, d1	12.8	
Progression Factor	0.53	
Incremental Delay, d2	0.4	
Delay (s)	7.2	
Level of Service	A	
Approach Delay (s)	7.8	
Approach LOS	A	
Intersection Summary		

# HCM Signalized Intersection Capacity Analysis

## 54: SR 580 & McMullen Booth Rd

06/24/2021

													
Movement	NBU	NBL	NBT	NBR	SBL	SBT	SBR	NEU	NEL	NET	NER	SWU	
Lane Configurations													
Traffic Volume (vph)	17	229	1647	805	109	2179	770	7	420	891	158	3	
Future Volume (vph)	17	229	1647	805	109	2179	770	7	420	891	158	3	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)		9.8	9.8	9.8	9.8	9.8	9.8		9.8	9.8	9.8		
Lane Util. Factor		0.97	0.91	1.00	0.97	0.91	1.00		0.97	0.91	1.00		
Frt		1.00	1.00	0.85	1.00	1.00	0.85		1.00	1.00	0.85		
Flt Protected		0.95	1.00	1.00	0.95	1.00	1.00		0.95	1.00	1.00		
Satd. Flow (prot)		3433	5085	1583	3433	5085	1583		3433	5085	1583		
Flt Permitted		0.95	1.00	1.00	0.95	1.00	1.00		0.95	1.00	1.00		
Satd. Flow (perm)		3433	5085	1583	3433	5085	1583		3433	5085	1583		
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	
Adj. Flow (vph)	18	246	1771	866	117	2343	828	8	452	958	170	3	
RTOR Reduction (vph)	0	0	0	182	0	0	166	0	0	0	115	0	
Lane Group Flow (vph)	0	264	1771	684	117	2343	662	0	460	958	55	0	
Turn Type	Prot	Prot	NA	Perm	Prot	NA	Perm	Prot	Prot	NA	Perm	Prot	
Protected Phases	1	1	6		5	2		3	3	8		7	
Permitted Phases				6			2					8	
Actuated Green, G (s)		10.2	84.0	84.0	7.6	81.4	81.4		40.4	74.0	74.0		
Effective Green, g (s)		10.2	84.0	84.0	7.6	81.4	81.4		40.4	74.0	74.0		
Actuated g/C Ratio		0.04	0.37	0.37	0.03	0.35	0.35		0.18	0.32	0.32		
Clearance Time (s)		9.8	9.8	9.8	9.8	9.8	9.8		9.8	9.8	9.8		
Vehicle Extension (s)		3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0		
Lane Grp Cap (vph)		152	1857	578	113	1799	560		603	1636	509		
v/s Ratio Prot		0.08	0.35		0.03	c0.46			c0.13	0.19			
v/s Ratio Perm				c0.43			0.42					0.03	
v/c Ratio		1.74	0.95	1.18	1.04	1.30	1.18		0.76	0.59	0.11		
Uniform Delay, d1		109.9	71.1	73.0	111.2	74.3	74.3		90.2	65.2	54.8		
Progression Factor		1.00	1.00	1.00	1.00	1.00	1.00		0.78	0.74	2.05		
Incremental Delay, d2		357.5	12.5	99.1	94.5	140.3	99.2		5.2	0.5	0.1		
Delay (s)		467.4	83.6	172.1	205.7	214.6	173.5		75.9	48.5	112.4		
Level of Service		F	F	F	F	F	F		E	D	F		
Approach Delay (s)			144.9			203.9				63.3			
Approach LOS			F			F				E			
<b>Intersection Summary</b>													
HCM 2000 Control Delay			164.6									HCM 2000 Level of Service	F
HCM 2000 Volume to Capacity ratio			1.17										
Actuated Cycle Length (s)			230.0									Sum of lost time (s)	39.2
Intersection Capacity Utilization			121.6%									ICU Level of Service	H
Analysis Period (min)			15										

c Critical Lane Group

# HCM Signalized Intersection Capacity Analysis

## 54: SR 580 & McMullen Booth Rd

06/24/2021



Movement	SWL	SWT	SWR
Lane Configurations			
Traffic Volume (vph)	616	998	213
Future Volume (vph)	616	998	213
Ideal Flow (vphpl)	1900	1900	1900
Total Lost time (s)	9.8	9.8	9.8
Lane Util. Factor	0.97	0.91	1.00
Frt	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00
Satd. Flow (prot)	3433	5085	1583
Flt Permitted	0.95	1.00	1.00
Satd. Flow (perm)	3433	5085	1583
Peak-hour factor, PHF	0.93	0.93	0.93
Adj. Flow (vph)	662	1073	229
RTOR Reduction (vph)	0	0	128
Lane Group Flow (vph)	665	1073	101
Turn Type	Prot	NA	Perm
Protected Phases	7	4	
Permitted Phases			4
Actuated Green, G (s)	25.2	58.8	58.8
Effective Green, g (s)	25.2	58.8	58.8
Actuated g/C Ratio	0.11	0.26	0.26
Clearance Time (s)	9.8	9.8	9.8
Vehicle Extension (s)	4.0	3.0	3.0
Lane Grp Cap (vph)	376	1299	404
v/s Ratio Prot	c0.19	c0.21	
v/s Ratio Perm			0.06
v/c Ratio	1.77	0.83	0.25
Uniform Delay, d1	102.4	80.8	68.1
Progression Factor	1.00	1.00	1.00
Incremental Delay, d2	356.6	4.4	0.3
Delay (s)	459.0	85.2	68.4
Level of Service	F	F	E
Approach Delay (s)		209.6	
Approach LOS		F	
<b>Intersection Summary</b>			

HCM Signalized Intersection Capacity Analysis  
 57: SR 580 & St Petersburg Dr/Forest Lakes Blvd

06/24/2021



Movement	SEL	SET	SER	NWL	NWT	NWR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations		↕	↕		↕		↕	↕↕	↕	↕	↕↕	↕
Traffic Volume (vph)	47	89	1217	202	69	0	474	1134	91	2	1147	29
Future Volume (vph)	47	89	1217	202	69	0	474	1134	91	2	1147	29
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		7.2	7.2		7.2		6.9	6.9	6.9	6.9	6.9	6.9
Lane Util. Factor		1.00	1.00		1.00		1.00	0.95	1.00	1.00	0.95	1.00
Frt		1.00	0.85		1.00		1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected		0.98	1.00		0.96		0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)		1831	1583		1796		1770	3539	1583	1770	3539	1583
Flt Permitted		0.81	1.00		0.66		0.07	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)		1517	1583		1238		127	3539	1583	1770	3539	1583
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Adj. Flow (vph)	51	96	1309	217	74	0	510	1219	98	2	1233	31
RTOR Reduction (vph)	0	0	233	0	0	0	0	0	39	0	0	21
Lane Group Flow (vph)	0	147	1076	0	291	0	510	1219	59	2	1233	10
Turn Type	Perm	NA	Perm	Perm	NA		pm+pt	NA	Perm	Prot	NA	Perm
Protected Phases		8			4		1	6		5		2
Permitted Phases	8		8	4			6		6			2
Actuated Green, G (s)		53.8	53.8		53.8		87.6	79.7	79.7	1.0	51.6	51.6
Effective Green, g (s)		53.8	53.8		53.8		87.6	79.7	79.7	1.0	51.6	51.6
Actuated g/C Ratio		0.35	0.35		0.35		0.56	0.51	0.51	0.01	0.33	0.33
Clearance Time (s)		7.2	7.2		7.2		6.9	6.9	6.9	6.9	6.9	6.9
Vehicle Extension (s)		3.0	3.0		3.0		4.0	4.0	4.0	3.0	5.0	5.0
Lane Grp Cap (vph)		524	547		428		379	1813	811	11	1174	525
v/s Ratio Prot							c0.25	0.34		0.00	0.35	
v/s Ratio Perm		0.10	c0.68		0.24		c0.50		0.04			0.01
v/c Ratio		0.28	1.97		0.68		1.35	0.67	0.07	0.18	1.05	0.02
Uniform Delay, d1		36.8	50.9		43.5		52.5	28.2	19.2	76.8	52.0	34.9
Progression Factor		1.00	1.00		1.00		1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2		0.3	441.4		4.3		172.2	2.0	0.2	7.8	40.5	0.1
Delay (s)		37.1	492.2		47.8		224.8	30.2	19.4	84.7	92.5	35.0
Level of Service		D	F		D		F	C	B	F	F	D
Approach Delay (s)		446.3			47.8			83.9			91.0	
Approach LOS		F			D			F			F	

Intersection Summary

HCM 2000 Control Delay	192.6	HCM 2000 Level of Service	F
HCM 2000 Volume to Capacity ratio	1.62		
Actuated Cycle Length (s)	155.5	Sum of lost time (s)	21.0
Intersection Capacity Utilization	139.6%	ICU Level of Service	H
Analysis Period (min)	15		

c Critical Lane Group

# HCM Signalized Intersection Capacity Analysis

59: SR 590/2nd St & SR 580

06/24/2021



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBL	SBT
Lane Configurations	↖	↗	↘	↖	↗	↘			↖	↗		↖
Traffic Volume (vph)	6	1432	114	429	1705	2	2	200	2	371	6	6
Future Volume (vph)	6	1432	114	429	1705	2	2	200	2	371	6	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	6.9	6.9	6.9	6.9	6.9				8.3	8.3		8.3
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95				1.00	1.00		1.00
Frt	1.00	1.00	0.85	1.00	1.00				1.00	0.85		1.00
Flt Protected	0.95	1.00	1.00	0.95	1.00				0.95	1.00		0.98
Satd. Flow (prot)	1770	3539	1583	1770	3539				1775	1583		1817
Flt Permitted	0.95	1.00	1.00	0.95	1.00				0.72	1.00		0.76
Satd. Flow (perm)	1770	3539	1583	1770	3539				1341	1583		1414
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Adj. Flow (vph)	6	1540	123	461	1833	2	2	215	2	399	6	6
RTOR Reduction (vph)	0	0	69	0	0	0	0	0	0	276	0	0
Lane Group Flow (vph)	6	1540	54	461	1835	0	0	0	219	123	0	12
Turn Type	Prot	NA	Perm	Prot	NA		Perm	Perm	NA	Perm	Perm	NA
Protected Phases	1	6		5	2				4			8
Permitted Phases			6				4	4		4	8	
Actuated Green, G (s)	1.4	55.6	55.6	29.1	83.3				18.7	18.7		18.7
Effective Green, g (s)	1.4	55.6	55.6	29.1	83.3				18.7	18.7		18.7
Actuated g/C Ratio	0.01	0.44	0.44	0.23	0.66				0.15	0.15		0.15
Clearance Time (s)	6.9	6.9	6.9	6.9	6.9				8.3	8.3		8.3
Vehicle Extension (s)	2.0	4.0	4.0	3.0	4.0				3.0	3.0		3.0
Lane Grp Cap (vph)	19	1567	701	410	2348				199	235		210
v/s Ratio Prot	0.00	c0.44		c0.26	0.52							
v/s Ratio Perm			0.03						c0.16	0.08		0.01
v/c Ratio	0.32	0.98	0.08	1.12	0.78				1.10	0.52		0.06
Uniform Delay, d1	61.6	34.5	20.2	48.2	14.7				53.4	49.3		45.8
Progression Factor	1.00	1.00	1.00	1.00	1.00				1.00	1.00		1.00
Incremental Delay, d2	3.5	19.0	0.2	82.8	2.7				93.3	2.1		0.1
Delay (s)	65.0	53.5	20.4	131.0	17.4				146.7	51.4		45.9
Level of Service	E	D	C	F	B				F	D		D
Approach Delay (s)		51.1			40.2				85.2			45.7
Approach LOS		D			D				F			D

## Intersection Summary

HCM 2000 Control Delay	50.2	HCM 2000 Level of Service	D
HCM 2000 Volume to Capacity ratio	1.04		
Actuated Cycle Length (s)	125.5	Sum of lost time (s)	22.1
Intersection Capacity Utilization	99.7%	ICU Level of Service	F
Analysis Period (min)	15		

c Critical Lane Group

# HCM Signalized Intersection Capacity Analysis

## 59: SR 590/2nd St & SR 580

06/24/2021

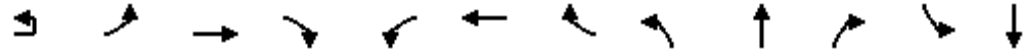


Movement	SBR
Lane Configurations	
Traffic Volume (vph)	21
Future Volume (vph)	21
Ideal Flow (vphpl)	1900
Total Lost time (s)	8.3
Lane Util. Factor	1.00
Frt	0.85
Flt Protected	1.00
Satd. Flow (prot)	1583
Flt Permitted	1.00
Satd. Flow (perm)	1583
Peak-hour factor, PHF	0.93
Adj. Flow (vph)	23
RTOR Reduction (vph)	20
Lane Group Flow (vph)	3
Turn Type	Perm
Protected Phases	
Permitted Phases	8
Actuated Green, G (s)	18.7
Effective Green, g (s)	18.7
Actuated g/C Ratio	0.15
Clearance Time (s)	8.3
Vehicle Extension (s)	3.0
Lane Grp Cap (vph)	235
v/s Ratio Prot	
v/s Ratio Perm	0.00
v/c Ratio	0.01
Uniform Delay, d1	45.5
Progression Factor	1.00
Incremental Delay, d2	0.0
Delay (s)	45.6
Level of Service	D
Approach Delay (s)	
Approach LOS	
Intersection Summary	



HCM Signalized Intersection Capacity Analysis  
68: S Bayview Blvd & SR 580

06/24/2021



Movement	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	1	225	705	6	23	1018	17	41	101	23	44	124
Future Volume (vph)	1	225	705	6	23	1018	17	41	101	23	44	124
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		6.9	6.9		6.9	6.9		7.5	7.5			7.5
Lane Util. Factor		1.00	0.95		1.00	0.95		1.00	1.00			1.00
Frt		1.00	1.00		1.00	1.00		1.00	0.97			0.94
Flt Protected		0.95	1.00		0.95	1.00		0.95	1.00			0.99
Satd. Flow (prot)		1770	3535		1770	3531		1770	1811			1740
Flt Permitted		0.22	1.00		0.34	1.00		0.32	1.00			0.92
Satd. Flow (perm)		410	3535		642	3531		589	1811			1614
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Adj. Flow (vph)	1	242	758	6	25	1095	18	44	109	25	47	133
RTOR Reduction (vph)	0	0	0	0	0	1	0	0	7	0	0	24
Lane Group Flow (vph)	0	243	764	0	25	1112	0	44	127	0	0	297
Turn Type	Perm	Perm	NA		Perm	NA		Perm	NA		Perm	NA
Protected Phases			6			2			4			8
Permitted Phases	6	6			2			4			8	
Actuated Green, G (s)		78.1	78.1		78.1	78.1		22.5	22.5			22.5
Effective Green, g (s)		78.1	78.1		78.1	78.1		22.5	22.5			22.5
Actuated g/C Ratio		0.68	0.68		0.68	0.68		0.20	0.20			0.20
Clearance Time (s)		6.9	6.9		6.9	6.9		7.5	7.5			7.5
Vehicle Extension (s)		5.0	5.0		5.0	5.0		4.0	4.0			4.0
Lane Grp Cap (vph)		278	2400		436	2398		115	354			315
v/s Ratio Prot			0.22			0.31			0.07			
v/s Ratio Perm		c0.59			0.04			0.07				c0.18
v/c Ratio		0.87	0.32		0.06	0.46		0.38	0.36			0.94
Uniform Delay, d1		14.6	7.6		6.2	8.6		40.2	40.0			45.6
Progression Factor		1.00	1.00		0.15	0.11		1.00	1.00			1.00
Incremental Delay, d2		29.5	0.3		0.2	0.4		2.9	0.8			35.9
Delay (s)		44.1	7.9		1.1	1.4		43.1	40.9			81.5
Level of Service		D	A		A	A		D	D			F
Approach Delay (s)			16.6			1.4			41.4			81.5
Approach LOS			B			A			D			F

Intersection Summary

HCM 2000 Control Delay	19.6	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.89		
Actuated Cycle Length (s)	115.0	Sum of lost time (s)	14.4
Intersection Capacity Utilization	93.0%	ICU Level of Service	F
Analysis Period (min)	15		

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis  
 68: S Bayview Blvd & SR 580

06/24/2021

Movement	SBR
Lane Configurations	
Traffic Volume (vph)	131
Future Volume (vph)	131
Ideal Flow (vphpl)	1900
Total Lost time (s)	
Lane Util. Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Peak-hour factor, PHF	0.93
Adj. Flow (vph)	141
RTOR Reduction (vph)	0
Lane Group Flow (vph)	0
Turn Type	
Protected Phases	
Permitted Phases	
Actuated Green, G (s)	
Effective Green, g (s)	
Actuated g/C Ratio	
Clearance Time (s)	
Vehicle Extension (s)	
Lane Grp Cap (vph)	
v/s Ratio Prot	
v/s Ratio Perm	
v/c Ratio	
Uniform Delay, d1	
Progression Factor	
Incremental Delay, d2	
Delay (s)	
Level of Service	
Approach Delay (s)	
Approach LOS	
Intersection Summary	

# HCM Signalized Intersection Capacity Analysis

## 71: Tampa Rd & SR 580

06/24/2021



Movement	EBL	EBR	SET	SER	NWL	NWT
Lane Configurations		↑	↑↑↑		↑↑	↑↑↑
Traffic Volume (vph)	0	829	2070	0	944	1536
Future Volume (vph)	0	829	2070	0	944	1536
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)		8.1	8.1		8.4	8.1
Lane Util. Factor		1.00	0.91		0.97	0.91
Frt		0.86	1.00		1.00	1.00
Flt Protected		1.00	1.00		0.95	1.00
Satd. Flow (prot)		1611	5085		3433	5085
Flt Permitted		1.00	1.00		0.95	1.00
Satd. Flow (perm)		1611	5085		3433	5085
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93
Adj. Flow (vph)	0	891	2226	0	1015	1652
RTOR Reduction (vph)	0	0	0	0	0	0
Lane Group Flow (vph)	0	891	2226	0	1015	1652
Turn Type		Perm	NA		Prot	NA
Protected Phases			6		5	2
Permitted Phases		2 5 6				
Actuated Green, G (s)		115.0	57.9		40.6	115.0
Effective Green, g (s)		115.0	57.9		40.6	115.0
Actuated g/C Ratio		1.00	0.50		0.35	1.00
Clearance Time (s)			8.1		8.4	8.1
Vehicle Extension (s)			1.0		1.0	1.0
Lane Grp Cap (vph)		1611	2560		1211	5085
v/s Ratio Prot			c0.44		c0.30	0.32
v/s Ratio Perm		0.55				
v/c Ratio		0.55	0.87		0.84	0.32
Uniform Delay, d1		0.0	25.2		34.2	0.0
Progression Factor		1.00	1.00		1.00	1.00
Incremental Delay, d2		0.2	4.4		5.0	0.2
Delay (s)		0.2	29.6		39.2	0.2
Level of Service		A	C		D	A
Approach Delay (s)	0.2		29.6			15.0
Approach LOS	A		C			B

### Intersection Summary

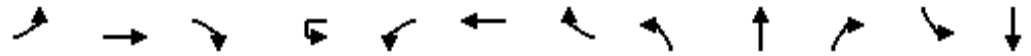
HCM 2000 Control Delay	18.3	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.86		
Actuated Cycle Length (s)	115.0	Sum of lost time (s)	16.5
Intersection Capacity Utilization	104.8%	ICU Level of Service	G
Analysis Period (min)	15		

c Critical Lane Group

# HCM Unsignalized Intersection Capacity Analysis

## 6: Douglass Ave & SR 580

06/24/2021



Movement	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT		
Lane Configurations														
Traffic Volume (veh/h)	0	396	74	2	181	524	25	48	0	127	12	6		
Future Volume (Veh/h)	0	396	74	2	181	524	25	48	0	127	12	6		
Sign Control	Free			Free			Stop			Stop				
Grade	0%			0%			0%			0%				
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93		
Hourly flow rate (vph)	0	426	80	0	195	563	27	52	0	137	13	6		
Pedestrians														
Lane Width (ft)														
Walking Speed (ft/s)														
Percent Blockage														
Right turn flare (veh)											4			
Median type	None					TWLTL								
Median storage (veh)	2													
Upstream signal (ft)	618													
pX, platoon unblocked	0.00													
vC, conflicting volume	590			0	506			1140	1446	253	1180	1472		
vC1, stage 1 conf vol								466	466				966	966
vC2, stage 2 conf vol								674	980				213	506
vCu, unblocked vol	590			0	506			1140	1446	253	1180	1472		
tC, single (s)	4.1			0.0	4.1			7.5	6.5	6.9	7.5	6.5		
tC, 2 stage (s)								6.5	5.5				6.5	5.5
tF (s)	2.2			0.0	2.2			3.5	4.0	3.3	3.5	4.0		
p0 queue free %	100			0	82			82	100	82	94	97		
cM capacity (veh/h)	982			0	1055			293	246	746	207	227		
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	WB 3	NB 1	SB 1						
Volume Total	0	284	222	195	375	215	189	21						
Volume Left	0	0	0	195	0	0	52	13						
Volume Right	0	0	80	0	0	27	137	2						
cSH	1700	1700	1700	1055	1700	1700	1030	236						
Volume to Capacity	0.00	0.17	0.13	0.18	0.22	0.13	0.18	0.09						
Queue Length 95th (ft)	0	0	0	17	0	0	17	7						
Control Delay (s)	0.0	0.0	0.0	9.2	0.0	0.0	13.4	22.2						
Lane LOS				A				B	C					
Approach Delay (s)	0.0			2.3			13.4	22.2						
Approach LOS							B	C						
Intersection Summary														
Average Delay			3.2											
Intersection Capacity Utilization			48.0%	ICU Level of Service					A					
Analysis Period (min)	15													

# HCM Unsignalized Intersection Capacity Analysis

## 6: Douglass Ave & SR 580

06/24/2021



Movement	SBR
Lane Configurations	7
Traffic Volume (veh/h)	2
Future Volume (Veh/h)	2
Sign Control	
Grade	
Peak Hour Factor	0.93
Hourly flow rate (vph)	2
Pedestrians	
Lane Width (ft)	
Walking Speed (ft/s)	
Percent Blockage	
Right turn flare (veh)	4
Median type	
Median storage (veh)	
Upstream signal (ft)	
pX, platoon unblocked	
vC, conflicting volume	295
vC1, stage 1 conf vol	
vC2, stage 2 conf vol	
vCu, unblocked vol	295
tC, single (s)	6.9
tC, 2 stage (s)	
tF (s)	3.3
p0 queue free %	100
cM capacity (veh/h)	701
Direction, Lane #	

# HCM Unsignalized Intersection Capacity Analysis

## 50: Rigsby Ln & SR 580

06/24/2021



Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL
Lane Configurations		↖	↑↑↑			↖	↑↑↑			↕		
Traffic Volume (veh/h)	1	4	1431	18	2	14	1932	3	19	0	36	0
Future Volume (Veh/h)	1	4	1431	18	2	14	1932	3	19	0	36	0
Sign Control			Free				Free			Stop		
Grade			0%				0%			0%		
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	0	4	1539	19	0	15	2077	3	20	0	39	0
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type												
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked	0.00				0.00							
vC, conflicting volume	0	2080			0	1558			2287	3666	522	2668
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	0	2080			0	1558			2287	3666	522	2668
tC, single (s)	0.0	4.1			0.0	4.1			7.5	6.5	6.9	7.5
tC, 2 stage (s)												
tF (s)	0.0	2.2			0.0	2.2			3.5	4.0	3.3	3.5
p0 queue free %	0	98			0	96			0	100	92	100
cM capacity (veh/h)	0	263			0	421			17	5	499	10
Direction, Lane #	EB 1	EB 2	EB 3	EB 4	WB 1	WB 2	WB 3	WB 4	NB 1	SB 1		
Volume Total	4	616	616	327	15	831	831	418	59	9		
Volume Left	4	0	0	0	15	0	0	0	20	0		
Volume Right	0	0	0	19	0	0	0	3	39	8		
cSH	263	1700	1700	1700	421	1700	1700	1700	46	37		
Volume to Capacity	0.02	0.36	0.36	0.19	0.04	0.49	0.49	0.25	1.28	0.24		
Queue Length 95th (ft)	1	0	0	0	3	0	0	0	140	20		
Control Delay (s)	18.9	0.0	0.0	0.0	13.9	0.0	0.0	0.0	368.5	131.7		
Lane LOS	C				B				F	F		
Approach Delay (s)	0.0				0.1				368.5	131.7		
Approach LOS									F	F		
Intersection Summary												
Average Delay			6.2									
Intersection Capacity Utilization			54.0%			ICU Level of Service				A		
Analysis Period (min)			15									

# HCM Unsignalized Intersection Capacity Analysis

## 50: Rigsby Ln & SR 580

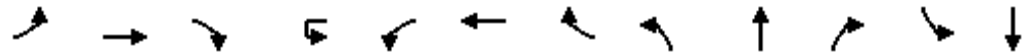
06/24/2021



Movement	SBT	SBR
Lane Configurations		
Traffic Volume (veh/h)	1	7
Future Volume (Veh/h)	1	7
Sign Control	Stop	
Grade	0%	
Peak Hour Factor	0.93	0.93
Hourly flow rate (vph)	1	8
Pedestrians		
Lane Width (ft)		
Walking Speed (ft/s)		
Percent Blockage		
Right turn flare (veh)		
Median type		
Median storage (veh)		
Upstream signal (ft)		
pX, platoon unblocked		
vC, conflicting volume	3674	694
vC1, stage 1 conf vol		
vC2, stage 2 conf vol		
vCu, unblocked vol	3674	694
tC, single (s)	6.5	6.9
tC, 2 stage (s)		
tF (s)	4.0	3.3
p0 queue free %	78	98
cM capacity (veh/h)	4	385
Direction, Lane #		

HCM Unsignalized Intersection Capacity Analysis  
67: Enterprise Road & SR 580

06/24/2021



Movement	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	
Lane Configurations	↶	↶↶↶				↶↶↶			↶↶			↶↶	
Traffic Volume (veh/h)	0	2581	136	3	48	2074	0	63	0	45	0	0	
Future Volume (Veh/h)	0	2581	136	3	48	2074	0	63	0	45	0	0	
Sign Control		Free				Free			Stop			Stop	
Grade		0%				0%			0%			0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	
Hourly flow rate (vph)	0	2775	146	0	52	2230	0	68	0	48	0	0	
Pedestrians													
Lane Width (ft)													
Walking Speed (ft/s)													
Percent Blockage													
Right turn flare (veh)													
Median type	TWLTL					None							
Median storage veh	2												
Upstream signal (ft)						926							
pX, platoon unblocked	0.76			0.00				0.76	0.76		0.76	0.76	
vC, conflicting volume	2230			0	2921			3696	5182	998	3307	5255	
vC1, stage 1 conf vol								2848	2848		2334	2334	
vC2, stage 2 conf vol								848	2334		973	2921	
vCu, unblocked vol	1527			0	2921			3447	5393	998	2937	5489	
tC, single (s)	4.1			0.0	4.1			7.5	6.5	6.9	7.5	6.5	
tC, 2 stage (s)								6.5	5.5		6.5	5.5	
tF (s)	2.2			0.0	2.2			3.5	4.0	3.3	3.5	4.0	
p0 queue free %	100			0	57			0	100	80	100	100	
cM capacity (veh/h)	330			0	122			17	30	242	41	0	
Direction, Lane #	EB 1	EB 2	EB 3	EB 4	WB 1	WB 2	WB 3	NB 1	SB 1				
Volume Total	0	1110	1110	701	610	1115	558	116	1				
Volume Left	0	0	0	0	52	0	0	68	0				
Volume Right	0	0	0	146	0	0	0	48	1				
cSH	1700	1700	1700	1700	122	1700	1700	27	828				
Volume to Capacity	0.00	0.65	0.65	0.41	0.43	0.66	0.33	4.26	0.00				
Queue Length 95th (ft)	0	0	0	0	46	0	0	Err	0				
Control Delay (s)	0.0	0.0	0.0	0.0	36.7	0.0	0.0	Err	9.4				
Lane LOS					E			F	A				
Approach Delay (s)	0.0				9.8			Err	9.4				
Approach LOS								F	A				
Intersection Summary													
Average Delay		222.2											
Intersection Capacity Utilization		96.2%		ICU Level of Service					F				
Analysis Period (min)		15											



HCM Unsignalized Intersection Capacity Analysis  
 67: Enterprise Road & SR 580

06/24/2021



Movement	SBR
Lane Configurations	
Traffic Volume (veh/h)	1
Future Volume (Veh/h)	1
Sign Control	
Grade	
Peak Hour Factor	0.93
Hourly flow rate (vph)	1
Pedestrians	
Lane Width (ft)	
Walking Speed (ft/s)	
Percent Blockage	
Right turn flare (veh)	
Median type	
Median storage (veh)	
Upstream signal (ft)	
pX, platoon unblocked	0.76
vC, conflicting volume	743
vC1, stage 1 conf vol	
vC2, stage 2 conf vol	
vCu, unblocked vol	0
tC, single (s)	6.9
tC, 2 stage (s)	
tF (s)	3.3
p0 queue free %	100
cM capacity (veh/h)	828
Direction, Lane #	

# HCM Unsignalized Intersection Capacity Analysis

## 80: State Street & SR 580

06/24/2021



Movement	EBU	EBT	EBR	WBL	WBT	NWL	NWR
Lane Configurations	⇐	⇑		⇑	⇑	⇑	
Traffic Volume (veh/h)	3	763	74	0	1043	66	2
Future Volume (Veh/h)	3	763	74	0	1043	66	2
Sign Control		Free			Free	Stop	
Grade		0%			0%	0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	0	820	80	0	1122	71	2
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type		None			None		
Median storage (veh)							
Upstream signal (ft)		1225			915		
pX, platoon unblocked	0.00			0.92		0.92	0.92
vC, conflicting volume	0			900		1421	450
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	0			727		1291	240
tC, single (s)	0.0			4.1		6.8	6.9
tC, 2 stage (s)							
tF (s)	0.0			2.2		3.5	3.3
p0 queue free %	0			100		50	100
cM capacity (veh/h)	0			806		143	703
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	WB 3	NW 1
Volume Total	547	353	0	0	561	561	73
Volume Left	0	0	0	0	0	0	71
Volume Right	0	80	0	0	0	0	2
cSH	1700	1700	1700	1700	1700	1700	146
Volume to Capacity	0.32	0.21	0.00	0.00	0.33	0.33	0.50
Queue Length 95th (ft)	0	0	0	0	0	0	59
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	51.9
Lane LOS							F
Approach Delay (s)	0.0			0.0			51.9
Approach LOS							F
Intersection Summary							
Average Delay			1.8				
Intersection Capacity Utilization			39.3%		ICU Level of Service		A
Analysis Period (min)			15				

## Arterial Level of Service

06/28/2021

### Arterial Level of Service: EB SR 580

Cross Street	Arterial Class	Flow Speed	Running Time	Signal Delay	Travel Time (s)	Dist (mi)	Arterial Speed	Arterial LOS
Main St	II	39	44.7	20.2	64.9	0.48	26.6	C
Patricia Ave	II	40	40.8	24.0	64.8	0.42	23.6	C
Crosley Dr	II	40	27.8	6.7	34.5	0.25	26.4	C
Lake Haven Rd	II	40	15.2	2.9	18.1	0.13	26.3	C
Keene Rd	II	45	35.4	59.5	94.9	0.37	14.0	E
Achieva Way	II	45	27.9	10.9	38.8	0.27	24.9	C
Overcash Dr	II	45	20.0	7.7	27.7	0.18	23.9	C
Pinewood Dr	II	45	30.8	10.4	41.2	0.31	27.2	C
Belcher Rd	II	45	26.4	38.3	64.7	0.25	14.1	E
US 19 Frontage	II	45	42.4	218.7	261.1	0.53	7.3	F
Summerdale Dr	II	45	26.8	1.2	28.0	0.26	33.2	B
Countryside Blvd	II	45	43.6	81.8	125.4	0.55	15.7	E
Landmark Dr	II	45	45.8	17.8	63.6	0.52	29.5	B
Charles Ave	II	45	32.5	3.3	35.8	0.34	34.0	B
McMullen Booth Rd	II	45	23.7	49.6	73.3	0.22	10.7	F
SR 590	II	45	75.7	70.0	145.7	0.95	23.4	C
St Petersburg Dr	II	45	97.2	27.8	125.0	1.22	35.0	B
S Bayview Blvd	II	45	58.7	8.0	66.7	0.73	39.6	A
Tampa Rd	II	45	38.1	4.5	42.6	0.41	34.3	B
Total	II		753.5	663.3	1416.8	8.39	21.3	D

### Arterial Level of Service: WB SR 580

Cross Street	Arterial Class	Flow Speed	Running Time	Signal Delay	Travel Time (s)	Dist (mi)	Arterial Speed	Arterial LOS
S Bayview Blvd	II	45	38.1	1.4	39.5	0.41	36.9	A
Forest Lakes Blvd	II	45	58.7	118.6	177.3	0.73	14.9	E
2nd St	II	45	97.2	15.1	112.3	1.22	39.0	A
McMullen Booth Rd	II	45	75.7	86.4	162.1	0.95	21.0	D
Charles Ave	II	45	23.7	7.2	30.9	0.22	25.4	C
Landmark Dr	II	45	32.5	16.1	48.6	0.34	25.0	C
Countryside Blvd	II	45	45.8	30.4	76.2	0.52	24.6	C
Summerdale Dr	II	45	43.6	17.1	60.7	0.55	32.3	B
US 19 Frontage	II	45	26.8	56.2	83.0	0.26	11.2	F
Belcher Rd	II	45	42.4	38.0	80.4	0.53	23.8	C
King Arthur Ct	II	45	26.4	10.6	37.0	0.25	24.7	C
Overcash Dr	II	45	30.8	7.0	37.8	0.31	29.6	B
Sunlight Dr	II	45	20.0	16.6	36.6	0.18	18.1	D
CR 1	II	45	27.9	42.7	70.6	0.27	13.7	E
Lake Haven Rd	II	45	35.4	0.5	35.9	0.37	37.0	A
Pinehurst Rd	II	40	15.2	5.7	20.9	0.13	22.8	C
Patricia Ave	II	40	27.8	11.1	38.9	0.25	23.4	C
Bass Blvd	II	40	40.8	7.1	47.9	0.42	31.9	B
Alt 19	II	39	44.7	0.0	44.7	0.48	38.7	A
Total	II		753.5	487.8	1241.3	8.39	24.3	C

# HCM Signalized Intersection Capacity Analysis

## 3: Alt 19 & SR 580

06/28/2021



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBL	SBT
Lane Configurations		↕			↕	↕			↕	↕	↕	↕
Traffic Volume (vph)	0	3	0	158	4	308	1	3	718	190	283	487
Future Volume (vph)	0	3	0	158	4	308	1	3	718	190	283	487
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		7.3			6.7	6.7			6.7	6.7	6.5	6.7
Lane Util. Factor		1.00			1.00	1.00			0.95	1.00	1.00	1.00
Frt		1.00			1.00	0.85			1.00	0.85	1.00	1.00
Flt Protected		1.00			0.95	1.00			1.00	1.00	0.95	1.00
Satd. Flow (prot)		1863			1776	1583			3538	1583	1770	1858
Flt Permitted		1.00			0.73	1.00			0.95	1.00	0.20	1.00
Satd. Flow (perm)		1863			1359	1583			3369	1583	381	1858
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	0	3	0	168	4	328	1	3	764	202	301	518
RTOR Reduction (vph)	0	0	0	0	0	261	0	0	0	134	0	1
Lane Group Flow (vph)	0	3	0	0	172	67	0	0	768	68	301	526
Turn Type		NA		Perm	NA	Perm	Perm	Perm	NA	Perm	pm+pt	NA
Protected Phases		8			4				6			5
Permitted Phases	8			4		4	6	6		6		2
Actuated Green, G (s)		12.6			13.2	13.2			21.8	21.8	38.2	38.2
Effective Green, g (s)		12.6			13.2	13.2			21.8	21.8	38.2	38.2
Actuated g/C Ratio		0.19			0.20	0.20			0.34	0.34	0.59	0.59
Clearance Time (s)		7.3			6.7	6.7			6.7	6.7	6.5	6.7
Vehicle Extension (s)		3.0			3.0	3.0			3.0	3.0	3.0	3.0
Lane Grp Cap (vph)		362			276	322			1133	532	436	1095
v/s Ratio Prot		0.00									c0.11	0.28
v/s Ratio Perm					c0.13	0.04			0.23	0.04	c0.30	
v/c Ratio		0.01			0.62	0.21			0.68	0.13	0.69	0.48
Uniform Delay, d1		21.1			23.5	21.5			18.5	14.9	8.7	7.6
Progression Factor		1.00			1.00	1.00			1.00	1.00	1.00	1.00
Incremental Delay, d2		0.0			4.3	0.3			3.3	0.5	4.7	1.5
Delay (s)		21.1			27.9	21.8			21.8	15.4	13.3	9.1
Level of Service		C			C	C			C	B	B	A
Approach Delay (s)		21.1			23.9				20.4			10.7
Approach LOS		C			C				C			B

### Intersection Summary

HCM 2000 Control Delay	17.7	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.73		
Actuated Cycle Length (s)	64.8	Sum of lost time (s)	20.5
Intersection Capacity Utilization	78.5%	ICU Level of Service	D
Analysis Period (min)	15		

c Critical Lane Group

# HCM Signalized Intersection Capacity Analysis

## 3: Alt 19 & SR 580

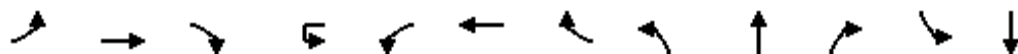
06/28/2021

Movement	SBR
Lane Configurations	
Traffic Volume (vph)	8
Future Volume (vph)	8
Ideal Flow (vphpl)	1900
Total Lost time (s)	
Lane Util. Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Peak-hour factor, PHF	0.94
Adj. Flow (vph)	9
RTOR Reduction (vph)	0
Lane Group Flow (vph)	0
Turn Type	
Protected Phases	
Permitted Phases	
Actuated Green, G (s)	
Effective Green, g (s)	
Actuated g/C Ratio	
Clearance Time (s)	
Vehicle Extension (s)	
Lane Grp Cap (vph)	
v/s Ratio Prot	
v/s Ratio Perm	
v/c Ratio	
Uniform Delay, d1	
Progression Factor	
Incremental Delay, d2	
Delay (s)	
Level of Service	
Approach Delay (s)	
Approach LOS	
Intersection Summary	

# HCM Signalized Intersection Capacity Analysis

## 4: Main St/Bass Blvd & SR 580

06/28/2021



Movement	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations	↖	↗			↖	↗			↖	↗	↖	↗
Traffic Volume (vph)	2	712	18	1	145	577	65	12	15	169	30	11
Future Volume (vph)	2	712	18	1	145	577	65	12	15	169	30	11
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	7.1	7.1			7.1	7.1			9.1	9.1	7.0	7.0
Lane Util. Factor	1.00	0.95			1.00	0.95			1.00	1.00	1.00	1.00
Frt	1.00	1.00			1.00	0.98			1.00	0.85	1.00	0.95
Flt Protected	0.95	1.00			0.95	1.00			0.98	1.00	0.95	1.00
Satd. Flow (prot)	1593	3174			1593	3137			1640	1425	1593	1593
Flt Permitted	0.95	1.00			0.95	1.00			0.98	1.00	0.95	1.00
Satd. Flow (perm)	1593	3174			1593	3137			1640	1425	1593	1593
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	2	757	19	1	154	614	69	13	16	180	32	12
RTOR Reduction (vph)	0	1	0	0	0	5	0	0	0	171	0	6
Lane Group Flow (vph)	2	775	0	0	155	678	0	0	29	9	32	12
Turn Type	Prot	NA		Prot	Prot	NA		Split	NA	Perm	Split	NA
Protected Phases	1	6		5	5	2		4	4		8	8
Permitted Phases										4		
Actuated Green, G (s)	1.2	63.2			15.1	77.1			6.1	6.1	5.3	5.3
Effective Green, g (s)	1.2	63.2			15.1	77.1			6.1	6.1	5.3	5.3
Actuated g/C Ratio	0.01	0.53			0.13	0.64			0.05	0.05	0.04	0.04
Clearance Time (s)	7.1	7.1			7.1	7.1			9.1	9.1	7.0	7.0
Vehicle Extension (s)	1.0	1.0			1.0	1.0			1.0	1.0	1.0	1.0
Lane Grp Cap (vph)	15	1671			200	2015			83	72	70	70
v/s Ratio Prot	0.00	c0.24			c0.10	0.22			c0.02		c0.02	0.01
v/s Ratio Perm										0.01		
v/c Ratio	0.13	0.46			0.78	0.34			0.35	0.13	0.46	0.18
Uniform Delay, d1	58.9	17.8			50.8	9.8			55.0	54.4	55.9	55.2
Progression Factor	1.00	1.00			0.82	1.20			1.00	1.00	1.00	1.00
Incremental Delay, d2	1.5	0.9			14.6	0.4			0.9	0.3	1.7	0.4
Delay (s)	60.4	18.7			56.4	12.2			56.0	54.7	57.7	55.7
Level of Service	E	B			E	B			E	D	E	E
Approach Delay (s)		18.8				20.4			54.9			57.0
Approach LOS		B				C			D			E

### Intersection Summary

HCM 2000 Control Delay	24.5	HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio	0.51		
Actuated Cycle Length (s)	120.0	Sum of lost time (s)	30.3
Intersection Capacity Utilization	72.5%	ICU Level of Service	C
Analysis Period (min)	15		

c Critical Lane Group

# HCM Signalized Intersection Capacity Analysis

## 4: Main St/Bass Blvd & SR 580

06/28/2021

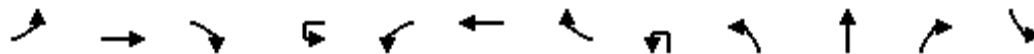


Movement	SBR
Lane Configurations	
Traffic Volume (vph)	6
Future Volume (vph)	6
Ideal Flow (vphpl)	1900
Total Lost time (s)	
Lane Util. Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Peak-hour factor, PHF	0.94
Adj. Flow (vph)	6
RTOR Reduction (vph)	0
Lane Group Flow (vph)	0
Turn Type	
Protected Phases	
Permitted Phases	
Actuated Green, G (s)	
Effective Green, g (s)	
Actuated g/C Ratio	
Clearance Time (s)	
Vehicle Extension (s)	
Lane Grp Cap (vph)	
v/s Ratio Prot	
v/s Ratio Perm	
v/c Ratio	
Uniform Delay, d1	
Progression Factor	
Incremental Delay, d2	
Delay (s)	
Level of Service	
Approach Delay (s)	
Approach LOS	
Intersection Summary	

# HCM Signalized Intersection Capacity Analysis

## 13: Patricia Ave & SR 580

06/28/2021



Movement	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBL
Lane Configurations												
Traffic Volume (vph)	1	905	86	10	310	907	8	2	116	2	325	5
Future Volume (vph)	1	905	86	10	310	907	8	2	116	2	325	5
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	6.9	6.9	6.9		6.9	6.9				6.7	6.7	
Lane Util. Factor	1.00	0.95	1.00		0.97	0.95				1.00	0.88	
Frt	1.00	1.00	0.85		1.00	1.00				1.00	0.85	
Flt Protected	0.95	1.00	1.00		0.95	1.00				0.95	1.00	
Satd. Flow (prot)	1770	3539	1583		3433	3534				1775	2787	
Flt Permitted	0.95	1.00	1.00		0.95	1.00				0.72	1.00	
Satd. Flow (perm)	1770	3539	1583		3433	3534				1340	2787	
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	1	963	91	11	330	965	9	2	123	2	346	5
RTOR Reduction (vph)	0	0	62	0	0	0	0	0	0	0	303	0
Lane Group Flow (vph)	1	963	29	0	341	974	0	0	0	127	43	0
Turn Type	Prot	NA	Perm	Prot	Prot	NA		Perm	Perm	NA	Perm	Perm
Protected Phases	1	6		5	5	2				4		
Permitted Phases			6					4	4		4	3 8
Actuated Green, G (s)	1.0	37.6	37.6		38.1	74.7				14.8	14.8	
Effective Green, g (s)	1.0	37.6	37.6		38.1	74.7				14.8	14.8	
Actuated g/C Ratio	0.01	0.31	0.31		0.32	0.62				0.12	0.12	
Clearance Time (s)	6.9	6.9	6.9		6.9	6.9				6.7	6.7	
Vehicle Extension (s)	1.0	1.0	1.0		1.0	1.0				1.0	1.0	
Lane Grp Cap (vph)	14	1108	496		1089	2199				165	343	
v/s Ratio Prot	0.00	c0.27			0.10	c0.28						
v/s Ratio Perm			0.02							c0.09	0.02	
v/c Ratio	0.07	0.87	0.06		0.31	0.44				0.77	0.12	
Uniform Delay, d1	59.0	38.9	28.8		31.0	11.8				50.9	46.8	
Progression Factor	1.51	0.64	1.00		0.83	0.48				1.00	1.00	
Incremental Delay, d2	0.7	6.7	0.0		0.1	0.6				17.5	0.1	
Delay (s)	89.9	31.7	28.8		25.7	6.3				68.4	46.9	
Level of Service	F	C	C		C	A				E	D	
Approach Delay (s)		31.5				11.4				52.7		
Approach LOS		C				B				D		

### Intersection Summary

HCM 2000 Control Delay	25.8	HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio	0.67		
Actuated Cycle Length (s)	120.0	Sum of lost time (s)	27.5
Intersection Capacity Utilization	72.6%	ICU Level of Service	C
Analysis Period (min)	15		

c Critical Lane Group



# HCM Signalized Intersection Capacity Analysis

## 13: Patricia Ave & SR 580

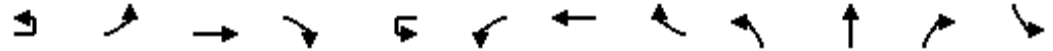
06/28/2021



Movement	SBT	SBR
Lane Configurations	↕	
Traffic Volume (vph)	5	4
Future Volume (vph)	5	4
Ideal Flow (vphpl)	1900	1900
Total Lost time (s)	7.0	
Lane Util. Factor	1.00	
Frt	0.96	
Flt Protected	0.98	
Satd. Flow (prot)	1759	
Flt Permitted	0.91	
Satd. Flow (perm)	1626	
Peak-hour factor, PHF	0.94	0.94
Adj. Flow (vph)	5	4
RTOR Reduction (vph)	3	0
Lane Group Flow (vph)	11	0
Turn Type	NA	
Protected Phases	3 8	
Permitted Phases		
Actuated Green, G (s)	23.5	
Effective Green, g (s)	23.5	
Actuated g/C Ratio	0.20	
Clearance Time (s)		
Vehicle Extension (s)		
Lane Grp Cap (vph)	318	
v/s Ratio Prot		
v/s Ratio Perm	c0.01	
v/c Ratio	0.03	
Uniform Delay, d1	39.1	
Progression Factor	1.00	
Incremental Delay, d2	0.0	
Delay (s)	39.1	
Level of Service	D	
Approach Delay (s)	39.1	
Approach LOS	D	
<b>Intersection Summary</b>		

HCM Signalized Intersection Capacity Analysis  
 18: Crosley Dr/Pinehurst Rd & SR 580

06/28/2021



Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL
Lane Configurations												
Traffic Volume (vph)	7	205	1336	6	3	32	1021	304	2	10	8	201
Future Volume (vph)	7	205	1336	6	3	32	1021	304	2	10	8	201
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		7.2	7.2			6.8	7.2	7.2		7.4	7.4	7.5
Lane Util. Factor		1.00	0.95			1.00	0.95	1.00		1.00	1.00	0.95
Frt		1.00	1.00			1.00	1.00	0.85		1.00	0.85	1.00
Flt Protected		0.95	1.00			0.95	1.00	1.00		0.99	1.00	0.95
Satd. Flow (prot)		1770	3537			1770	3539	1583		1849	1583	1681
Flt Permitted		0.22	1.00			0.16	1.00	1.00		0.99	1.00	0.95
Satd. Flow (perm)		412	3537			297	3539	1583		1849	1583	1681
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	7	218	1421	6	3	34	1086	323	2	11	9	214
RTOR Reduction (vph)	0	0	0	0	0	0	0	51	0	0	9	0
Lane Group Flow (vph)	0	225	1427	0	0	37	1086	272	0	13	0	109
Turn Type	pm+pt	pm+pt	NA		pm+pt	pm+pt	NA	Perm	Split	NA	Perm	Split
Protected Phases	1	1	6		5	5	2		4	4		8
Permitted Phases	6	6			2	2		2				4
Actuated Green, G (s)		195.0	184.2			181.2	177.1	177.1		3.7	3.7	19.1
Effective Green, g (s)		195.0	184.2			181.2	177.1	177.1		3.7	3.7	19.1
Actuated g/C Ratio		0.81	0.77			0.75	0.74	0.74		0.02	0.02	0.08
Clearance Time (s)		7.2	7.2			6.8	7.2	7.2		7.4	7.4	7.5
Vehicle Extension (s)		1.0	1.0			1.0	1.0	1.0		1.0	1.0	1.0
Lane Grp Cap (vph)		395	2714			249	2611	1168		28	24	133
v/s Ratio Prot		c0.03	c0.40			0.00	0.31			c0.01		0.06
v/s Ratio Perm		c0.44				0.11		0.17				0.00
v/c Ratio		0.57	0.53			0.15	0.42	0.23		0.46	0.01	0.82
Uniform Delay, d1		8.1	10.9			8.9	11.9	10.0		117.2	116.3	108.8
Progression Factor		1.62	0.44			0.72	0.53	0.26		1.00	1.00	1.00
Incremental Delay, d2		1.0	0.1			0.1	0.5	0.5		4.4	0.0	29.6
Delay (s)		14.2	4.8			6.5	6.7	3.0		121.5	116.4	138.4
Level of Service		B	A			A	A	A		F	F	F
Approach Delay (s)			6.1				5.9			119.4		
Approach LOS			A				A			F		

Intersection Summary

HCM 2000 Control Delay	17.9	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.60		
Actuated Cycle Length (s)	240.0	Sum of lost time (s)	29.3
Intersection Capacity Utilization	75.2%	ICU Level of Service	D
Analysis Period (min)	15		

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis  
 18: Crosley Dr/Pinehurst Rd & SR 580

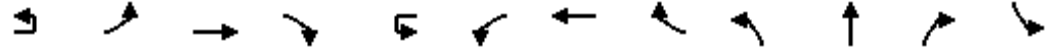
06/28/2021



Movement	SBT	SBR
Lane Configurations	↕	↗
Traffic Volume (vph)	5	90
Future Volume (vph)	5	90
Ideal Flow (vphpl)	1900	1900
Total Lost time (s)	7.5	7.5
Lane Util. Factor	0.95	1.00
Frt	1.00	0.85
Flt Protected	0.95	1.00
Satd. Flow (prot)	1689	1583
Flt Permitted	0.95	1.00
Satd. Flow (perm)	1689	1583
Peak-hour factor, PHF	0.94	0.94
Adj. Flow (vph)	5	96
RTOR Reduction (vph)	0	88
Lane Group Flow (vph)	110	8
Turn Type	NA	Perm
Protected Phases	8	
Permitted Phases		8
Actuated Green, G (s)	19.1	19.1
Effective Green, g (s)	19.1	19.1
Actuated g/C Ratio	0.08	0.08
Clearance Time (s)	7.5	7.5
Vehicle Extension (s)	1.0	1.0
Lane Grp Cap (vph)	134	125
v/s Ratio Prot	c0.07	
v/s Ratio Perm		0.00
v/c Ratio	0.82	0.06
Uniform Delay, d1	108.8	102.2
Progression Factor	1.00	1.00
Incremental Delay, d2	30.3	0.1
Delay (s)	139.0	102.2
Level of Service	F	F
Approach Delay (s)	127.6	
Approach LOS	F	
Intersection Summary		

HCM Signalized Intersection Capacity Analysis  
 20: Lake Haven Rd & SR 580

06/28/2021



Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL
Lane Configurations		↖	↑↑↑	↗		↖	↑↑↑			↖	↗	↖
Traffic Volume (vph)	8	8	1648	45	7	96	1230	31	55	10	101	17
Future Volume (vph)	8	8	1648	45	7	96	1230	31	55	10	101	17
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		6.8	6.8	6.8		6.8	6.8			8.0	8.0	8.0
Lane Util. Factor		1.00	0.91	1.00		1.00	0.91			1.00	1.00	1.00
Frt		1.00	1.00	0.85		1.00	1.00			1.00	0.85	1.00
Flt Protected		0.95	1.00	1.00		0.95	1.00			0.96	1.00	0.95
Satd. Flow (prot)		1770	5085	1583		1770	5067			1787	1583	1770
Flt Permitted		0.19	1.00	1.00		0.12	1.00			0.75	1.00	0.66
Satd. Flow (perm)		349	5085	1583		220	5067			1399	1583	1232
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	9	9	1753	48	7	102	1309	33	59	11	107	18
RTOR Reduction (vph)	0	0	0	6	0	0	1	0	0	0	60	0
Lane Group Flow (vph)	0	18	1753	42	0	109	1341	0	0	70	47	18
Turn Type	Perm	Perm	NA	Perm	Perm	Perm	NA		Perm	NA	Perm	Perm
Protected Phases			6				2			4		
Permitted Phases	6	6		6	2	2			4		4	8
Actuated Green, G (s)		209.8	209.8	209.8		209.8	209.8			15.4	15.4	15.4
Effective Green, g (s)		209.8	209.8	209.8		209.8	209.8			15.4	15.4	15.4
Actuated g/C Ratio		0.87	0.87	0.87		0.87	0.87			0.06	0.06	0.06
Clearance Time (s)		6.8	6.8	6.8		6.8	6.8			8.0	8.0	8.0
Vehicle Extension (s)		1.0	1.0	1.0		1.0	1.0			1.0	1.0	1.0
Lane Grp Cap (vph)		305	4445	1383		192	4429			89	101	79
v/s Ratio Prot			0.34				0.26					
v/s Ratio Perm		0.05		0.03		c0.50				c0.05	0.03	0.01
v/c Ratio		0.06	0.39	0.03		0.57	0.30			0.79	0.47	0.23
Uniform Delay, d1		2.0	2.9	2.0		3.8	2.6			110.7	108.3	106.7
Progression Factor		0.52	1.01	0.64		3.64	0.10			1.00	1.00	1.00
Incremental Delay, d2		0.0	0.0	0.0		9.0	0.1			33.2	1.2	0.5
Delay (s)		1.1	2.9	1.3		22.7	0.4			143.9	109.6	107.2
Level of Service		A	A	A		C	A			F	F	F
Approach Delay (s)			2.9				2.1			123.2		
Approach LOS			A				A			F		

Intersection Summary

HCM 2000 Control Delay	9.6	HCM 2000 Level of Service	A
HCM 2000 Volume to Capacity ratio	0.58		
Actuated Cycle Length (s)	240.0	Sum of lost time (s)	14.8
Intersection Capacity Utilization	75.3%	ICU Level of Service	D
Analysis Period (min)	15		

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis  
 20: Lake Haven Rd & SR 580

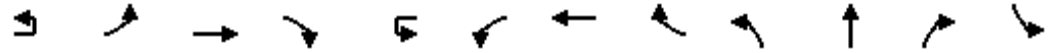
06/28/2021



Movement	SBT	SBR
Lane Configurations	⬆	➡
Traffic Volume (vph)	9	2
Future Volume (vph)	9	2
Ideal Flow (vphpl)	1900	1900
Total Lost time (s)	8.0	
Lane Util. Factor	1.00	
Frt	0.97	
Flt Protected	1.00	
Satd. Flow (prot)	1816	
Flt Permitted	1.00	
Satd. Flow (perm)	1816	
Peak-hour factor, PHF	0.94	0.94
Adj. Flow (vph)	10	2
RTOR Reduction (vph)	2	0
Lane Group Flow (vph)	10	0
Turn Type	NA	
Protected Phases	8	
Permitted Phases		
Actuated Green, G (s)	15.4	
Effective Green, g (s)	15.4	
Actuated g/C Ratio	0.06	
Clearance Time (s)	8.0	
Vehicle Extension (s)	1.0	
Lane Grp Cap (vph)	116	
v/s Ratio Prot	0.01	
v/s Ratio Perm		
v/c Ratio	0.09	
Uniform Delay, d1	105.7	
Progression Factor	1.00	
Incremental Delay, d2	0.1	
Delay (s)	105.8	
Level of Service	F	
Approach Delay (s)	106.6	
Approach LOS	F	
Intersection Summary		

HCM Signalized Intersection Capacity Analysis  
 23: Keene Rd/CR 1 & SR 580

06/28/2021



Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL
Lane Configurations		↔↔	↔↔↔			↔↔	↔↔↔	↔	↔	↔↔	↔	↔
Traffic Volume (vph)	7	249	1346	113	1	220	1164	484	225	833	301	411
Future Volume (vph)	7	249	1346	113	1	220	1164	484	225	833	301	411
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		7.3	7.3			7.3	7.3	7.3	7.1	7.1	7.1	7.4
Lane Util. Factor		0.97	0.91			0.97	0.91	1.00	1.00	0.95	1.00	0.91
Frt		1.00	0.99			1.00	1.00	0.85	1.00	1.00	0.85	1.00
Flt Protected		0.95	1.00			0.95	1.00	1.00	0.95	1.00	1.00	0.95
Satd. Flow (prot)		3433	5026			3433	5085	1583	1770	3539	1583	1610
Flt Permitted		0.95	1.00			0.95	1.00	1.00	0.95	1.00	1.00	0.95
Satd. Flow (perm)		3433	5026			3433	5085	1583	1770	3539	1583	1610
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	7	265	1432	120	1	234	1238	515	239	886	320	437
RTOR Reduction (vph)	0	0	4	0	0	0	0	95	0	0	89	0
Lane Group Flow (vph)	0	272	1548	0	0	235	1238	420	239	886	231	280
Turn Type	Prot	Prot	NA		Prot	Prot	NA	Perm	Split	NA	Perm	Split
Protected Phases	1	1	6		5	5	2		4	4		8
Permitted Phases							2				4	
Actuated Green, G (s)		21.1	83.4			19.8	82.1	82.1	62.3	62.3	62.3	45.4
Effective Green, g (s)		21.1	83.4			19.8	82.1	82.1	62.3	62.3	62.3	45.4
Actuated g/C Ratio		0.09	0.35			0.08	0.34	0.34	0.26	0.26	0.26	0.19
Clearance Time (s)		7.3	7.3			7.3	7.3	7.3	7.1	7.1	7.1	7.4
Vehicle Extension (s)		1.0	1.0			1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lane Grp Cap (vph)		301	1746			283	1739	541	459	918	410	304
v/s Ratio Prot		0.08	c0.31			0.07	0.24		0.14	c0.25		c0.17
v/s Ratio Perm								c0.27			0.15	
v/c Ratio		0.90	0.89			0.83	0.71	0.78	0.52	0.97	0.56	0.92
Uniform Delay, d1		108.4	73.8			108.4	68.7	70.7	76.1	87.8	77.1	95.5
Progression Factor		1.11	0.73			0.78	0.71	0.70	1.00	1.00	1.00	1.00
Incremental Delay, d2		26.7	6.7			15.1	2.1	8.9	0.5	21.3	1.1	31.4
Delay (s)		146.8	60.8			100.0	50.7	58.4	76.6	109.1	78.1	126.9
Level of Service		F	E			F	D	E	E	F	E	F
Approach Delay (s)			73.6				58.5			96.9		
Approach LOS			E				E			F		

Intersection Summary		
HCM 2000 Control Delay	80.3	HCM 2000 Level of Service
HCM 2000 Volume to Capacity ratio	0.92	F
Actuated Cycle Length (s)	240.0	Sum of lost time (s)
Intersection Capacity Utilization	97.3%	29.1
Analysis Period (min)	15	ICU Level of Service
		F

c Critical Lane Group

# HCM Signalized Intersection Capacity Analysis

## 23: Keene Rd/CR 1 & SR 580

06/28/2021

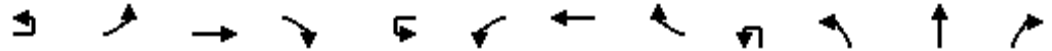


Movement	SBT	SBR
Lane Configurations	↔↑↑	↔↑
Traffic Volume (vph)	394	124
Future Volume (vph)	394	124
Ideal Flow (vphpl)	1900	1900
Total Lost time (s)	7.4	7.4
Lane Util. Factor	0.91	1.00
Frt	1.00	0.85
Flt Protected	0.99	1.00
Satd. Flow (prot)	3345	1583
Flt Permitted	0.99	1.00
Satd. Flow (perm)	3345	1583
Peak-hour factor, PHF	0.94	0.94
Adj. Flow (vph)	419	132
RTOR Reduction (vph)	0	107
Lane Group Flow (vph)	576	25
Turn Type	NA	Perm
Protected Phases	8	
Permitted Phases		8
Actuated Green, G (s)	45.4	45.4
Effective Green, g (s)	45.4	45.4
Actuated g/C Ratio	0.19	0.19
Clearance Time (s)	7.4	7.4
Vehicle Extension (s)	1.0	1.0
Lane Grp Cap (vph)	632	299
v/s Ratio Prot	0.17	
v/s Ratio Perm		0.02
v/c Ratio	0.91	0.08
Uniform Delay, d1	95.3	80.2
Progression Factor	1.00	1.00
Incremental Delay, d2	17.2	0.0
Delay (s)	112.5	80.2
Level of Service	F	F
Approach Delay (s)	112.3	
Approach LOS	F	
Intersection Summary		

# HCM Signalized Intersection Capacity Analysis

## 26: Achieva Way/Sunlight Dr & SR 580

06/28/2021



Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR
Lane Configurations												
Traffic Volume (vph)	3	42	2193	113	2	217	1830	51	6	82	36	358
Future Volume (vph)	3	42	2193	113	2	217	1830	51	6	82	36	358
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		7.2	7.2			7.2	7.2			7.6	7.6	7.6
Lane Util. Factor		1.00	0.91			1.00	0.91			1.00	1.00	1.00
Frt		1.00	0.99			1.00	1.00			1.00	1.00	0.85
Flt Protected		0.95	1.00			0.95	1.00			0.95	1.00	1.00
Satd. Flow (prot)		1770	5048			1770	5065			1770	1863	1583
Flt Permitted		0.09	1.00			0.03	1.00			0.26	1.00	1.00
Satd. Flow (perm)		176	5048			51	5065			489	1863	1583
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	3	45	2333	120	2	231	1947	54	6	87	38	381
RTOR Reduction (vph)	0	0	2	0	0	0	1	0	0	0	0	232
Lane Group Flow (vph)	0	48	2451	0	0	233	2000	0	0	93	38	149
Turn Type	pm+pt	pm+pt	NA		pm+pt	pm+pt	NA		Perm	Perm	NA	Perm
Protected Phases	1	1	6		5	5	2				4	
Permitted Phases	6	6			2	2			4	4		4
Actuated Green, G (s)		153.4	153.4			175.5	175.5			34.7	34.7	34.7
Effective Green, g (s)		153.4	153.4			175.5	175.5			34.7	34.7	34.7
Actuated g/C Ratio		0.64	0.64			0.73	0.73			0.14	0.14	0.14
Clearance Time (s)		7.2	7.2			7.2	7.2			7.6	7.6	7.6
Vehicle Extension (s)		1.0	1.0			1.0	1.0			1.0	1.0	1.0
Lane Grp Cap (vph)		164	3226			251	3703			70	269	228
v/s Ratio Prot		0.01	c0.49			c0.12	0.39				0.02	
v/s Ratio Perm		0.18				c0.56				c0.19		0.09
v/c Ratio		0.29	0.76			0.93	0.54			1.33	0.14	0.65
Uniform Delay, d1		21.5	30.4			89.4	14.3			102.7	89.6	97.0
Progression Factor		0.29	0.28			0.86	0.55			1.00	1.00	1.00
Incremental Delay, d2		0.2	1.2			34.1	0.5			218.3	0.1	5.1
Delay (s)		6.6	9.7			110.7	8.4			320.9	89.7	102.0
Level of Service		A	A			F	A			F	F	F
Approach Delay (s)			9.7				19.1				140.9	
Approach LOS			A				B				F	

### Intersection Summary

HCM 2000 Control Delay	31.6	HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio	0.97		
Actuated Cycle Length (s)	240.0	Sum of lost time (s)	22.0
Intersection Capacity Utilization	110.5%	ICU Level of Service	H
Analysis Period (min)	15		

c Critical Lane Group



# HCM Signalized Intersection Capacity Analysis

## 26: Achieva Way/Sunlight Dr & SR 580

06/28/2021



Movement	SBL	SBT	SBR
Lane Configurations	↶	↷	
Traffic Volume (vph)	120	104	97
Future Volume (vph)	120	104	97
Ideal Flow (vphpl)	1900	1900	1900
Total Lost time (s)	7.6	7.6	
Lane Util. Factor	1.00	1.00	
Frt	1.00	0.93	
Flt Protected	0.95	1.00	
Satd. Flow (prot)	1770	1728	
Flt Permitted	0.73	1.00	
Satd. Flow (perm)	1364	1728	
Peak-hour factor, PHF	0.94	0.94	0.94
Adj. Flow (vph)	128	111	103
RTOR Reduction (vph)	0	15	0
Lane Group Flow (vph)	128	199	0
Turn Type	Perm	NA	
Protected Phases		8	
Permitted Phases	8		
Actuated Green, G (s)	34.7	34.7	
Effective Green, g (s)	34.7	34.7	
Actuated g/C Ratio	0.14	0.14	
Clearance Time (s)	7.6	7.6	
Vehicle Extension (s)	1.0	1.0	
Lane Grp Cap (vph)	197	249	
v/s Ratio Prot		0.12	
v/s Ratio Perm	0.09		
v/c Ratio	0.65	0.80	
Uniform Delay, d1	96.9	99.3	
Progression Factor	1.00	1.00	
Incremental Delay, d2	5.4	15.8	
Delay (s)	102.4	115.1	
Level of Service	F	F	
Approach Delay (s)		110.4	
Approach LOS		F	
<b>Intersection Summary</b>			

# HCM Signalized Intersection Capacity Analysis

## 29: Overcash Dr & SR 580

06/28/2021



Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR
Lane Configurations		↖	↗↖↗			↖	↗↖↗	↗			↖	↗
Traffic Volume (vph)	13	125	2487	16	8	65	1966	77	1	29	2	44
Future Volume (vph)	13	125	2487	16	8	65	1966	77	1	29	2	44
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		7.2	7.2			7.2	7.2	7.2			7.6	7.6
Lane Util. Factor		1.00	0.91			1.00	0.91	1.00			1.00	1.00
Frt		1.00	1.00			1.00	1.00	0.85			1.00	0.85
Flt Protected		0.95	1.00			0.95	1.00	1.00			0.96	1.00
Satd. Flow (prot)		1770	5080			1770	5085	1583			1779	1583
Flt Permitted		0.07	1.00			0.04	1.00	1.00			0.70	1.00
Satd. Flow (perm)		133	5080			66	5085	1583			1300	1583
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	14	133	2646	17	9	69	2091	82	1	31	2	47
RTOR Reduction (vph)	0	0	0	0	0	0	0	17	0	0	0	44
Lane Group Flow (vph)	0	147	2663	0	0	78	2091	65	0	0	34	3
Turn Type	pm+pt	pm+pt	NA		pm+pt	pm+pt	NA	Perm	Perm	Perm	NA	Perm
Protected Phases	1	1	6		5	5	2				4	
Permitted Phases	6	6			2	2		2	4	4		4
Actuated Green, G (s)		208.2	194.9			200.6	191.1	191.1			13.6	13.6
Effective Green, g (s)		208.2	194.9			200.6	191.1	191.1			13.6	13.6
Actuated g/C Ratio		0.87	0.81			0.84	0.80	0.80			0.06	0.06
Clearance Time (s)		7.2	7.2			7.2	7.2	7.2			7.6	7.6
Vehicle Extension (s)		1.0	1.0			1.0	1.0	1.0			1.0	1.0
Lane Grp Cap (vph)		206	4125			122	4048	1260			73	89
v/s Ratio Prot		c0.04	0.52			0.03	0.41				0.03	0.00
v/s Ratio Perm		c0.58				0.50		0.04			0.03	0.00
v/c Ratio		0.71	0.65			0.64	0.52	0.05			0.47	0.03
Uniform Delay, d1		24.7	8.9			34.2	8.5	5.2			109.7	107.0
Progression Factor		2.21	0.12			1.22	0.69	2.49			1.00	1.00
Incremental Delay, d2		6.2	0.5			5.8	0.3	0.1			1.7	0.0
Delay (s)		60.7	1.5			47.5	6.2	13.0			111.4	107.0
Level of Service		E	A			D	A	B			F	F
Approach Delay (s)			4.6				7.9				108.9	
Approach LOS			A				A				F	

### Intersection Summary

HCM 2000 Control Delay	10.3	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.73		
Actuated Cycle Length (s)	240.0	Sum of lost time (s)	22.0
Intersection Capacity Utilization	90.6%	ICU Level of Service	E
Analysis Period (min)	15		

c Critical Lane Group

# HCM Signalized Intersection Capacity Analysis

## 29: Overcash Dr & SR 580

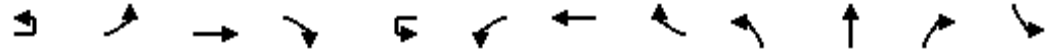
06/28/2021



Movement	SBL	SBT	SBR
Lane Configurations		↕	↗
Traffic Volume (vph)	50	3	59
Future Volume (vph)	50	3	59
Ideal Flow (vphpl)	1900	1900	1900
Total Lost time (s)		7.6	7.6
Lane Util. Factor		1.00	1.00
Frt		1.00	0.85
Flt Protected		0.95	1.00
Satd. Flow (prot)		1779	1583
Flt Permitted		0.71	1.00
Satd. Flow (perm)		1327	1583
Peak-hour factor, PHF	0.94	0.94	0.94
Adj. Flow (vph)	53	3	63
RTOR Reduction (vph)	0	0	59
Lane Group Flow (vph)	0	56	4
Turn Type	Perm	NA	Perm
Protected Phases		8	
Permitted Phases	8		8
Actuated Green, G (s)		13.6	13.6
Effective Green, g (s)		13.6	13.6
Actuated g/C Ratio		0.06	0.06
Clearance Time (s)		7.6	7.6
Vehicle Extension (s)		1.0	1.0
Lane Grp Cap (vph)		75	89
v/s Ratio Prot			
v/s Ratio Perm		0.04	0.00
v/c Ratio		0.75	0.04
Uniform Delay, d1		111.5	107.0
Progression Factor		1.00	1.00
Incremental Delay, d2		29.3	0.1
Delay (s)		140.8	107.1
Level of Service		F	F
Approach Delay (s)		122.9	
Approach LOS		F	
<b>Intersection Summary</b>			

HCM Signalized Intersection Capacity Analysis  
 32: Pinewood Dr/King Arthur Ct & SR 580

06/28/2021



Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL
Lane Configurations												
Traffic Volume (vph)	1	120	2522	18	2	19	2065	31	31	9	10	55
Future Volume (vph)	1	120	2522	18	2	19	2065	31	31	9	10	55
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		7.4	7.4			7.4	7.4			7.2		
Lane Util. Factor		1.00	0.91			1.00	0.91			1.00		
Frt		1.00	1.00			1.00	1.00			0.97		
Flt Protected		0.95	1.00			0.95	1.00			0.97		
Satd. Flow (prot)		1770	5080			1770	5074			1758		
Flt Permitted		0.95	1.00			0.95	1.00			0.78		
Satd. Flow (perm)		1770	5080			1770	5074			1405		
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	1	128	2683	19	2	20	2197	33	33	10	11	59
RTOR Reduction (vph)	0	0	0	0	0	0	1	0	0	8	0	0
Lane Group Flow (vph)	0	129	2702	0	0	22	2229	0	0	46	0	0
Turn Type	Prot	Prot	NA		Prot	Prot	NA		Perm	NA		Perm
Protected Phases	1	1	6		5	5	2			4		
Permitted Phases									4			8
Actuated Green, G (s)		12.2	83.9			4.2	75.9			9.9		
Effective Green, g (s)		12.2	83.9			4.2	75.9			9.9		
Actuated g/C Ratio		0.10	0.70			0.04	0.63			0.08		
Clearance Time (s)		7.4	7.4			7.4	7.4			7.2		
Vehicle Extension (s)		1.0	1.0			1.0	1.0			1.0		
Lane Grp Cap (vph)		179	3551			61	3209			115		
v/s Ratio Prot		c0.07	c0.53			0.01	0.44					
v/s Ratio Perm										0.03		
v/c Ratio		0.72	0.76			0.36	0.69			0.40		
Uniform Delay, d1		52.2	11.6			56.6	14.5			52.2		
Progression Factor		1.06	1.41			0.89	1.28			1.00		
Incremental Delay, d2		9.2	1.3			0.5	0.5			0.8		
Delay (s)		64.7	17.7			50.7	19.0			53.0		
Level of Service		E	B			D	B			D		
Approach Delay (s)			19.8				19.3			53.0		
Approach LOS			B				B			D		
<b>Intersection Summary</b>												
HCM 2000 Control Delay			20.9									C
HCM 2000 Volume to Capacity ratio			0.78									
Actuated Cycle Length (s)			120.0							22.0		
Intersection Capacity Utilization			83.3%									E
Analysis Period (min)			15									

c Critical Lane Group

# HCM Signalized Intersection Capacity Analysis

## 32: Pinewood Dr/King Arthur Ct & SR 580

06/28/2021

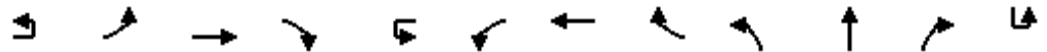


Movement	SBT	SBR
Lane Configurations	↔	↗
Traffic Volume (vph)	6	74
Future Volume (vph)	6	74
Ideal Flow (vphpl)	1900	1900
Total Lost time (s)	7.2	7.2
Lane Util. Factor	1.00	1.00
Frt	1.00	0.85
Flt Protected	0.96	1.00
Satd. Flow (prot)	1782	1583
Flt Permitted	0.71	1.00
Satd. Flow (perm)	1317	1583
Peak-hour factor, PHF	0.94	0.94
Adj. Flow (vph)	6	79
RTOR Reduction (vph)	0	72
Lane Group Flow (vph)	65	7
Turn Type	NA	Perm
Protected Phases	8	
Permitted Phases		8
Actuated Green, G (s)	9.9	9.9
Effective Green, g (s)	9.9	9.9
Actuated g/C Ratio	0.08	0.08
Clearance Time (s)	7.2	7.2
Vehicle Extension (s)	1.0	1.0
Lane Grp Cap (vph)	108	130
v/s Ratio Prot		
v/s Ratio Perm	c0.05	0.00
v/c Ratio	0.60	0.05
Uniform Delay, d1	53.1	50.7
Progression Factor	1.00	1.00
Incremental Delay, d2	6.3	0.1
Delay (s)	59.5	50.8
Level of Service	E	D
Approach Delay (s)	54.7	
Approach LOS	D	
Intersection Summary		

# HCM Signalized Intersection Capacity Analysis

## 35: Belcher Rd & SR 580

06/28/2021



Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBU
Lane Configurations		↔↔	↑↑↑	↗		↖↖	↑↑↑	↗	↖	↑↑	↗	
Traffic Volume (vph)	10	553	1894	206	11	236	1887	583	152	650	106	5
Future Volume (vph)	10	553	1894	206	11	236	1887	583	152	650	106	5
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		7.4	7.4	7.4		7.4	7.4	7.4	7.4	7.4	7.4	
Lane Util. Factor		0.97	0.91	1.00		0.97	0.91	1.00	1.00	0.95	1.00	
Frt		1.00	1.00	0.85		1.00	1.00	0.85	1.00	1.00	0.85	
Flt Protected		0.95	1.00	1.00		0.95	1.00	1.00	0.95	1.00	1.00	
Satd. Flow (prot)		3433	5085	1583		3433	5085	1583	1770	3539	1583	
Flt Permitted		0.95	1.00	1.00		0.95	1.00	1.00	0.95	1.00	1.00	
Satd. Flow (perm)		3433	5085	1583		3433	5085	1583	1770	3539	1583	
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	11	588	2015	219	12	251	2007	620	162	691	113	5
RTOR Reduction (vph)	0	0	0	66	0	0	0	176	0	0	91	0
Lane Group Flow (vph)	0	599	2015	153	0	263	2007	444	162	691	22	0
Turn Type	Prot	Prot	NA	Perm	Prot	Prot	NA	Perm	Prot	NA	Perm	Prot
Protected Phases	1	1	6		5	5	2		7	4		3
Permitted Phases				6				2				4
Actuated Green, G (s)		41.6	115.5	115.5		20.5	94.4	94.4	25.4	46.6	46.6	
Effective Green, g (s)		41.6	115.5	115.5		20.5	94.4	94.4	25.4	46.6	46.6	
Actuated g/C Ratio		0.17	0.48	0.48		0.09	0.39	0.39	0.11	0.19	0.19	
Clearance Time (s)		7.4	7.4	7.4		7.4	7.4	7.4	7.4	7.4	7.4	
Vehicle Extension (s)		1.0	1.0	1.0		1.0	1.0	1.0	1.0	1.0	1.0	
Lane Grp Cap (vph)		595	2447	761		293	2000	622	187	687	307	
v/s Ratio Prot		c0.17	0.40			0.08	c0.39		0.09	c0.20		
v/s Ratio Perm				0.10				0.28				0.01
v/c Ratio		1.01	0.82	0.20		0.90	1.00	0.71	0.87	1.01	0.07	
Uniform Delay, d1		99.2	53.5	35.8		108.7	72.8	61.4	105.6	96.7	79.0	
Progression Factor		0.90	0.95	1.39		0.98	0.69	0.63	1.00	1.00	1.00	
Incremental Delay, d2		32.5	2.3	0.4		18.1	16.2	4.1	30.8	35.8	0.0	
Delay (s)		122.0	53.2	50.3		125.2	66.7	42.9	136.5	132.5	79.1	
Level of Service		F	D	D		F	E	D	F	F	E	
Approach Delay (s)			67.5				66.9			126.9		
Approach LOS			E				E			F		

### Intersection Summary

HCM 2000 Control Delay	79.0	HCM 2000 Level of Service	E
HCM 2000 Volume to Capacity ratio	1.01		
Actuated Cycle Length (s)	240.0	Sum of lost time (s)	29.6
Intersection Capacity Utilization	106.0%	ICU Level of Service	G
Analysis Period (min)	15		

c Critical Lane Group

# HCM Signalized Intersection Capacity Analysis

## 35: Belcher Rd & SR 580

06/28/2021

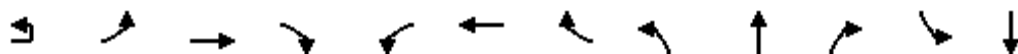


Movement	SBL	SBT	SBR
Lane Configurations			
Traffic Volume (vph)	191	331	207
Future Volume (vph)	191	331	207
Ideal Flow (vphpl)	1900	1900	1900
Total Lost time (s)	7.4	7.4	7.4
Lane Util. Factor	1.00	0.95	1.00
Frt	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00
Satd. Flow (prot)	1770	3539	1583
Flt Permitted	0.95	1.00	1.00
Satd. Flow (perm)	1770	3539	1583
Peak-hour factor, PHF	0.94	0.94	0.94
Adj. Flow (vph)	203	352	220
RTOR Reduction (vph)	0	0	175
Lane Group Flow (vph)	208	352	45
Turn Type	Prot	NA	Perm
Protected Phases	3	8	
Permitted Phases			8
Actuated Green, G (s)	27.8	49.0	49.0
Effective Green, g (s)	27.8	49.0	49.0
Actuated g/C Ratio	0.12	0.20	0.20
Clearance Time (s)	7.4	7.4	7.4
Vehicle Extension (s)	1.0	1.0	1.0
Lane Grp Cap (vph)	205	722	323
v/s Ratio Prot	c0.12	0.10	
v/s Ratio Perm			0.03
v/c Ratio	1.01	0.49	0.14
Uniform Delay, d1	106.1	84.4	78.2
Progression Factor	1.00	1.00	1.00
Incremental Delay, d2	66.7	0.2	0.1
Delay (s)	172.8	84.6	78.3
Level of Service	F	F	E
Approach Delay (s)		106.3	
Approach LOS		F	
<b>Intersection Summary</b>			

# HCM Signalized Intersection Capacity Analysis

## 38: US 19 Frontage & SR 580

06/28/2021



Movement	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations		↔↔	↔↔↔		↔↔	↔↔↔		↔	↔↔	↔	↔	↔↔
Traffic Volume (vph)	5	393	1103	624	553	957	200	1043	389	473	278	217
Future Volume (vph)	5	393	1103	624	553	957	200	1043	389	473	278	217
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		10.5	10.5		10.5	10.5		10.1	10.1	10.1	10.1	10.1
Lane Util. Factor		0.97	0.91		0.97	0.91		0.91	0.91	1.00	0.91	0.91
Frt		1.00	0.95		1.00	0.97		1.00	1.00	0.85	1.00	1.00
Flt Protected		0.95	1.00		0.95	1.00		0.95	0.97	1.00	0.95	0.98
Satd. Flow (prot)		3433	4810		3433	4953		1610	3296	1583	1610	3332
Flt Permitted		0.95	1.00		0.95	1.00		0.95	0.97	1.00	0.95	0.98
Satd. Flow (perm)		3433	4810		3433	4953		1610	3296	1583	1610	3332
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	5	418	1173	664	588	1018	213	1110	414	503	296	231
RTOR Reduction (vph)	0	0	43	0	0	14	0	0	0	156	0	0
Lane Group Flow (vph)	0	423	1794	0	588	1217	0	555	969	347	172	355
Turn Type	Prot	Prot	NA		Prot	NA		Split	NA	Perm	Split	NA
Protected Phases	1	1	6		5	2		4	4		8	8
Permitted Phases										4		
Actuated Green, G (s)		36.1	77.5		36.5	77.9		65.9	65.9	65.9	18.9	18.9
Effective Green, g (s)		36.1	77.5		36.5	77.9		65.9	65.9	65.9	18.9	18.9
Actuated g/C Ratio		0.15	0.32		0.15	0.32		0.27	0.27	0.27	0.08	0.08
Clearance Time (s)		10.5	10.5		10.5	10.5		10.1	10.1	10.1	10.1	10.1
Vehicle Extension (s)		1.0	1.0		1.0	1.0		1.0	1.0	1.0	1.0	1.0
Lane Grp Cap (vph)		516	1553		522	1607		442	905	434	126	262
v/s Ratio Prot		0.12	c0.37		c0.17	0.25		c0.34	0.29		c0.11	0.11
v/s Ratio Perm										0.22		
v/c Ratio		0.82	1.20dr		1.13	0.76		1.26	1.20dl	0.80	1.37	1.35
Uniform Delay, d1		98.8	81.2		101.8	72.6		87.0	87.0	80.9	110.5	110.5
Progression Factor		0.67	0.59		0.77	0.90		1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2		5.7	74.7		76.7	3.0		132.4	50.7	9.3	206.7	182.7
Delay (s)		72.2	122.4		154.8	68.0		219.5	137.8	90.2	317.3	293.2
Level of Service		E	F		F	E		F	F	F	F	F
Approach Delay (s)			113.0			96.1			148.3			237.8
Approach LOS			F			F			F			F

### Intersection Summary

HCM 2000 Control Delay	133.7	HCM 2000 Level of Service	F
HCM 2000 Volume to Capacity ratio	1.20		
Actuated Cycle Length (s)	240.0	Sum of lost time (s)	41.2
Intersection Capacity Utilization	123.7%	ICU Level of Service	H
Analysis Period (min)	15		

dl Defacto Left Lane. Recode with 1 though lane as a left lane.

dr Defacto Right Lane. Recode with 1 though lane as a right lane.

c Critical Lane Group



# HCM Signalized Intersection Capacity Analysis

## 38: US 19 Frontage & SR 580

06/28/2021

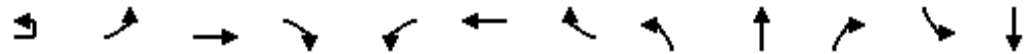


Movement	SBR
Lane Configurations	F
Traffic Volume (vph)	274
Future Volume (vph)	274
Ideal Flow (vphpl)	1900
Total Lost time (s)	10.1
Lane Util. Factor	1.00
Frt	0.85
Flt Protected	1.00
Satd. Flow (prot)	1583
Flt Permitted	1.00
Satd. Flow (perm)	1583
Peak-hour factor, PHF	0.94
Adj. Flow (vph)	291
RTOR Reduction (vph)	202
Lane Group Flow (vph)	89
Turn Type	Perm
Protected Phases	
Permitted Phases	8
Actuated Green, G (s)	18.9
Effective Green, g (s)	18.9
Actuated g/C Ratio	0.08
Clearance Time (s)	10.1
Vehicle Extension (s)	1.0
Lane Grp Cap (vph)	124
v/s Ratio Prot	
v/s Ratio Perm	0.06
v/c Ratio	0.72
Uniform Delay, d1	108.0
Progression Factor	1.00
Incremental Delay, d2	15.3
Delay (s)	123.3
Level of Service	F
Approach Delay (s)	
Approach LOS	
Intersection Summary	

# HCM Signalized Intersection Capacity Analysis

## 41: Summerdale Dr & SR 580

06/28/2021



Movement	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations		↖	↖↖↖		↖	↖↖↖		↖	↖		↖	↖
Traffic Volume (vph)	3	250	1793	105	28	1405	74	70	15	21	104	15
Future Volume (vph)	3	250	1793	105	28	1405	74	70	15	21	104	15
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		6.8	7.0		6.9	7.0		7.5	7.5		7.5	7.5
Lane Util. Factor		1.00	0.91		1.00	0.91		1.00	1.00		1.00	1.00
Frt		1.00	0.99		1.00	0.99		1.00	0.91		1.00	0.86
Flt Protected		0.95	1.00		0.95	1.00		0.95	1.00		0.95	1.00
Satd. Flow (prot)		1770	5043		1770	5047		1770	1701		1770	1607
Flt Permitted		0.10	1.00		0.09	1.00		0.40	1.00		0.73	1.00
Satd. Flow (perm)		191	5043		159	5047		744	1701		1364	1607
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	3	266	1907	112	30	1495	79	74	16	22	111	16
RTOR Reduction (vph)	0	0	4	0	0	3	0	0	20	0	0	153
Lane Group Flow (vph)	0	269	2015	0	30	1571	0	74	18	0	111	35
Turn Type	pm+pt	pm+pt	NA		pm+pt	NA		Perm	NA		Perm	NA
Protected Phases	1	1	6		5	2			4			8
Permitted Phases	6	6			2			4			8	
Actuated Green, G (s)		92.3	82.4		71.8	68.8		13.2	13.2		13.2	13.2
Effective Green, g (s)		92.3	82.4		71.8	68.8		13.2	13.2		13.2	13.2
Actuated g/C Ratio		0.77	0.69		0.60	0.57		0.11	0.11		0.11	0.11
Clearance Time (s)		6.8	7.0		6.9	7.0		7.5	7.5		7.5	7.5
Vehicle Extension (s)		1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0
Lane Grp Cap (vph)		366	3462		135	2893		81	187		150	176
v/s Ratio Prot		c0.10	0.40		0.01	0.31			0.01			0.02
v/s Ratio Perm		c0.46			0.13			c0.10			0.08	
v/c Ratio		0.73	0.58		0.22	0.54		0.91	0.10		0.74	0.20
Uniform Delay, d1		22.4	9.8		10.0	15.9		52.8	48.0		51.7	48.6
Progression Factor		1.77	0.38		0.81	0.84		1.00	1.00		1.00	1.00
Incremental Delay, d2		2.1	0.2		0.2	0.5		70.2	0.1		15.1	0.2
Delay (s)		41.8	3.9		8.3	13.8		123.0	48.1		66.9	48.8
Level of Service		D	A		A	B		F	D		E	D
Approach Delay (s)			8.4			13.7			97.6			55.5
Approach LOS			A			B			F			E

### Intersection Summary

HCM 2000 Control Delay	16.0	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.78		
Actuated Cycle Length (s)	120.0	Sum of lost time (s)	21.4
Intersection Capacity Utilization	81.8%	ICU Level of Service	D
Analysis Period (min)	15		

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis  
 41: Summerdale Dr & SR 580

06/28/2021



Movement	SBR
Lane Configurations	
Traffic Volume (vph)	162
Future Volume (vph)	162
Ideal Flow (vphpl)	1900
Total Lost time (s)	
Lane Util. Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Peak-hour factor, PHF	0.94
Adj. Flow (vph)	172
RTOR Reduction (vph)	0
Lane Group Flow (vph)	0
Turn Type	
Protected Phases	
Permitted Phases	
Actuated Green, G (s)	
Effective Green, g (s)	
Actuated g/C Ratio	
Clearance Time (s)	
Vehicle Extension (s)	
Lane Grp Cap (vph)	
v/s Ratio Prot	
v/s Ratio Perm	
v/c Ratio	
Uniform Delay, d1	
Progression Factor	
Incremental Delay, d2	
Delay (s)	
Level of Service	
Approach Delay (s)	
Approach LOS	
Intersection Summary	

# HCM Signalized Intersection Capacity Analysis

## 44: Countryside Blvd & SR 580

06/28/2021



Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL
Lane Configurations		↖	↑↑↑			↖	↑↑↑		↖	↑↑	↖	↖
Traffic Volume (vph)	3	282	1486	124	1	352	1344	120	52	337	724	76
Future Volume (vph)	3	282	1486	124	1	352	1344	120	52	337	724	76
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		6.9	7.2			7.0	7.2		6.9	7.3	7.3	6.7
Lane Util. Factor		1.00	0.91			1.00	0.91		1.00	0.95	1.00	1.00
Frt		1.00	0.99			1.00	0.99		1.00	1.00	0.85	1.00
Flt Protected		0.95	1.00			0.95	1.00		0.95	1.00	1.00	0.95
Satd. Flow (prot)		1770	5027			1770	5023		1770	3539	1583	1770
Flt Permitted		0.05	1.00			0.04	1.00		0.63	1.00	1.00	0.46
Satd. Flow (perm)		89	5027			84	5023		1175	3539	1583	852
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	3	300	1581	132	1	374	1430	128	55	359	770	81
RTOR Reduction (vph)	0	0	4	0	0	0	4	0	0	0	256	0
Lane Group Flow (vph)	0	303	1709	0	0	375	1554	0	55	359	514	81
Turn Type	pm+pt	pm+pt	NA		pm+pt	pm+pt	NA		pm+pt	NA	Perm	pm+pt
Protected Phases	1	1	6		5	5	2		7	4		3
Permitted Phases	6	6			2	2			4		4	8
Actuated Green, G (s)		122.6	83.6			133.1	88.9		83.8	78.5	78.5	84.0
Effective Green, g (s)		122.6	83.6			133.1	88.9		83.8	78.5	78.5	84.0
Actuated g/C Ratio		0.51	0.35			0.55	0.37		0.35	0.33	0.33	0.35
Clearance Time (s)		6.9	7.2			7.0	7.2		6.9	7.3	7.3	6.7
Vehicle Extension (s)		1.0	1.0			1.0	1.0		1.0	1.0	1.0	1.0
Lane Grp Cap (vph)		318	1751			357	1860		423	1157	517	319
v/s Ratio Prot		0.15	0.34			c0.19	c0.31		0.00	0.10		c0.01
v/s Ratio Perm		0.33				c0.39			0.04		c0.32	0.08
v/c Ratio		0.95	0.98			1.05	0.84		0.13	0.31	0.99	0.25
Uniform Delay, d1		84.2	77.2			86.4	68.9		52.5	60.5	80.5	55.8
Progression Factor		0.94	0.89			0.77	0.96		1.00	1.00	1.00	1.00
Incremental Delay, d2		33.8	14.8			54.5	3.4		0.1	0.1	38.0	0.2
Delay (s)		113.3	83.3			121.4	69.5		52.5	60.5	118.5	55.9
Level of Service		F	F			F	E		D	E	F	E
Approach Delay (s)			87.8				79.5			97.9		
Approach LOS			F				E			F		

### Intersection Summary

HCM 2000 Control Delay	84.6	HCM 2000 Level of Service	F
HCM 2000 Volume to Capacity ratio	1.03		
Actuated Cycle Length (s)	240.0	Sum of lost time (s)	28.4
Intersection Capacity Utilization	123.6%	ICU Level of Service	H
Analysis Period (min)	15		

c Critical Lane Group

# HCM Signalized Intersection Capacity Analysis

## 44: Countryside Blvd & SR 580

06/28/2021

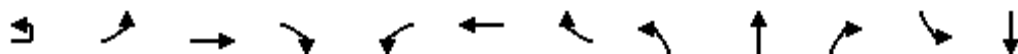


Movement	SBT	SBR
Lane Configurations	↑↑	↑
Traffic Volume (vph)	164	186
Future Volume (vph)	164	186
Ideal Flow (vphpl)	1900	1900
Total Lost time (s)	7.3	7.3
Lane Util. Factor	0.95	1.00
Frt	1.00	0.85
Flt Protected	1.00	1.00
Satd. Flow (prot)	3539	1583
Flt Permitted	1.00	1.00
Satd. Flow (perm)	3539	1583
Peak-hour factor, PHF	0.94	0.94
Adj. Flow (vph)	174	198
RTOR Reduction (vph)	0	133
Lane Group Flow (vph)	174	65
Turn Type	NA	Perm
Protected Phases	8	
Permitted Phases		8
Actuated Green, G (s)	78.5	78.5
Effective Green, g (s)	78.5	78.5
Actuated g/C Ratio	0.33	0.33
Clearance Time (s)	7.3	7.3
Vehicle Extension (s)	1.0	1.0
Lane Grp Cap (vph)	1157	517
v/s Ratio Prot	0.05	
v/s Ratio Perm		0.04
v/c Ratio	0.15	0.13
Uniform Delay, d1	57.1	56.7
Progression Factor	1.00	1.00
Incremental Delay, d2	0.0	0.0
Delay (s)	57.2	56.7
Level of Service	E	E
Approach Delay (s)	56.7	
Approach LOS	E	
<b>Intersection Summary</b>		

# HCM Signalized Intersection Capacity Analysis

## 47: Landmark Dr & SR 580

06/28/2021



Movement	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	9	138	1845	294	32	1473	42	272	212	47	43	112
Future Volume (vph)	9	138	1845	294	32	1473	42	272	212	47	43	112
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		6.9	6.9		6.9	6.9		6.2	6.8		6.2	6.8
Lane Util. Factor		1.00	0.91		1.00	0.91		1.00	0.95		1.00	0.95
Frt		1.00	0.98		1.00	1.00		1.00	0.97		1.00	0.92
Flt Protected		0.95	1.00		0.95	1.00		0.95	1.00		0.95	1.00
Satd. Flow (prot)		1770	4980		1770	5064		1770	3443		1770	3264
Flt Permitted		0.09	1.00		0.07	1.00		0.29	1.00		0.58	1.00
Satd. Flow (perm)		163	4980		134	5064		542	3443		1084	3264
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	10	147	1963	313	34	1567	45	289	226	50	46	119
RTOR Reduction (vph)	0	0	15	0	0	2	0	0	17	0	0	118
Lane Group Flow (vph)	0	157	2261	0	34	1610	0	289	259	0	46	129
Turn Type	pm+pt	pm+pt	NA		pm+pt	NA		pm+pt	NA		pm+pt	NA
Protected Phases	1	1	6		5	2		7	4		3	8
Permitted Phases	6	6			2			4			8	
Actuated Green, G (s)		75.7	65.8		58.6	55.6		30.6	20.4		13.4	9.4
Effective Green, g (s)		75.7	65.8		58.6	55.6		30.6	20.4		13.4	9.4
Actuated g/C Ratio		0.63	0.55		0.49	0.46		0.26	0.17		0.11	0.08
Clearance Time (s)		6.9	6.9		6.9	6.9		6.2	6.8		6.2	6.8
Vehicle Extension (s)		1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0
Lane Grp Cap (vph)		279	2730		106	2346		291	585		143	255
v/s Ratio Prot		c0.06	c0.45		0.01	0.32		c0.12	0.08		0.01	0.04
v/s Ratio Perm		0.29			0.15			c0.13			0.03	
v/c Ratio		0.56	0.83		0.32	0.69		0.99	0.44		0.32	0.51
Uniform Delay, d1		31.5	22.4		46.0	25.3		41.4	44.7		48.6	53.1
Progression Factor		0.78	0.88		1.22	0.94		1.00	1.00		1.00	1.00
Incremental Delay, d2		0.5	1.0		0.6	1.5		50.6	0.2		0.5	0.6
Delay (s)		25.0	20.6		56.8	25.4		92.0	44.9		49.1	53.6
Level of Service		C	C		E	C		F	D		D	D
Approach Delay (s)			20.9			26.1			69.0			52.9
Approach LOS			C			C			E			D

### Intersection Summary

HCM 2000 Control Delay	30.0	HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio	0.92		
Actuated Cycle Length (s)	120.0	Sum of lost time (s)	26.8
Intersection Capacity Utilization	90.7%	ICU Level of Service	E
Analysis Period (min)	15		

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis  
 47: Landmark Dr & SR 580

06/28/2021

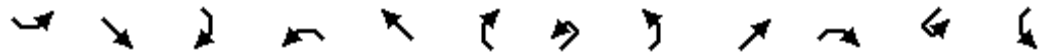


Movement	SBR
Lane Configurations	
Traffic Volume (vph)	120
Future Volume (vph)	120
Ideal Flow (vphpl)	1900
Total Lost time (s)	
Lane Util. Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Peak-hour factor, PHF	0.94
Adj. Flow (vph)	128
RTOR Reduction (vph)	0
Lane Group Flow (vph)	0
Turn Type	
Protected Phases	
Permitted Phases	
Actuated Green, G (s)	
Effective Green, g (s)	
Actuated g/C Ratio	
Clearance Time (s)	
Vehicle Extension (s)	
Lane Grp Cap (vph)	
v/s Ratio Prot	
v/s Ratio Perm	
v/c Ratio	
Uniform Delay, d1	
Progression Factor	
Incremental Delay, d2	
Delay (s)	
Level of Service	
Approach Delay (s)	
Approach LOS	
Intersection Summary	

# HCM Signalized Intersection Capacity Analysis

## 51: SR 580 & Charles Ave

06/28/2021



Movement	SEL	SET	SER	NWL	NWT	NWR	NEU	NEL	NET	NER	SWU	SWL
Lane Configurations		↕	↗		↔			↖	↗↗↗			↖
Traffic Volume (vph)	19	23	23	50	1	24	7	11	1885	43	8	5
Future Volume (vph)	19	23	23	50	1	24	7	11	1885	43	8	5
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		7.6	7.6		7.6			6.9	6.9			7.1
Lane Util. Factor		1.00	1.00		1.00			1.00	0.91			1.00
Frt		1.00	0.85		0.96			1.00	1.00			1.00
Flt Protected		0.98	1.00		0.97			0.95	1.00			0.95
Satd. Flow (prot)		1821	1583		1724			1770	5068			1770
Flt Permitted		0.85	1.00		0.77			0.13	1.00			0.07
Satd. Flow (perm)		1586	1583		1378			237	5068			132
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	20	24	24	53	1	26	7	12	2005	46	9	5
RTOR Reduction (vph)	0	0	22	0	16	0	0	0	1	0	0	0
Lane Group Flow (vph)	0	44	2	0	64	0	0	19	2050	0	0	14
Turn Type	Perm	NA	Perm	Perm	NA		pm+pt	pm+pt	NA		pm+pt	pm+pt
Protected Phases		8			4		1	1	6		5	5
Permitted Phases	8		8	4			6	6			2	2
Actuated Green, G (s)		10.5	10.5		10.5			88.1	85.3			87.9
Effective Green, g (s)		10.5	10.5		10.5			88.1	85.3			87.9
Actuated g/C Ratio		0.09	0.09		0.09			0.73	0.71			0.73
Clearance Time (s)		7.6	7.6		7.6			6.9	6.9			7.1
Vehicle Extension (s)		4.0	4.0		4.0			4.0	5.0			4.0
Lane Grp Cap (vph)		138	138		120			209	3602			132
v/s Ratio Prot								0.00	c0.40			c0.00
v/s Ratio Perm		0.03	0.00		c0.05			0.06				0.08
v/c Ratio		0.32	0.02		0.53			0.09	0.57			0.11
Uniform Delay, d1		51.4	50.0		52.4			7.9	8.4			11.4
Progression Factor		1.00	1.00		1.00			0.17	0.34			0.80
Incremental Delay, d2		1.8	0.1		5.4			0.2	0.4			0.3
Delay (s)		53.2	50.1		57.8			1.5	3.3			9.4
Level of Service		D	D		E			A	A			A
Approach Delay (s)		52.1			57.8				3.3			
Approach LOS		D			E				A			

### Intersection Summary

HCM 2000 Control Delay	7.2	HCM 2000 Level of Service	A
HCM 2000 Volume to Capacity ratio	0.55		
Actuated Cycle Length (s)	120.0	Sum of lost time (s)	21.6
Intersection Capacity Utilization	64.2%	ICU Level of Service	C
Analysis Period (min)	15		

c Critical Lane Group



HCM Signalized Intersection Capacity Analysis  
 51: SR 580 & Charles Ave

06/28/2021



Movement	SWT	SWR
Lane Configurations	↑↑↑	
Traffic Volume (vph)	1511	3
Future Volume (vph)	1511	3
Ideal Flow (vphpl)	1900	1900
Total Lost time (s)	6.9	
Lane Util. Factor	0.91	
Frt	1.00	
Flt Protected	1.00	
Satd. Flow (prot)	5084	
Flt Permitted	1.00	
Satd. Flow (perm)	5084	
Peak-hour factor, PHF	0.94	0.94
Adj. Flow (vph)	1607	3
RTOR Reduction (vph)	0	0
Lane Group Flow (vph)	1610	0
Turn Type	NA	
Protected Phases	2	
Permitted Phases		
Actuated Green, G (s)	85.3	
Effective Green, g (s)	85.3	
Actuated g/C Ratio	0.71	
Clearance Time (s)	6.9	
Vehicle Extension (s)	5.0	
Lane Grp Cap (vph)	3613	
v/s Ratio Prot	0.32	
v/s Ratio Perm		
v/c Ratio	0.45	
Uniform Delay, d1	7.3	
Progression Factor	1.04	
Incremental Delay, d2	0.2	
Delay (s)	7.9	
Level of Service	A	
Approach Delay (s)	7.9	
Approach LOS	A	
Intersection Summary		

# HCM Signalized Intersection Capacity Analysis

## 54: SR 580 & McMullen Booth Rd

06/28/2021

Movement	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR	NEU	NEL	NET	NER
Lane Configurations												
Traffic Volume (vph)	16	245	2197	860	2	220	1468	742	3	615	1053	173
Future Volume (vph)	16	245	2197	860	2	220	1468	742	3	615	1053	173
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		9.8	9.8	9.8		9.8	9.8	9.8		9.8	9.8	9.8
Lane Util. Factor		0.97	0.91	1.00		0.97	0.91	1.00		0.97	0.91	1.00
Frt		1.00	1.00	0.85		1.00	1.00	0.85		1.00	1.00	0.85
Flt Protected		0.95	1.00	1.00		0.95	1.00	1.00		0.95	1.00	1.00
Satd. Flow (prot)		3433	5085	1583		3433	5085	1583		3433	5085	1583
Flt Permitted		0.95	1.00	1.00		0.95	1.00	1.00		0.95	1.00	1.00
Satd. Flow (perm)		3433	5085	1583		3433	5085	1583		3433	5085	1583
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	17	261	2337	915	2	234	1562	789	3	654	1120	184
RTOR Reduction (vph)	0	0	0	138	0	0	0	149	0	0	0	88
Lane Group Flow (vph)	0	278	2337	777	0	236	1562	640	0	657	1120	96
Turn Type	Prot	Prot	NA	Perm	Prot	Prot	NA	Perm	Prot	Prot	NA	Perm
Protected Phases	1	1	6		5	5	2		3	3	8	
Permitted Phases				6				2				8
Actuated Green, G (s)		17.3	104.1	104.1		10.2	97.0	97.0		27.2	63.3	63.3
Effective Green, g (s)		17.3	104.1	104.1		10.2	97.0	97.0		27.2	63.3	63.3
Actuated g/C Ratio		0.07	0.43	0.43		0.04	0.40	0.40		0.11	0.26	0.26
Clearance Time (s)		9.8	9.8	9.8		9.8	9.8	9.8		9.8	9.8	9.8
Vehicle Extension (s)		3.0	3.0	3.0		3.0	3.0	3.0		3.0	3.0	3.0
Lane Grp Cap (vph)		247	2205	686		145	2055	639		389	1341	417
v/s Ratio Prot		c0.08	0.46			c0.07	0.31			c0.19	c0.22	
v/s Ratio Perm				c0.49				0.40				0.06
v/c Ratio		1.13	1.06	1.13		1.63	0.76	1.00		1.69	0.84	0.23
Uniform Delay, d1		111.3	68.0	68.0		114.9	61.5	71.5		106.4	83.4	69.2
Progression Factor		1.00	1.00	1.00		1.00	1.00	1.00		1.15	0.85	1.04
Incremental Delay, d2		95.2	37.4	77.3		311.6	2.7	36.0		319.6	4.1	0.2
Delay (s)		206.6	105.3	145.2		426.5	64.2	107.5		441.8	75.2	72.2
Level of Service		F	F	F		F	E	F		F	E	E
Approach Delay (s)			123.6				110.5				197.7	
Approach LOS			F				F				F	
<b>Intersection Summary</b>												
HCM 2000 Control Delay			145.2				HCM 2000 Level of Service				F	
HCM 2000 Volume to Capacity ratio			1.19									
Actuated Cycle Length (s)			240.0				Sum of lost time (s)			39.2		
Intersection Capacity Utilization			126.7%				ICU Level of Service			H		
Analysis Period (min)			15									

c Critical Lane Group

# HCM Signalized Intersection Capacity Analysis

## 54: SR 580 & McMullen Booth Rd

06/28/2021



Movement	SWU	SWL	SWT	SWR
Lane Configurations				
Traffic Volume (vph)	1	494	783	177
Future Volume (vph)	1	494	783	177
Ideal Flow (vphpl)	1900	1900	1900	1900
Total Lost time (s)		9.8	9.8	9.8
Lane Util. Factor		0.97	0.91	1.00
Frt		1.00	1.00	0.85
Flt Protected		0.95	1.00	1.00
Satd. Flow (prot)		3433	5085	1583
Flt Permitted		0.95	1.00	1.00
Satd. Flow (perm)		3433	5085	1583
Peak-hour factor, PHF	0.94	0.94	0.94	0.94
Adj. Flow (vph)	1	526	833	188
RTOR Reduction (vph)	0	0	0	90
Lane Group Flow (vph)	0	527	833	98
Turn Type	Prot	Prot	NA	Perm
Protected Phases	7	7	4	
Permitted Phases				4
Actuated Green, G (s)		23.2	59.3	59.3
Effective Green, g (s)		23.2	59.3	59.3
Actuated g/C Ratio		0.10	0.25	0.25
Clearance Time (s)		9.8	9.8	9.8
Vehicle Extension (s)		4.0	3.0	3.0
Lane Grp Cap (vph)		331	1256	391
v/s Ratio Prot		0.15	0.16	
v/s Ratio Perm				0.06
v/c Ratio		1.59	0.66	0.25
Uniform Delay, d1		108.4	81.4	72.5
Progression Factor		1.00	1.00	1.00
Incremental Delay, d2		280.4	1.3	0.3
Delay (s)		388.8	82.7	72.8
Level of Service		F	F	E
Approach Delay (s)			185.7	
Approach LOS			F	
<b>Intersection Summary</b>				

# HCM Signalized Intersection Capacity Analysis

## 57: SR 580 & St Petersburg Dr/Forest Lakes Blvd

06/28/2021



Movement	SEL	SET	SER	NWL	NWT	NWR	NEU	NEL	NET	NER	SWL	SWT
Lane Configurations		↕	↕		↕			↕	↕↕	↕	↕	↕↕
Traffic Volume (vph)	38	120	918	146	68	2	1	643	1294	198	3	894
Future Volume (vph)	38	120	918	146	68	2	1	643	1294	198	3	894
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		7.2	7.2		7.2			6.9	6.9	6.9	6.9	6.9
Lane Util. Factor		1.00	1.00		1.00			1.00	0.95	1.00	1.00	0.95
Frt		1.00	0.85		1.00			1.00	1.00	0.85	1.00	1.00
Flt Protected		0.99	1.00		0.97			0.95	1.00	1.00	0.95	1.00
Satd. Flow (prot)		1841	1583		1800			1770	3539	1583	1770	3539
Flt Permitted		0.88	1.00		0.63			0.15	1.00	1.00	0.95	1.00
Satd. Flow (perm)		1635	1583		1173			272	3539	1583	1770	3539
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	40	128	977	155	72	2	1	684	1377	211	3	951
RTOR Reduction (vph)	0	0	507	0	0	0	0	0	0	63	0	0
Lane Group Flow (vph)	0	168	470	0	229	0	0	685	1377	148	3	951
Turn Type	Perm	NA	Perm	Perm	NA		pm+pt	pm+pt	NA	Perm	Prot	NA
Protected Phases		8			4		1	1	6		5	2
Permitted Phases	8		8	4			6	6		6		
Actuated Green, G (s)		26.0	26.0		26.0			72.0	72.0	72.0	1.0	28.4
Effective Green, g (s)		26.0	26.0		26.0			72.0	72.0	72.0	1.0	28.4
Actuated g/C Ratio		0.22	0.22		0.22			0.60	0.60	0.60	0.01	0.24
Clearance Time (s)		7.2	7.2		7.2			6.9	6.9	6.9	6.9	6.9
Vehicle Extension (s)		3.0	3.0		3.0			4.0	4.0	4.0	3.0	5.0
Lane Grp Cap (vph)		354	342		254			719	2123	949	14	837
v/s Ratio Prot								c0.35	0.39		0.00	c0.27
v/s Ratio Perm		0.10	c0.30		0.20			0.22		0.09		
v/c Ratio		0.47	1.37		0.90			0.95	0.65	0.16	0.21	1.14
Uniform Delay, d1		41.0	47.0		45.8			32.5	15.7	10.6	59.1	45.8
Progression Factor		1.00	1.00		1.00			1.00	1.00	1.00	1.32	0.47
Incremental Delay, d2		1.0	186.2		31.8			22.7	1.5	0.4	6.9	74.8
Delay (s)		42.0	233.2		77.5			55.2	17.3	10.9	85.1	96.2
Level of Service		D	F		E			E	B	B	F	F
Approach Delay (s)		205.1			77.5				28.1			93.7
Approach LOS		F			E				C			F

### Intersection Summary

HCM 2000 Control Delay	88.3	HCM 2000 Level of Service	F
HCM 2000 Volume to Capacity ratio	1.12		
Actuated Cycle Length (s)	120.0	Sum of lost time (s)	21.0
Intersection Capacity Utilization	152.5%	ICU Level of Service	H
Analysis Period (min)	15		

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis  
 57: SR 580 & St Petersburg Dr/Forest Lakes Blvd

06/28/2021



Movement	SWR
Lane Configurations	7
Traffic Volume (vph)	39
Future Volume (vph)	39
Ideal Flow (vphpl)	1900
Total Lost time (s)	6.9
Lane Util. Factor	1.00
Frt	0.85
Flt Protected	1.00
Satd. Flow (prot)	1583
Flt Permitted	1.00
Satd. Flow (perm)	1583
Peak-hour factor, PHF	0.94
Adj. Flow (vph)	41
RTOR Reduction (vph)	31
Lane Group Flow (vph)	10
Turn Type	Perm
Protected Phases	
Permitted Phases	2
Actuated Green, G (s)	28.4
Effective Green, g (s)	28.4
Actuated g/C Ratio	0.24
Clearance Time (s)	6.9
Vehicle Extension (s)	5.0
Lane Grp Cap (vph)	374
v/s Ratio Prot	
v/s Ratio Perm	0.01
v/c Ratio	0.03
Uniform Delay, d1	35.2
Progression Factor	1.00
Incremental Delay, d2	0.1
Delay (s)	35.3
Level of Service	D
Approach Delay (s)	
Approach LOS	
Intersection Summary	

# HCM Signalized Intersection Capacity Analysis

## 59: SR 590/2nd St & SR 580

06/28/2021



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Traffic Volume (vph)	16	1780	154	246	1436	16	4	186	9	524	8	4
Future Volume (vph)	16	1780	154	246	1436	16	4	186	9	524	8	4
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	6.9	6.9	6.9	6.9	6.9				8.3	8.3		8.3
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95				1.00	1.00		1.00
Frt	1.00	1.00	0.85	1.00	1.00				1.00	0.85		1.00
Flt Protected	0.95	1.00	1.00	0.95	1.00				0.95	1.00		0.97
Satd. Flow (prot)	1770	3539	1583	1770	3533				1778	1583		1800
Flt Permitted	0.95	1.00	1.00	0.95	1.00				0.73	1.00		0.75
Satd. Flow (perm)	1770	3539	1583	1770	3533				1354	1583		1388
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	17	1894	164	262	1528	17	4	198	10	557	9	4
RTOR Reduction (vph)	0	0	69	0	0	0	0	0	0	143	0	0
Lane Group Flow (vph)	17	1894	95	262	1545	0	0	0	212	414	0	13
Turn Type	Prot	NA	Perm	Prot	NA		Perm	Perm	NA	Perm	Perm	NA
Protected Phases	1	6		5	2				4			8
Permitted Phases			6				4	4		4	8	
Actuated Green, G (s)	2.8	77.8	77.8	20.1	95.1				24.1	24.1		24.1
Effective Green, g (s)	2.8	77.8	77.8	20.1	95.1				24.1	24.1		24.1
Actuated g/C Ratio	0.02	0.54	0.54	0.14	0.66				0.17	0.17		0.17
Clearance Time (s)	6.9	6.9	6.9	6.9	6.9				8.3	8.3		8.3
Vehicle Extension (s)	2.0	4.0	4.0	3.0	4.0				3.0	3.0		3.0
Lane Grp Cap (vph)	34	1910	854	246	2331				226	264		232
v/s Ratio Prot	0.01	c0.54		c0.15	0.44							
v/s Ratio Perm			0.06						0.16	c0.26		0.01
v/c Ratio	0.50	0.99	0.11	1.07	0.66				0.94	1.57		0.06
Uniform Delay, d1	70.0	32.8	16.2	62.0	14.8				59.3	60.0		50.4
Progression Factor	1.00	1.00	1.00	1.00	1.00				1.00	1.00		1.00
Incremental Delay, d2	4.2	18.7	0.3	75.6	1.5				42.5	272.9		0.1
Delay (s)	74.1	51.5	16.5	137.6	16.3				101.7	332.9		50.5
Level of Service	E	D	B	F	B				F	F		D
Approach Delay (s)		49.0			33.9				269.2			50.3
Approach LOS		D			C				F			D

### Intersection Summary

HCM 2000 Control Delay	79.3	HCM 2000 Level of Service	E
HCM 2000 Volume to Capacity ratio	1.12		
Actuated Cycle Length (s)	144.1	Sum of lost time (s)	22.1
Intersection Capacity Utilization	109.6%	ICU Level of Service	H
Analysis Period (min)	15		

c Critical Lane Group

# HCM Signalized Intersection Capacity Analysis

59: SR 590/2nd St & SR 580

06/28/2021



Movement	SBR
Lane Configurations	7
Traffic Volume (vph)	14
Future Volume (vph)	14
Ideal Flow (vphpl)	1900
Total Lost time (s)	8.3
Lane Util. Factor	1.00
Frt	0.85
Flt Protected	1.00
Satd. Flow (prot)	1583
Flt Permitted	1.00
Satd. Flow (perm)	1583
Peak-hour factor, PHF	0.94
Adj. Flow (vph)	15
RTOR Reduction (vph)	12
Lane Group Flow (vph)	3
Turn Type	Perm
Protected Phases	
Permitted Phases	8
Actuated Green, G (s)	24.1
Effective Green, g (s)	24.1
Actuated g/C Ratio	0.17
Clearance Time (s)	8.3
Vehicle Extension (s)	3.0
Lane Grp Cap (vph)	264
v/s Ratio Prot	
v/s Ratio Perm	0.00
v/c Ratio	0.01
Uniform Delay, d1	50.0
Progression Factor	1.00
Incremental Delay, d2	0.0
Delay (s)	50.1
Level of Service	D
Approach Delay (s)	
Approach LOS	
Intersection Summary	

# HCM Signalized Intersection Capacity Analysis

## 68: S Bayview Blvd & SR 580

06/28/2021



Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL
Lane Configurations												
Traffic Volume (vph)	1	84	1078	15	1	11	812	17	44	75	13	14
Future Volume (vph)	1	84	1078	15	1	11	812	17	44	75	13	14
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		6.9	6.9			6.9	6.9		7.5	7.5		
Lane Util. Factor		1.00	0.95			1.00	0.95		1.00	1.00		
Frt		1.00	1.00			1.00	1.00		1.00	0.98		
Flt Protected		0.95	1.00			0.95	1.00		0.95	1.00		
Satd. Flow (prot)		1770	3532			1770	3528		1770	1821		
Flt Permitted		0.29	1.00			0.19	1.00		0.27	1.00		
Satd. Flow (perm)		535	3532			361	3528		496	1821		
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	1	89	1147	16	1	12	864	18	47	80	14	15
RTOR Reduction (vph)	0	0	1	0	0	0	1	0	0	6	0	0
Lane Group Flow (vph)	0	90	1162	0	0	13	881	0	47	88	0	0
Turn Type	Perm	Perm	NA		Perm	Perm	NA		Perm	NA		Perm
Protected Phases			6				2			4		
Permitted Phases	6	6			2	2			4			8
Actuated Green, G (s)		76.3	76.3			76.3	76.3		29.3	29.3		
Effective Green, g (s)		76.3	76.3			76.3	76.3		29.3	29.3		
Actuated g/C Ratio		0.64	0.64			0.64	0.64		0.24	0.24		
Clearance Time (s)		6.9	6.9			6.9	6.9		7.5	7.5		
Vehicle Extension (s)		5.0	5.0			5.0	5.0		4.0	4.0		
Lane Grp Cap (vph)		340	2245			229	2243		121	444		
v/s Ratio Prot			c0.33				0.25			0.05		
v/s Ratio Perm		0.17				0.04			0.09			
v/c Ratio		0.26	0.52			0.06	0.39		0.39	0.20		
Uniform Delay, d1		9.6	11.9			8.3	10.6		37.9	36.0		
Progression Factor		0.35	0.32			0.12	0.16		1.00	1.00		
Incremental Delay, d2		1.5	0.7			0.4	0.4		2.8	0.3		
Delay (s)		4.9	4.5			1.3	2.1		40.7	36.3		
Level of Service		A	A			A	A		D	D		
Approach Delay (s)			4.5				2.1			37.8		
Approach LOS			A				A			D		

### Intersection Summary

HCM 2000 Control Delay	12.6	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.59		
Actuated Cycle Length (s)	120.0	Sum of lost time (s)	14.4
Intersection Capacity Utilization	93.5%	ICU Level of Service	F
Analysis Period (min)	15		

c Critical Lane Group



HCM Signalized Intersection Capacity Analysis  
 68: S Bayview Blvd & SR 580

06/28/2021



Movement	SBT	SBR
Lane Configurations	↕	
Traffic Volume (vph)	100	262
Future Volume (vph)	100	262
Ideal Flow (vphpl)	1900	1900
Total Lost time (s)	7.5	
Lane Util. Factor	1.00	
Frt	0.91	
Flt Protected	1.00	
Satd. Flow (prot)	1684	
Flt Permitted	0.99	
Satd. Flow (perm)	1665	
Peak-hour factor, PHF	0.94	0.94
Adj. Flow (vph)	106	279
RTOR Reduction (vph)	82	0
Lane Group Flow (vph)	318	0
Turn Type	NA	
Protected Phases	8	
Permitted Phases		
Actuated Green, G (s)	29.3	
Effective Green, g (s)	29.3	
Actuated g/C Ratio	0.24	
Clearance Time (s)	7.5	
Vehicle Extension (s)	4.0	
Lane Grp Cap (vph)	406	
v/s Ratio Prot		
v/s Ratio Perm	c0.19	
v/c Ratio	0.78	
Uniform Delay, d1	42.4	
Progression Factor	1.00	
Incremental Delay, d2	10.0	
Delay (s)	52.4	
Level of Service	D	
Approach Delay (s)	52.4	
Approach LOS	D	
<b>Intersection Summary</b>		

# HCM Signalized Intersection Capacity Analysis

## 71: Tampa Rd & SR 580

06/28/2021



Movement	EBL	EBR	SET	SER	NWU	NWL	NWT
Lane Configurations		↑	↑↑↑			↑↑	↑↑↑
Traffic Volume (vph)	0	1043	1647	0	1	942	2175
Future Volume (vph)	0	1043	1647	0	1	942	2175
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		8.1	8.1			8.4	8.1
Lane Util. Factor		1.00	0.91			0.97	0.91
Frt		0.86	1.00			1.00	1.00
Flt Protected		1.00	1.00			0.95	1.00
Satd. Flow (prot)		1611	5085			3433	5085
Flt Permitted		1.00	1.00			0.95	1.00
Satd. Flow (perm)		1611	5085			3433	5085
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	0	1110	1752	0	1	1002	2314
RTOR Reduction (vph)	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	1110	1752	0	0	1003	2314
Turn Type		Perm	NA		Prot	Prot	NA
Protected Phases			6		5	5	2
Permitted Phases		2 5 6					
Actuated Green, G (s)		120.0	63.3			40.2	120.0
Effective Green, g (s)		120.0	63.3			40.2	120.0
Actuated g/C Ratio		1.00	0.53			0.34	1.00
Clearance Time (s)			8.1			8.4	8.1
Vehicle Extension (s)			1.0			1.0	1.0
Lane Grp Cap (vph)		1611	2682			1150	5085
v/s Ratio Prot			0.34			c0.29	0.46
v/s Ratio Perm		c0.69					
v/c Ratio		0.69	0.65			0.87	0.46
Uniform Delay, d1		0.0	20.4			37.5	0.0
Progression Factor		1.00	1.00			1.00	1.00
Incremental Delay, d2		0.9	1.3			7.3	0.3
Delay (s)		0.9	21.7			44.7	0.3
Level of Service		A	C			D	A
Approach Delay (s)	0.9		21.7				13.7
Approach LOS	A		C				B

### Intersection Summary

HCM 2000 Control Delay	13.7	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.81		
Actuated Cycle Length (s)	120.0	Sum of lost time (s)	16.5
Intersection Capacity Utilization	143.8%	ICU Level of Service	H
Analysis Period (min)	15		

c Critical Lane Group

## Arterial Level of Service

06/28/2021

### Arterial Level of Service: EB SR 580

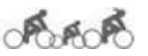
Cross Street	Arterial Class	Flow Speed	Running Time	Signal Delay	Travel Time (s)	Dist (mi)	Arterial Speed	Arterial LOS
Main St	II	39	44.7	20.0	64.7	0.48	26.7	C
Patricia Ave	II	40	40.8	22.5	63.3	0.42	24.2	C
Crosley Dr	II	40	27.9	5.4	33.3	0.25	27.4	C
Lake Haven Rd	II	40	15.2	3.4	18.6	0.13	25.6	C
Keene Rd	II	45	35.4	60.8	96.2	0.37	13.8	E
Achieva Way	II	45	27.9	10.6	38.5	0.27	25.1	C
Overcash Dr	II	45	20.0	1.6	21.6	0.18	30.6	B
Pinewood Dr	II	45	30.8	17.2	48.0	0.31	23.3	C
Belcher Rd	II	45	26.4	53.7	80.1	0.25	11.4	F
US 19 Frontage	II	45	42.4	114.1	156.5	0.53	12.2	F
Summerdale Dr	II	45	26.8	3.9	30.7	0.26	30.2	B
Countryside Blvd	II	45	43.6	82.3	125.9	0.55	15.6	E
Landmark Dr	II	45	45.8	18.2	64.0	0.52	29.3	B
Charles Ave	II	45	32.5	3.0	35.5	0.34	34.3	B
McMullen Booth Rd	II	45	23.7	76.4	100.1	0.22	7.8	F
SR 590	II	45	75.7	58.0	133.7	0.95	25.5	C
St Petersburg Dr	II	45	97.2	14.4	111.6	1.22	39.2	A
S Bayview Blvd	II	45	58.7	5.0	63.7	0.73	41.5	A
Tampa Rd	II	45	38.1	14.0	52.1	0.41	28.0	B
Total	II		753.6	584.5	1338.1	8.39	22.6	C

### Arterial Level of Service: WB SR 580

Cross Street	Arterial Class	Flow Speed	Running Time	Signal Delay	Travel Time (s)	Dist (mi)	Arterial Speed	Arterial LOS
S Bayview Blvd	II	45	38.1	2.3	40.4	0.41	36.1	A
Forest Lakes Blvd	II	45	58.7	39.3	98.0	0.73	27.0	C
2nd St	II	45	97.2	15.3	112.5	1.22	38.9	A
McMullen Booth Rd	II	45	75.7	83.7	159.4	0.95	21.4	D
Charles Ave	II	45	23.7	7.3	31.0	0.22	25.3	C
Landmark Dr	II	45	32.5	22.8	55.3	0.34	22.0	C
Countryside Blvd	II	45	45.8	70.1	115.9	0.52	16.2	E
Summerdale Dr	II	45	43.6	14.9	58.5	0.55	33.6	B
US 19 Frontage	II	45	26.8	67.2	94.0	0.26	9.9	F
Belcher Rd	II	45	42.4	66.0	108.4	0.53	17.6	D
King Arthur Ct	II	45	26.4	20.2	46.6	0.25	19.6	D
Overcash Dr	II	45	30.8	6.7	37.5	0.31	29.8	B
Sunlight Dr	II	45	20.0	8.9	28.9	0.18	22.9	C
CR 1	II	45	27.9	51.7	79.6	0.27	12.1	F
Lake Haven Rd	II	45	35.4	0.4	35.8	0.37	37.1	A
Pinehurst Rd	II	40	15.2	7.0	22.2	0.13	21.4	D
Patricia Ave	II	40	27.9	5.0	32.9	0.25	27.7	C
Bass Blvd	II	40	40.8	9.8	50.6	0.42	30.2	B
Alt 19	II	39	44.7	33.5	78.2	0.48	22.1	C
Total	II		753.6	532.1	1285.7	8.39	23.5	C



**SR 580 Corridor Planning and Concept Development Study**  
**APPENDIX B: DRAFT Bicycle and Pedestrian Level of Service Results**



Seg_ID	SR	Road Name	From	To	Length (Ls) (mi)	Dir. of Sur.	Lanes (L)		ADT	Tks. (HV) (%)	Post. Spd. (SP <sub>p</sub> ) mph	Width of Pavement			Total Pvmt Width (TPW) (ft)	Occ. Park. (OSPA) (%)	PC <sub>1</sub> (1..5)	Bike Lane Mark (Y/N)	Cross Sec. (C/S)	Buff. Width (BW) (ft)	Tree Spcg. in Buffer (ft/ctr)	% with Sidewalk	Swalk Width (Ws) (ft)	Road Profile Cond (1,2,3)	Bicycle LOS		Pedestrian LOS		Comments
							Th #	Con				W <sub>t</sub> (ft)	W <sub>i</sub> (ft)	W <sub>ps</sub> (ft)											Score (0..7)	Grade (A..F)	Value (0..7)	Grade (A..F)	
1.0	580	Skinner Blvd	Broadway (Alt 19)	Douglas Ave	0.11	E	4	D	10,600	3	35	14.0	4.0	0.0	24.0	0	5.0	Y	C	5.0		100	6.0	1	2.36	B	2.49	B	SW is pavers from Broadway to Trail
1.0	580	Skinner Blvd	Broadway (Alt 19)	Douglas Ave	0.11	W	4	D	10,600	3	35	14.0	4.0	0.0	24.0	0	5.0	Y	C	5.0		100	6.0	1	2.36	B	2.49	B	SW is pavers from Broadway to Trail
1.1	580	Skinner Blvd	Howard Ave	Main St	0.36	E	4	S	10,600	3	40	15.5	4.5	0.0	65.5	0	5.0	Y	C	5.0		100	6.0	1	2.05	B	2.61	C	Buffer is 13' E of on N side between Douglas and Highland
1.1	580	Skinner Blvd	Howard Ave	Main St	0.36	W	4	S	10,600	3	40	15.0	4.0	0.0	65.5	0	5.0	Y	C	7.5		100	6.0	1	2.25	B	2.56	C	Buffer is 13' E of on N side between Douglas and Highland
2.0	580	Main St	Skinner Blvd/BassBlvd	Patricia Ave	0.44	E	4	S	17,800	4	40	15.0	4.0	0.0	60.0	0	4.5	Y	C	5.0		100	6.0	1	2.59	C	3.03	C	
2.0	580	Main St	Skinner Blvd/BassBlvd	Patricia Ave	0.44	W	4	S	17,800	4	40	15.0	4.0	0.0	60.0	0	4.5	Y	C	5.0		100	6.0	1	2.59	C	3.03	C	
2.1	580	Main St	Patricia Ave	Crosley Dr/Pinehurst Rd	0.25	E	4	S	28,500	4	40	15.0	4.0	0.0	60.0	0	4.5	Y	C	5.0		100	6.0	1	2.83	C	3.64	D	
2.1	580	Main St	Patricia Ave	Crosley Dr/Pinehurst Rd	0.25	W	4	S	28,500	4	40	15.0	4.0	0.0	60.0	0	4.5	Y	C	5.0		100	6.0	1	2.83	C	3.64	D	
3.0	580	Main St	Crosley Dr/Pinehurst Rd	Keene Rd/CR 1	0.50	E	6	S	31,500	3	45	16.0	4.0	0.0	95.0	0	4.5	Y	C	4.5		100	6.0	1	2.52	C	3.36	C	
3.0	580	Main St	Crosley Dr/Pinehurst Rd	Keene Rd/CR 1	0.50	W	6	S	31,500	3	45	16.0	4.0	0.0	95.0	0	4.5	Y	C	4.5		100	6.0	1	2.52	C	3.36	C	
4.0	580	Main St	Keene Rd/CR 1	Belcher Rd	1.00	E	6	S	47,500	3	45	16.0	4.0	0.0	95.0	0	4.0	Y	C	5.0		100	6.0	1	2.82	C	3.96	D	
4.0	580	Main St	Keene Rd/CR 2	Belcher Rd	1.00	W	6	S	47,500	3	45	16.0	4.0	0.0	95.0	0	4.0	Y	C	5.0		100	6.0	1	2.82	C	3.96	D	
5.0	580	Main St	Belcher Rd	US 19	0.53	E	6	S	48,500	3	45	16.0	4.0	0.0	95.0	0	4.0	Y	C	5.0		100	6.0	1	2.84	C	3.99	D	
5.0	580	Main St	Belcher Rd	US 19	0.53	W	6	S	48,500	3	45	16.0	4.0	0.0	95.0	0	4.0	Y	C	5.0		100	6.0	1	2.84	C	3.99	D	
6.0	580	SR 580	US 19	Countryside Blvd	0.80	E	6	S	40,000	3	45	16.0	4.0	0.0	95.0	0	4.0	Y	C	5.0		100	6.0	1	2.73	C	3.67	D	
6.0	580	SR 580	US 19	Countryside Blvd	0.80	W	6	S	40,000	3	45	16.0	4.0	0.0	95.0	0	4.0	Y	C	5.0		100	6.0	1	2.73	C	3.67	D	
7.0	580	SR 580	Countryside Blvd	McMullen Booth Rd	1.08	E	6	D	34,500	2	45	14.0	0.0	0.0	38.0	0	4.0	N	C	3.0		100	5.0	1	3.67	D	3.65	D	
7.0	580	SR 580	Countryside Blvd	McMullen Booth Rd	1.08	W	6	D	34,500	2	45	14.0	0.0	0.0	38.0	0	4.0	N	C	2.5		100	5.0	1	3.67	D	3.67	D	
8.0	580	SR 580	McMullen Booth Rd	Muellers Lane	0.49	E	6	D	37,000	4	45	14.5	0.0	0.0	38.5	0	4.0	N	C	3.0		100	5.0	1	3.66	D	3.73	D	
8.0	580	SR 580	McMullen Booth Rd	Muellers Lane	0.49	W	6	D	37,000	4	45	14.5	0.0	0.0	38.5	0	4.0	N	C	3.0		100	5.0	1	3.66	D	3.73	D	
8.1	580	SR 580	Muellers Lane	Phillippe Pkwy	0.47	E	4	D	37,000	4	45	15.0	4.0	0.0	26.5	0	4.0	Y	C	4.0		100	5.0	1	3.11	C	4.39	D	2 lanes EB/3 lanes WB
8.1	580	SR 580	Muellers Lane	Phillippe Pkwy	0.47	W	6	D	37,000	4	45	15.0	4.0	0.0	38.5	0	4.0	Y	C	3.0		100	5.0	1	2.90	C	3.72	D	3 lanes EB/3 lanes WB
9.0	580	SR 580	Phillippe	W end bridge	0.17	E	4	D	40,500	3	45	15.0	4.0	0.0	26.0	0	4.0	Y	C	4.0		100	5.0	1	3.14	C	4.59	E	
9.0	580	SR 580	Phillippe	W end bridge	0.17	W	4	D	40,500	3	45	15.0	4.0	0.0	26.0	0	4.0	Y	C	4.0		100	5.0	1	3.14	C	4.59	E	
9.1	580	SR 580	W end bridge	E end bridge	0.49	E	4	D	40,500	3	45	22.0	10.0	0.0	40.0	0	4.0	Y	C	1.0	1	100	5.0	4	0.00	A	3.72	D	Jersey Barrier
9.1	580	SR 580	W end bridge	E end bridge	0.49	W	4	D	40,500	3	45	22.0	10.0	0.0	40.0	0	4.0	Y	C	1.0	1	100	5.0	4	0.00	A	3.72	D	Jersey Barrier
9.2	580	SR 580	E end bridge	St Clair Ave	0.17	E	4	D	40,500	3	45	15.0	4.0	0.0	26.0	0	4.0	Y	C	4.0		100	5.0	1	3.14	C	4.59	E	
9.2	580	SR 580	E end bridge	St Clair Ave	0.17	W	4	D	40,500	3	45	15.0	4.0	0.0	26.0	0	4.0	Y	C	4.0		100	5.0	1	3.14	C	4.59	E	
9.3	580	SR 580	St Clair Ave	St Petersburg Dr/Forest Lakes Blvd	0.40	E	4	D	40,500	3	45	14.0	0.0	0.0	26.5	0	4.0	N	C	3.0		100	5.0	1	3.97	D	4.65	E	
9.3	580	SR 580	St Clair Ave	St Petersburg Dr/Forest Lakes Blvd	0.40	W	4	D	40,500	3	45	14.0	0.0	0.0	26.5	0	4.0	N	C	3.0		100	5.0	1	3.97	D	4.65	E	
10.0	580	State St	St Petersburg Dr/Forest Lakes Blvd	Hillsborough Ave	1.10	E	4	D	22,900	4	45	14.0	0.0	0.0	26.5	0	4.0	N	C	3.0		100	5.0	1	3.70	D	3.64	D	
10.0	580	State St	St Petersburg Dr/Forest Lakes Blvd	Hillsborough Ave	1.10	W	4	D	22,900	4	45	14.0	0.0	0.0	26.5	0	4.0	N	C	3.0		100	5.0	1	3.70	D	3.64	D	